TOWN OF MOUNTAIN VILLAGE DESIGN REVIEW BOARD REGULAR MEETING THURSDAY MAY 7, 2015, 10:00 AM

2nd FLOOR CONFERENCE ROOM, MOUNTAIN VILLAGE TOWN HALL 455 MOUNTAIN VILLAGE BLVD, MOUNTAIN VILLAGE, COLORADO AGENDA

REVISION 2

| | Time | Min. | Presenter | Туре | Description | | | |
|-----|-------|------|-----------|-------------|---|--|--|--|
| 1. | 10:00 | 45 | Hawkins | Site Visit | DRB will start a site visit at Hotel Madeline, 568 Mountain Village Blvd., meeting at the Reflection Plaza Ice Rink Area to Review Current Unit Configurations for the Major PUD Amendment Request to Be Heard Under Agenda Item No. 4 | | | |
| 2. | 10:45 | | | | Call to Order | | | |
| 3. | 10:45 | 5 | Hawkins | Action | Reading and Approval of Summary of Motions of the April 2, 2015 meeting of the Town Council & Design Review Board | | | |
| 4. | 10:50 | 45 | Hawkins | Action | Major PUD amendment to allow for: (A) the combination of a maximum of nine (9) lodge units to be rezoned into five (5) condominiums; (B) five (5) hotel condominiums to be rezoned as lodge units; (C) the combination of seven (7) condominium units into three (3) condominium units; and (D) a density transfer as needed to accomplish the foregoing. | | | |
| 5. | 11:35 | 15 | Bangert | Action | Consideration of a Minor Revisions Application for Lot 355 Requesting a General Easement Encroachment | | | |
| 6. | 11:50 | 30 | | | Lunch | | | |
| 7. | 12:20 | 15 | Bangert | Action | Consideration of a Design Review Process Development Application for Lot 204 Requesting a General Easement Encroachment | | | |
| 8. | 12:35 | 15 | Jameson | Action | Consideration of a Design Review Process Development Application for Lot 1001 and Tract OS- 1R1 Roof and Fence Variation | | | |
| 9. | 12:50 | 30 | Jameson | Worksession | Conceptual Worksession Regarding Synthetic Roofing Material in the Mountain Village | | | |
| 10. | 1:20 | 30 | Hawkins | Action | Major PUD amendment to extend the length of validity and vested property rights for a site specific development plan for Lot 109R from December 8, 2015 to December 8, 2020 | | | |

DESIGN REVIEW BOARD MEETING AGENDA MAY 7, 2015

| 11. | 1:50 | 45 | Hawkins | Worksession | Conceptual Worksession to Discuss a Proposed Rezoning, Density Transfer and Replat for Lots 376R and Lot 387R |
|-----|------|----|---------|-------------|---|
| 12. | 2:35 | | | | Adjourn |

SUMMARY OF MOTIONS TOWN OF MOUNTAIN VILLAGE DESIGN REVIEW BOARD/TOWN COUNCIL JOINT MEETING THURSDAY, APRIL 2, 2015

Agenda item #3

Call to Order

Mayor, Dan Jansen, called the joint meeting to order at 10:00 a.m. on Thursday, April 2, 2015, in the Conference Room at 455 Mountain Village Boulevard, Mountain Village, Colorado, 81435.

Attendance

The following Board/Alternate/Council members were present and acting:

Dan Jansen Cath Jett
Bill Hoins Dave Eckman
Greer Garner Phil Evans
Keith Brown Luke Trujillo
David Craige Jean Vatter
Banks Brown

The following Board members were absent:

Town Staff in attendance:

Chris Hawkins, Director of Community Development Savannah Jameson, Planner II Dave Bangert, Town Forester Jim Mahoney, Attorney

Public in attendance:

Dan Garner PJ Bauser Jackie Kennefick Gordon Richard **Andy Drissell** Erick Goodfriend Penelope Gleason **Brian Eaton** Brian Kanaga Michael McAllister Randy Pudolsky **David Ballode** Mike McCreedy **Travis Parsons** Stephanie Solomon **Tom Conyers**

<u>Presentation of the Town Hall Subarea Task Force Recommendation and Conceptual Work Session</u> with the Design Review Board for New Medical Center in the Town Hall Subarea.

Banks Brown joined the meeting at 10:46 a.m.

Town Council Adjourn at 11:00 a.m.

Reading and Approval of Summary of Motions of the March 5, 2015 Design Review Board Meeting
On a Motion made by Phil Evans and seconded by Greer Garner, the DRB voted 7-0 to approve the
Summary of Motions from the March 5, 2015 meeting with the addition of Jonathan Augello attending

the meeting and Michael Balser and Ronald Alvarez not attending.

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Design Review Board Annual Election of Chair, Vice-Chair and Temporary Chair

On a **Motion** made by Phil Evans and seconded by Banks Brown, the DRB voted 7-0 to keep Bill Hoins as Chair and Dave Eckman as Vice-Chair.

<u>Consideration of a Design Review Process Application for a New Single-family Residence on Lot 364R</u>
Bill Hoins left the meeting at 11:06 a.m. David Craige recused himself due to a conflict of interest for this agenda item.

Town Forester/Planner Dave Bangert, presented for the Design Review Process application. Owner's representative and applicant, Tom Conyers, presented for the application.

Upon review and discussions, on a **Motion** by Banks Brown and seconded by Keith Brown the DRB voted 7-0, to approve a Resolution approving a Design Review Process application for Lot 364R.

Community Development Director Chris Hawkins requested the Design Review Board consider hearing item number 9 – Amendments to Community Development Code (CDC) next, thereby moving item number 8 Conditional Use Permit and Variance for 100' Tower to be heard after lunch.

Consideration of a Recommendation to the Town Council for Amendments to the Community Development Code (CDC) at (A) 17.3.4(F)(4) to Allow for the Resubdivision and Rezoning of Single-Family Lots Subject to Modified Criteria; and (B) 17.6.3 to Revise the Condominium-Hotel Regulations. Director of Community Development, Chris Hawkins, requested consideration of a recommendation to the Town Council for amendments to the Community Development Code (CDC).

Upon review and discussions, on a **Motion** made by Greer Garner and seconded by Banks Brown, the DRB voted 7-0 approve the recommendation to the Town Council for amendments to the Community Development Code (CDC) with the amendments as read into the record by staff.

Lunch 12 pm - 12:30 pm

Consideration of a Conditional Use Permit and Variance for 100'-tall Telecommunication Tower Located Next to Existing Tower on OSP49

Upon review and discussions on a **Motion** made by Greer Garner and seconded by Phil Evans, the DRB voted 7-0 to approve the recommendation to Town Council for a conditional use permit and variance with findings as stated in the staff memo of record dated March 26, 2015 and the following conditions:

- 1. The tower shall not include a light beacon or be brightly painted to stand out to aircraft.
- 2. The tower shall implement the following visual mitigation plan: The towers and antennas shall be painted to match the surrounding tree color.
- 3. The new tower shall be designed to handle as much colocation as possible.
- 4. The current and proposed towers shall be made available for colocation of new telecommunication equipment so long as: a) there is enough room on the tower for the new equipment (given the vertical & horizontal separation requirements of the current users), b) there is enough structural capacity for the new equipment, and c) the new equipment will not cause interference to the current users.
- 5. Prior to issuing a building permit, the applicant shall submit long-term easements from The Ridge, TSG ant any other intervening property owner for (1) the access road to the

tower site; (2) the tower site; and (3) utility routes for existing and new utilities to the site.

- 6. Prior to issuing a building permit, the applicant shall submit a composite utility plan to show the planned routes for power and fiber to the site.
- 7. The applicant shall show collocated antennas on the proposed plans.
- 8. The applicant shall be required to provide replacement trees for any trees removed.
- 9. Dimensions of antennas shall be shown on all plans.

Public Comment:

Stephanie Soloman addressed the Board regarding the existing tower and its ownership.

Other Business:

With no other business on a Motion made by Keith Brown and seconded Luke Trujillo, the DRB voted 7-0 to adjourn the April 2, 2015 meeting of the Mountain Village Design Review Board at 2:10 p.m.

Respectfully Submitted,

Savannah Jameson Planner II



COMMUNITY DEVELOPMENT DEPARTMENT PLANNING DIVISION

455 Mountain Village Blvd. Mountain Village, CO 81435 (970) 728-1392

TO: Design Review Board

FROM: Chris Hawkins, Director of Community Development

FOR: Meeting of May 7, 2015

DATE: April 30, 2015

RE: Major PUD amendment to allow for: (A) the combination of a maximum of nine (9) lodge units to be rezoned into five (5) condominiums; (B) five (5) hotel condominiums to be rezoned as lodge units; (C) the combination of seven (7) condominium units into three (3) condominium units; and (D) a density transfer as needed to accomplish the foregoing.

PROJECT GEOGRAPHY

Legal Description: Lot 38-50-51R and OS-1-MVB, Mountain Village Filing No. 1

Address: 568 Mountain Village Blvd.

Applicant/Agent: Dylan Henderson

Owner: Madeline Property Owner, LLC, dba Northview Hotel Group

Zoning: Village Center Zone District Existing Use: Mixed Use Development

Proposed Use: Outlined in Memo

Adjacent Land Uses:

North: Franz Klammer LodgeSouth: Meadows Ski Run & Granita

East: Plaza Condos and Columbia Condos

> West: Courcheval & Heritage Parking Garage Entry

Lot Size:

> OS-1-MVB: 0.092 acre (3,996 sq. ft.)

> Lot 38-50-51R: 1.892 acres

ATTACHMENTS

1. Exhibit A. Applicant Narrative and Plans

RECORD DOCUMENTS

- Town of Mountain Village Community Development Code (as amended)
- Town of Mountain Village Home Rule Charter (as amended)
- Design Review Application as maintained by the Community Development Department.

BACKGROUND

The applicant is proposing a PUD amendment to allow for the combination of units as follows:

| Building | Combined Units | Existing Designation | Proposed Designation |
|-------------|-----------------------|--------------------------|-----------------------------|
| 38 Building | 1403 & 1404 | Lodge | Condominium |
| 38 Building | 1405 & 1406 | Lodge | Condominium |
| 38 Building | 1407 & 1408 | Lodge | Condominium |
| 38 Building | 1502, 1505 & 1508 | Lodge & Efficiency Lodge | Condominium & Lodge |
| 38 Building | 1503 & 1504 | Lodge & Condominium | Condominium & Lodge |

The proposed combination has the following overall density change:

| Existing Unit Number | Existing Designation | Existing Person Equivalent Density | Proposed Unit Number | Proposed Designation | Proposed Person Equivalent Density |
|-----------------------------|-------------------------|---|----------------------------|-------------------------|---|
| 1403 | Lodge | 0.75 | 1404 | Condominium | 3 |
| 1404 | Lodge | 0.75 | | | |
| 1405 | Lodge | 0.75 | 1405 | Condominium | 3 |
| 1406 | Lodge | 0.75 | | | |
| 1407 | Lodge | 0.75 | 1407 | Condominium | 3 |
| 1408 | Lodge | 0.75 | | | |
| 1502 | Lodge | 0.75 | 1502 | Condominium | 3 |
| 1503 | Condominium | 3 | 1503 | Condominium | 3 |
| 1504 | Lodge | 0.75 | 1504 | Condominium | 3 |
| 1505 | Lodge | 0.75 | 1505 | Lodge | 0.75 |
| 1508 | Eff. Lodge | 0.5 | | | |
| HC 329, 419, 519, 520 & 525 | Eff. Lodge | 2.5 | No change | Lodge | 3.75 |
| Total Density | | 12.75 | | | 22.5 |
| Density to Trans | fer | 9.75 | | | |
| Total Units | 11 | | | 7 | |

The proposal is to convert 11 units as outlined above into 6 condominium units and one lodge unit. The applicant is seeking these unit combinations due to poor architectural design and due to the fact that two and three bedroom units have an approximate 10% higher occupancy rate than one-bedroom units. The DRB should refer to the applicant's narrative for further background on the reasons for the request.

The applicant is also seeking to clean up a situation where five efficiency lodge units were designed and constructed as two room spaces, which is a lodge unit per the following definitions of the Community Development Code (CDC):

Efficiency Lodge: A zoning designation that allows for a habitable, one-room space with separate bath and limited kitchen facilities used primarily for short-term accommodations. Limited kitchen facilities may include a sink, microwave, two-element

burner, and six (6) cubic foot (maximum) refrigerator. These units may be in a condominium community.

Lodge: A zoning designation that allows for a two (2) room space plus a mezzanine with up to two separate baths and a full kitchen. These units may be in a condominium community.

The proposed unit combinations results in the need to transfer 9.75 person equivalents of density. The applicant is proposing that the density be transferred from either (1) an allowance to combine units on the top floor of the 50-51 Building; or (2) a density transfer from the density bank. Staff is supportive of this approach since it provides the Madeline Hotel with flexibility and options, with the revised PUD agreement requiring a minor amendment to reflect the final density transfer and any on-site unit combinations.

The PUD amendment is also seeking to vary the following CDC limitation in Section 17.4.9(E)(7)(f):

"Lodge, efficiency lodge, hotel and hotel efficiency zoning designations may not be rezoned to condominium zoning designations."

Staff believes this CDC provision was established to prevent the loss of hotbed units over time. The PUD Regulations allow for the creation of unique policies for a site to allow for creativity and flexibility in development. Staff is supportive of this change since it appears that most of the proposed condominium units will be placed in the Madeline Hotel's rental pool, with several of the non-hotel deed restricted units that have been purchased within the property placed into the rental program even though there is no legal requirement. Moreover, several of the units are poorly designed and would create a negative guest experience if they are not allowed to combine the units. Last, the unit combinations allow the applicant to create attractive, functional units that can be better marketed and sold.

CRITERIA FOR DECISION

The criteria for decision for a PUD amendment are the same as for the creation of a PUD:

- 1. The proposed PUD is in general conformity with the policies, principles and standards set forth in the Comprehensive Plan;
- 2. The proposed PUD is consistent with the underlying zone district and zoning designations on the site or to be applied to the site unless the PUD is proposing a variation to such standards:
- The development proposed for the PUD represents a creative approach to the development, use of land and related facilities to produce a better development than would otherwise be possible and will provide amenities for residents of the PUD and the public in general;
- 4. The proposed PUD is consistent with and furthers the PUD purposes and intent;
- 5. The PUD meets the PUD general standards;
- 6. The PUD provides adequate community benefits;
- 7. Adequate public facilities and services are or will be available to serve the intended land uses:
- 8. The proposed PUD shall not create vehicular or pedestrian circulation hazards or cause parking, trash or service delivery congestion; and
- 9. The proposed PUD meets all applicable Town regulations and standards unless a PUD is proposing a variation to such standards.

MAJOR PUD AMENDMENT ANALYSIS

Staff believes that the proposed PUD amendment meets the major PUD amendment criteria for decision as outlined in the findings set forth below the proposed motion.

RECOMMENDATION

Staff recommends the DRB vote to recommend Town Council approve the major PUD amendment with the following motion:

"I move to recommend Town Council the major PUD amendment with the findings and conditions contained in the staff memo of record dated April 30, 2015.

Findings:

- 1. The proposed PUD is in general conformity with the policies, principles and standards set forth in the Comprehensive Plan;
- 2. The proposed PUD is consistent with the underlying zone district and zoning designations on the site or to be applied to the site unless the PUD is proposing a variation to such standards;
- 3. The development proposed for the PUD represents a creative approach to the development, use of land and related facilities to produce a better development than would otherwise be possible and will provide amenities for residents of the PUD and the public in general;
- 4. The proposed PUD is consistent with and furthers the PUD purposes and intent;
- 5. The PUD meets the PUD general standards;
- 6. The PUD provides adequate community benefits;
- 7. Adequate public facilities and services are or will be available to serve the intended land uses:
- 8. The proposed PUD shall not create vehicular or pedestrian circulation hazards or cause parking, trash or service delivery congestion; and
- 9. The proposed PUD meets all applicable Town regulations and standards unless a PUD is proposing a variation to such standards.

Conditions:

- The PUD agreement will be amended to require density to be transferred from either within the property or from the density bank, with minor PUD amendment development applications in the future to account for unit combinations and density transfers.
- 2. The PUD agreement will allow for the conversion of lodge and efficiency lodge units only in the Lot 38 Building to condominium units in order to create attractive, functional and higher occupancy units.
- 3. The PUD agreement will allow for the combination of only the condominium units only on the top floor of the Lot 50-51 Building.
- 4. The PUD agreement will require the Town to be notified of a proposed unit combination and the associated, required density transfer prior to or concurrent with the required building permits.
- 5. Unit combinations shall require an amendment to the condo map and declaration prior to the issuance of a certificate of completion for such combinations.



April 1, 2015

Town of Mountain Village Town Council and Design Review Board Members,

Affiliates of Northview Hotel Group ("Northview"), in partnership with a fund (the "Partnership") managed by an affiliate of Apollo Global Management, LLC (NYSE:APO) purchased Madeline Hotel and Residences (the "Property"), on August 22, 2014. Northview is operating the Property on behalf of the Partnership, and overseeing multiple capital improvements projects totaling approximately \$15mm, and the sale of 60 Residences and 11 Hotel Condominiums.

The Partnership and Mountain Village have much to gain from the successful (i) sale of the Property's Residences (the Property's bed base will more than double), and (ii) implementation of improvement projects, which will grow the value of the Residences as well as allow the Property to compete globally for high-end business.

The Property was intended to be Town of Mountain Village's premier, luxury resort property. The Partnership would like to ensure this becomes our new reality, collectively. While the Property is Virtuoso®-preferred, a member of The Leading Hotels of The World©, and AAA four-diamond rated, the Property has considerable flaws and falls short of its full potential.

MADELINE BUILDING 38 RESIDENCE RECONFIGURATIONS

One of the considerable flaws is the design of certain Residences in the 38 Building. Due to the roof lines, certain Residences are severely compromised.



The image to the left is of the living/dining room in Residence 1403, a one-bedroom Residence. As you can see, the roof line terribly compromises the Residence, from an ownership and guest enjoyment standpoint. This is a Residence that would need to sell for a very low price, yet in the end, filling it with guests that assume they are staying in Telluride's finest hotel would be a disaster. If Residence 1403 were combined with the adjacent Residence 1404, MPO could create a very nice three-

bedroom Residence, with great high-ceiling living room and mountain views, and 75% of the space in the photo above would be converted into a kids' media/bunk room.

The attached floor plates depict the changes MPO would like to make to ten Residences in the 38 Building.

Residences 1403 and 1404. These two one-bedroom Residences would be combined into one three-bedroom Residence. The combination solves the challenge of the roof line which destroys Residence 1403. A draft Residence reconfiguration plan is included with the floor plates and these drawings will be more detailed and included in the final PUD Amendment Application ASAP. To achieve this design, MPO would combine two



lodge units (0.75 persons of density each) into one condominium unit (3.0 persons of density required), requiring an additional 1.5 persons of density.

Residences 1405 and 1406. These two one-bedroom Residences are a mirror image of Residences 1403/1404, and the same scope is proposed. To achieve this design, MPO would combine two lodge units (0.75 persons of density each) into one condominium unit (3.0 persons of density required), requiring an additional 1.5 persons of density.

Residences 1407 and 1408. As shown in the floor plates, Residence 1407 is severely impacted by the roof line. These two one-bedroom Residences would be combined into one two-bedroom Residence. To achieve this design, MPO would combine two lodge units (0.75 persons of density each) into one condominium unit (3.0 persons of density required), requiring an additional 1.5 persons of density.

Residences 1502 and 1505/1508. As shown in the floor plates, the living/dining space in Residence 1502 is significantly larger than in Residence 1505/08, yet Residence 1502 is a one-bedroom Residence and 1505/1508 is a one-bedroom Residence with a very large loft space above. MPO is proposing Residence 1502 assume the upstairs loft space of Residence 1505/08 to make a great two-bedroom Residence. To achieve this design, MPO would rezone one lodge unit (0.75 persons of density each) into one condominium unit (3.0 persons of density required), requiring an additional 2.25 persons of density.

Residences 1503 and 1504. As shown in the floor plates, the living spaces of these Residences are identical, yet Residence 1503 is a three-bedroom Residences and Residence 1504 is a one-bedroom Residence. MPO is proposing Residence 1504 assume the third bedroom of Residence 1503 to make two great two-bedroom Residences. To achieve this design, MPO would rezone one lodge unit (0.75 persons of density each) into one condominium unit (3.0 persons of density required), requiring an additional 2.25 persons of density.

MPO is requesting the option to perform these Residence reconfigurations in the future, provided it has the necessary density. In total, MPO would need 9.0 persons of density to achieve these designs, it would be adding two bedrooms in total, and it would be converting (the attached table includes more detail):

- 1. On the fourth floor, six one-bedroom lodge units into three condominium units.
- 2. On the fifth floor, two one-bedroom lodge units into two condominium units.

All of the reconfigurations are proposed in order to make the Residences sellable, as well as to ensure the enjoyment by the guests of Telluride's finest hotel.

LODGE UNIT VS RESIDENCE DEMAND

Madeline Hotel and Residences has been operating a residence rental management program for at least two years, including approximately 20 of the 60 residences. The table below highlights the 2013 performance:

| | AVAILABLE | OCCUPANCY | AVG DAILY RATE |
|-------------------------|-----------|-----------|----------------|
| One-Bedroom Residence | 4 | 34% | \$347 |
| Two-Bedroom Residence | 7 | 44% | \$443 |
| Three-Bedroom Residence | 8 | 44% | \$567 |
| Four-Bedroom Residence | 1 | 38% | \$900 |



Even with double the available inventory of two- and three-bedroom Residences vs. one-bedroom Residences, the occupancy of the two- and three-bedroom Residences is 30% higher than the one-bedroom Residences. This is a trend MPO has experienced throughout the industry – larger accommodations are in higher and higher demand each year, given the growing size of the guest party, including multiple generations.

By combining these one-bedroom Residences into two- and three-bedroom Residences, MPO firmly believes overnight transient demand will increase, not decrease. Moreover, with these reconfigurations, the guest experience and loyalty will increase significantly.

MADELINE HOTEL CONDOMINIUM SUITES

One of the other flaws in the project is the zoning of the larger hotel condominium units, including HC 329, HC 419, HC 519, HC 520 and HC 525. For reference, the average size of these larger hotel condominium units is 1,182 square feet; the average size of a Madeline one-bedroom Residence (zoned Condominium units) is 990 square feet. The floorplans of these hotel condominiums are attached and show that these units include a two (2) room space, yet they are zoned Efficiency Lodge units.

Per the CDC, Efficiency Lodge is "a zoning designation that allows for a habitable, one-room space with separate bath." Per the CDC Table 3-2, Efficiency Lodge units are 0.5 persons of density.

Per the CDC, Lodge is "a zoning designation that allows for a two (2) room space plus a mezzanine with up to two (2) separate baths." Per the CDC Table 3-2, Lodge units are 0.75 persons of density.

MPO plans to sell these hotel condominium to individual purchasers, yet this inaccuracy has been causing significant hurdles for potential purchasers. To clear up the confusion, MPO is requesting that these five hotel condominiums be rezoned as Lodge units, which would require 1.25 persons of density in total. This request is not intended to change the Hotel Deed Restriction or Unit Reconfiguration Restrictions associated with these hotel condominiums.

DENSITY REQUIREMENTS

As proposed, 10.25 persons of density would be required to achieve the proposed reconfigurations and rezonings. MPO is proposing two options to obtain the necessary density.

The first is MPO would combine seven Residences (condominium units) on the seventh floor of the 50/50 Building, to create three Penthouse Residences (condominium units). This would make available 12.0 persons of density. MPO will be testing the market feasibility of this option this summer. The second is MPO would purchase the necessary density.

If the necessary density was not obtained, MPO would not have the ability to move forward with the proposed reconfigurations or rezonings.



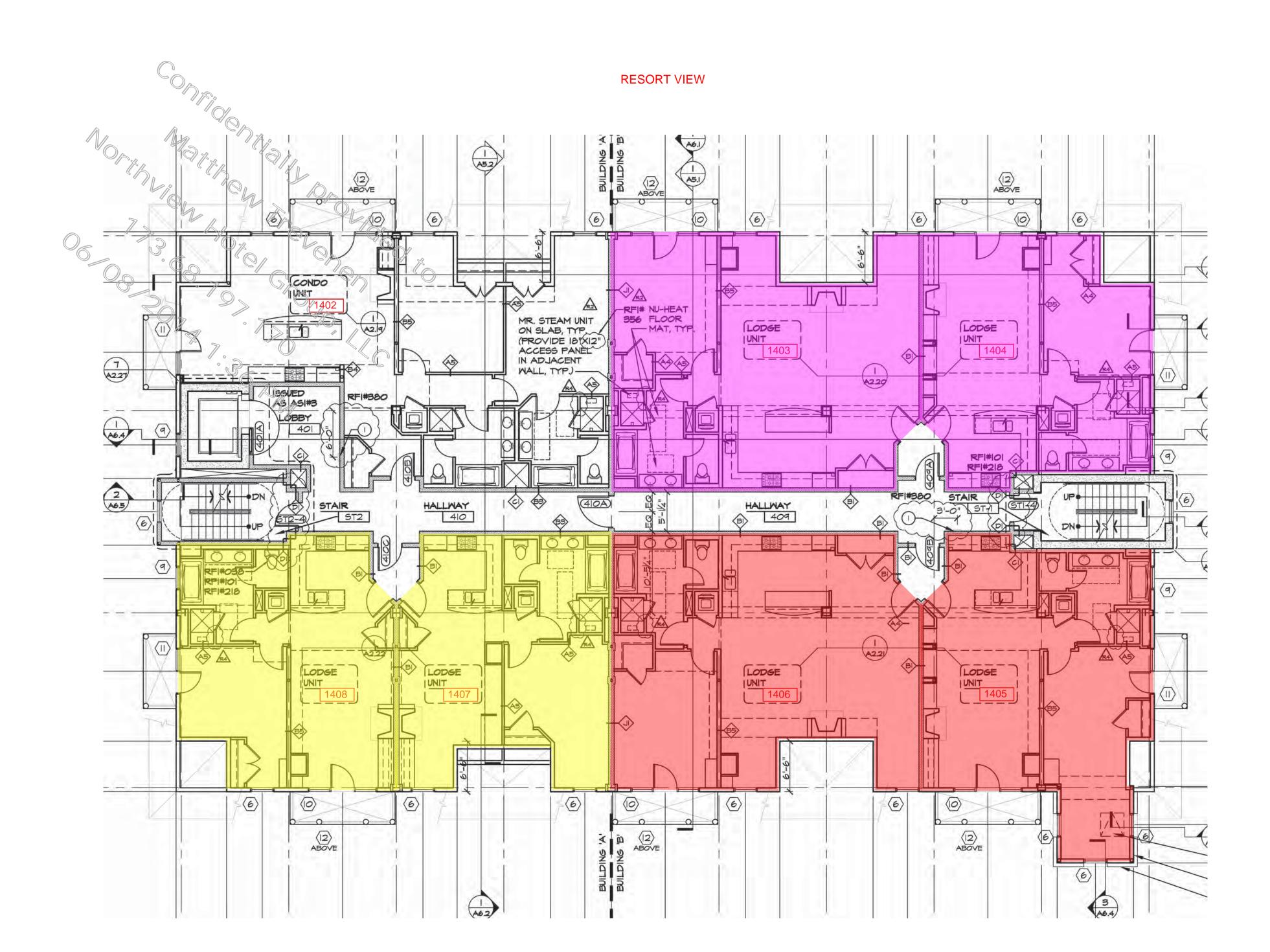
It is time the Property achieve its potential, and I truly appreciate you your time and consideration.

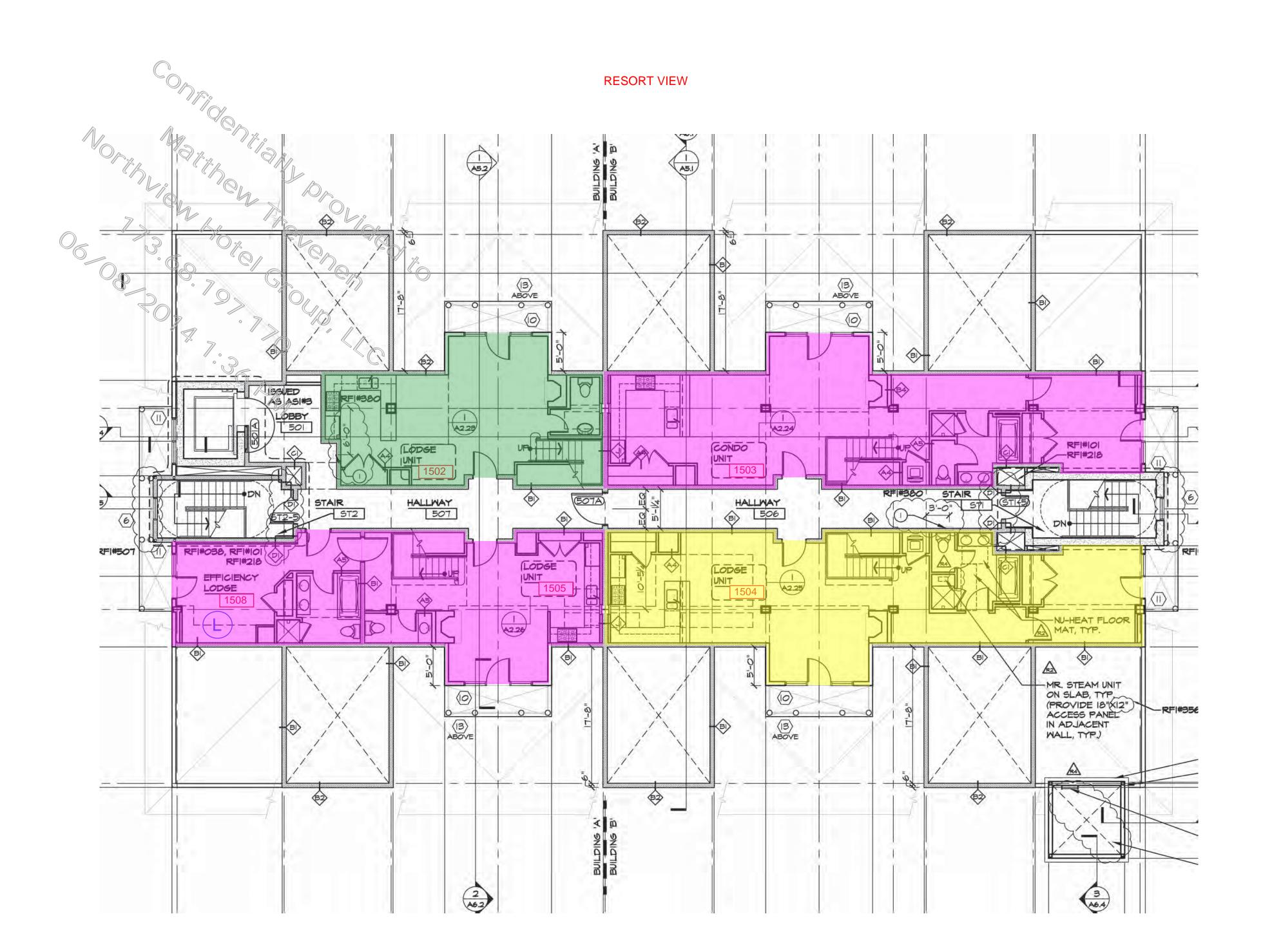
Sincerely,

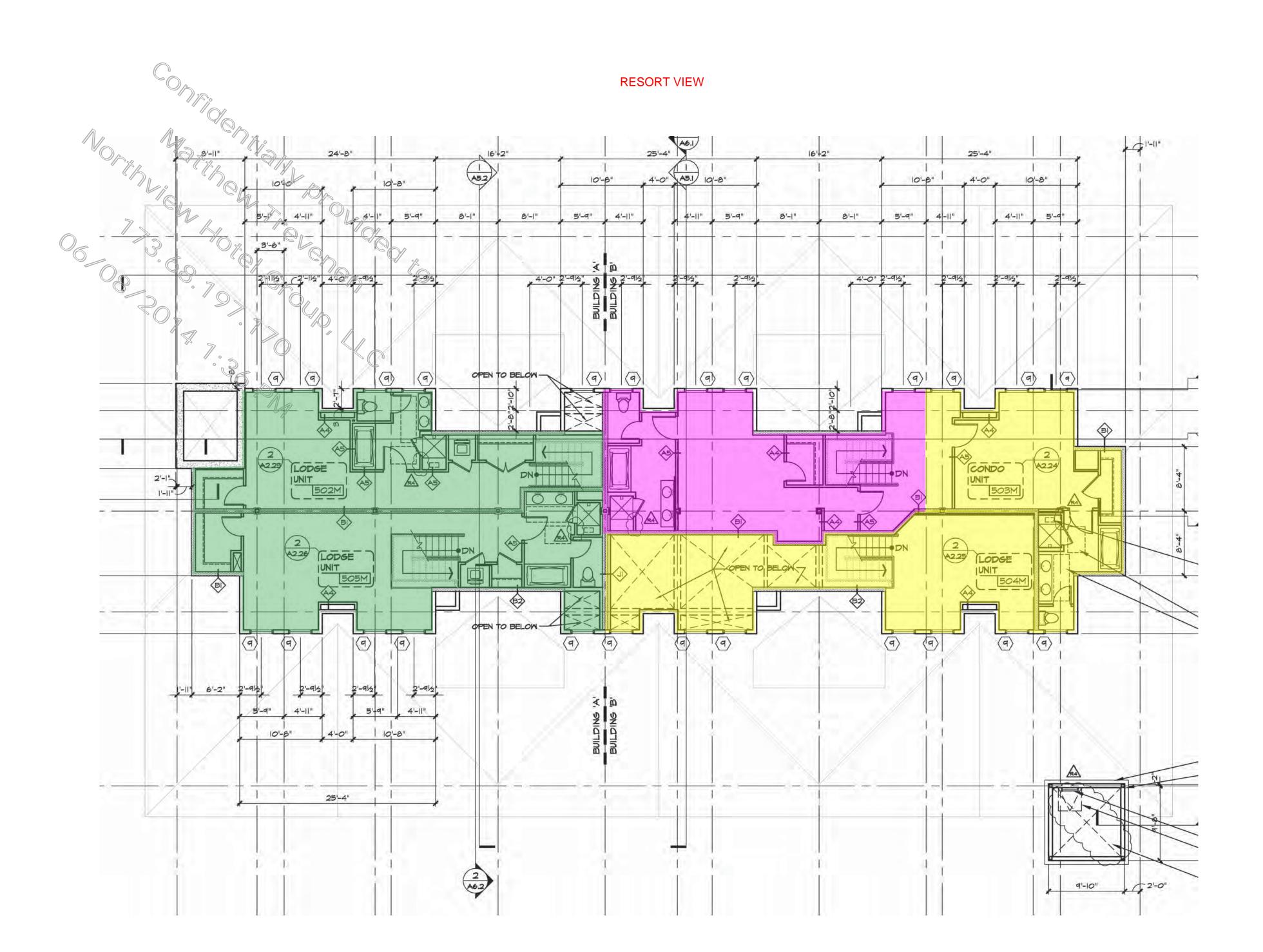
Brent P. McLean

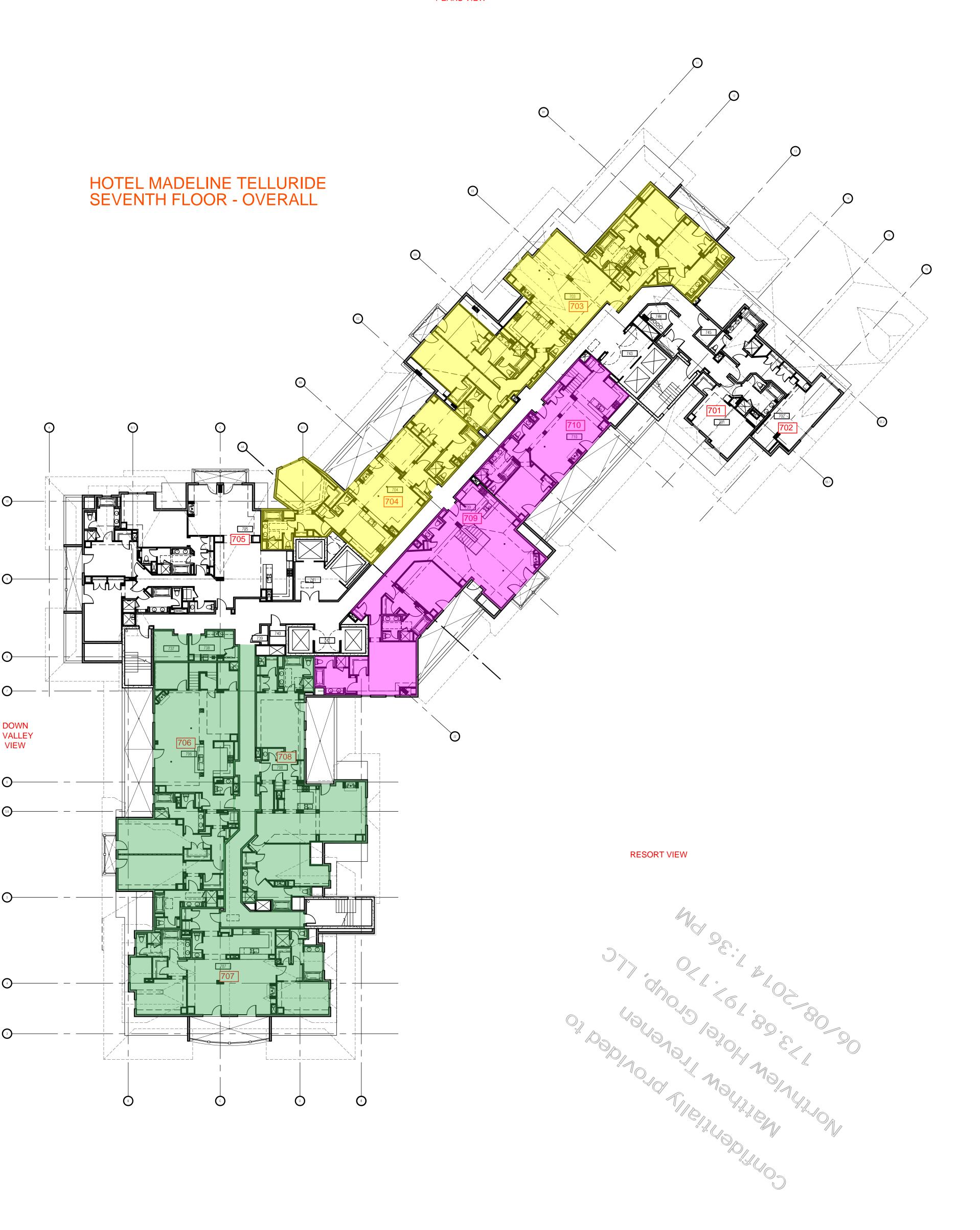
Senior Vice President – Real Estate

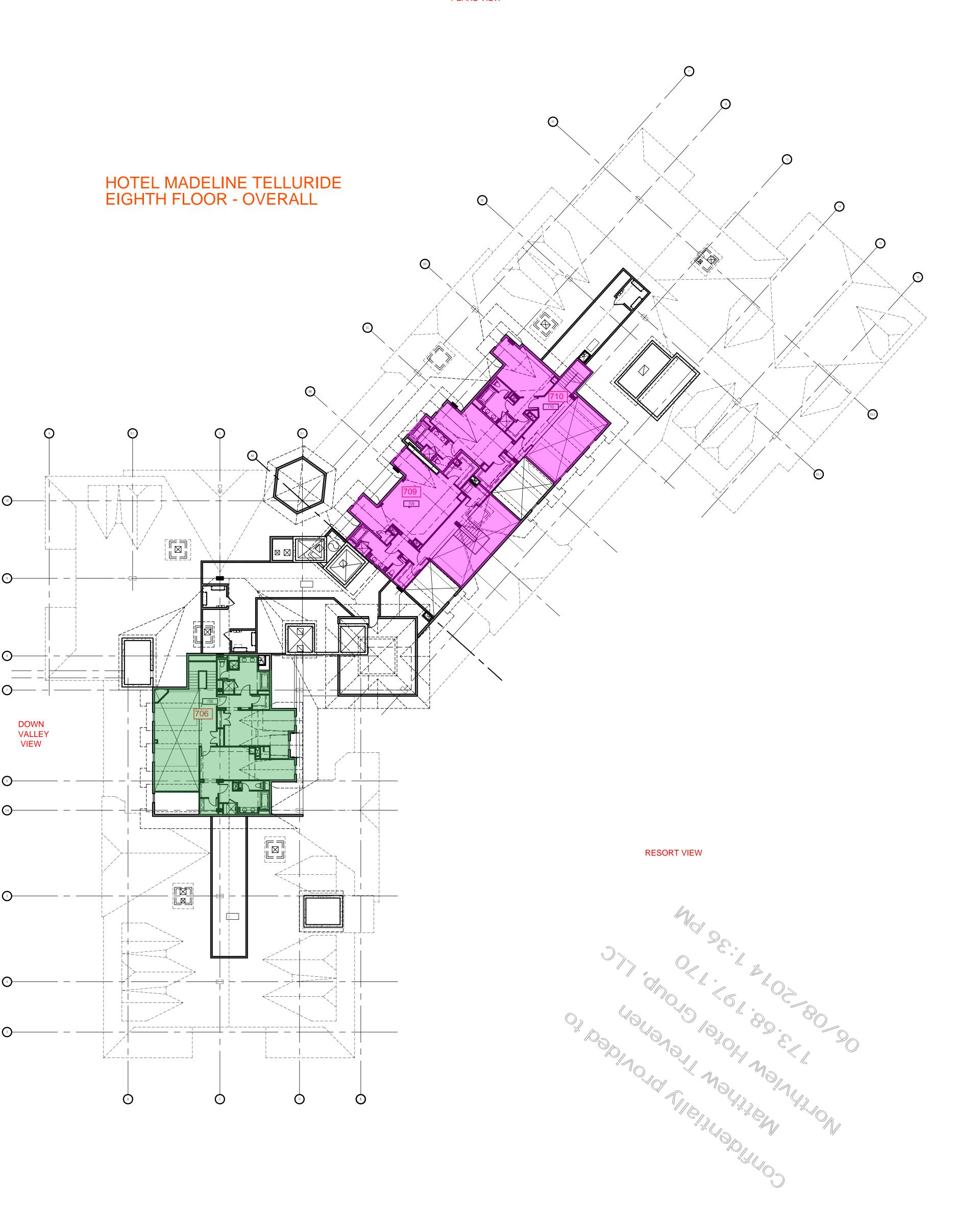
Northview Hotel Group







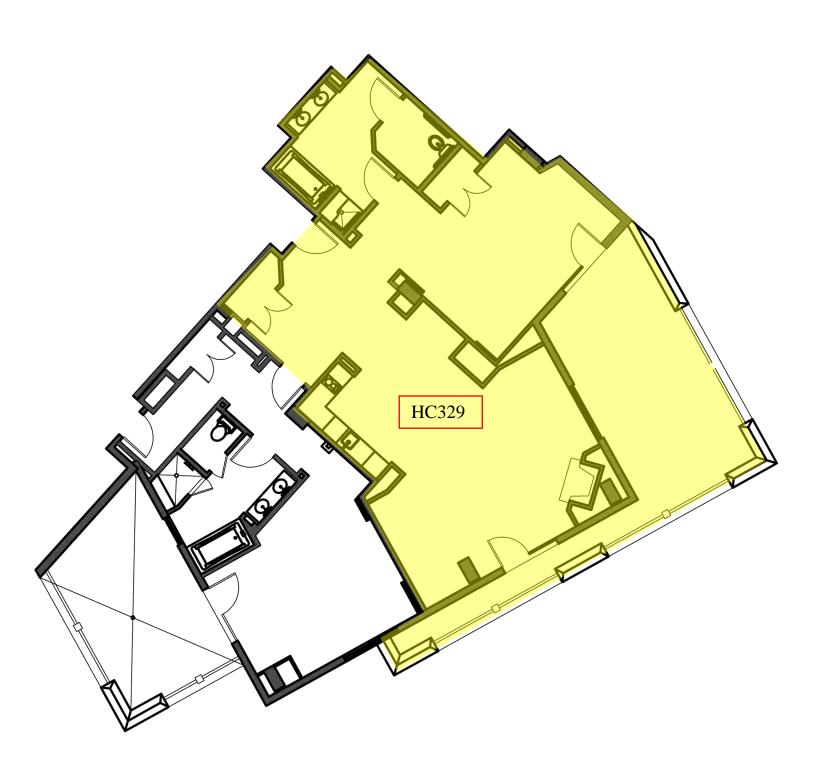


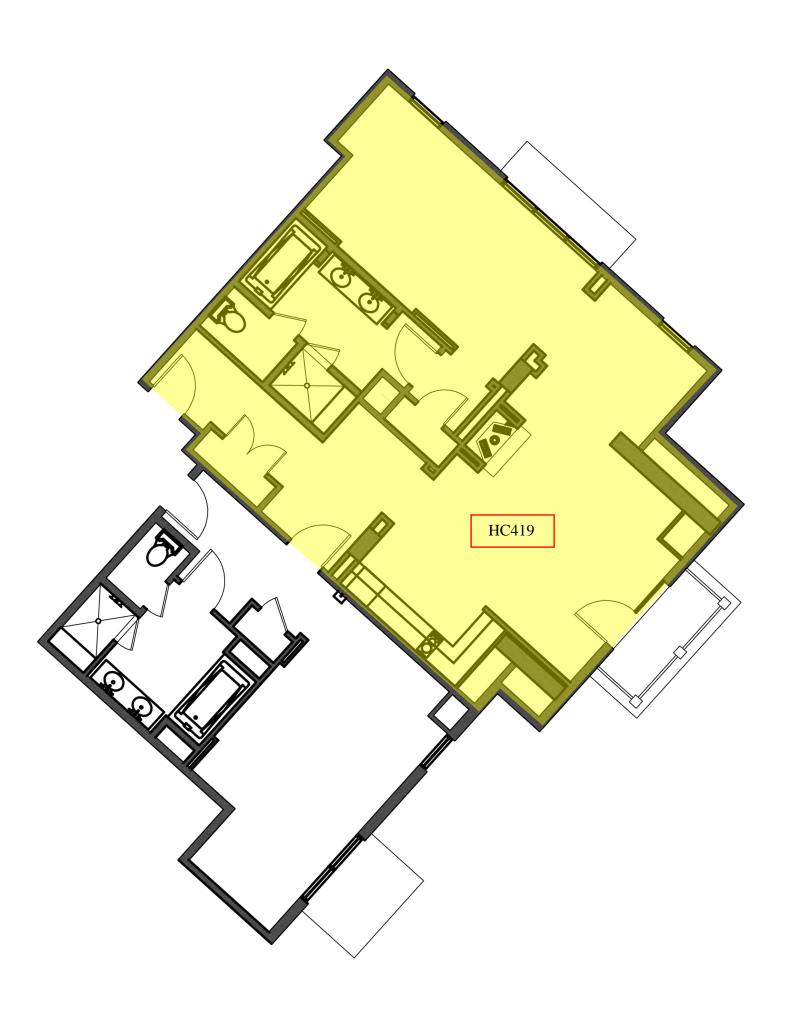


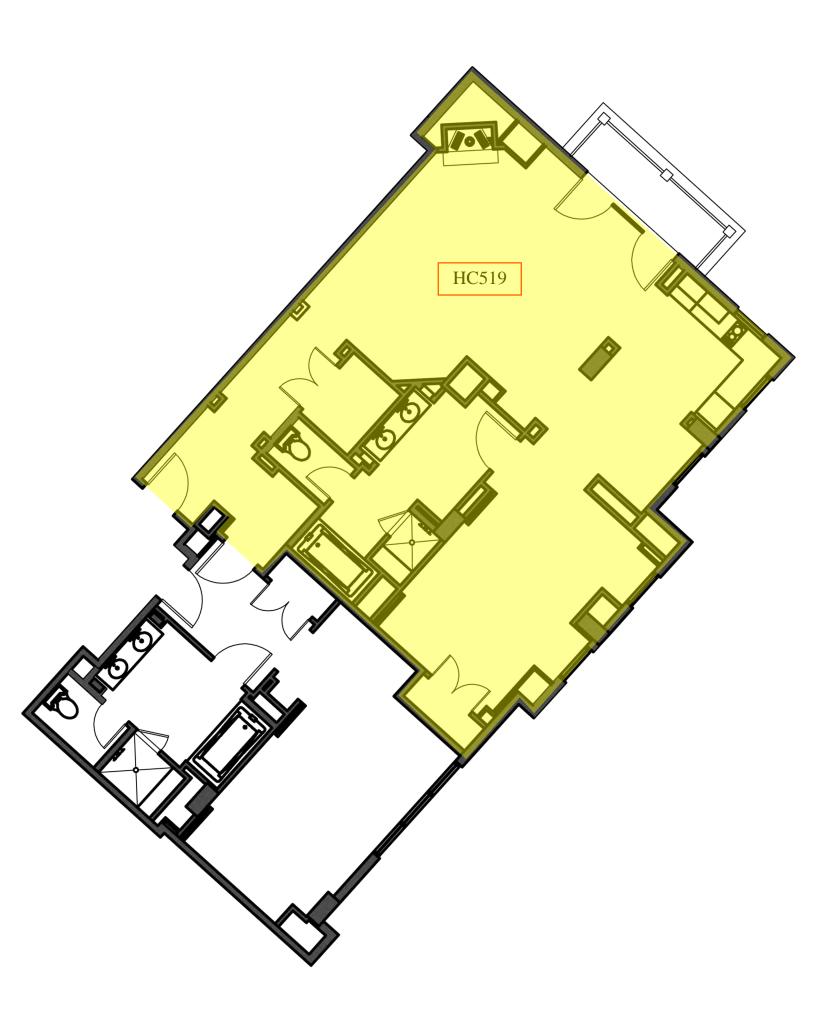
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|---------------|-------------------|-------|-----------|-----------------------|-------------|-------|--------|-------|---------|--|
| Hotel Unit | Condo Map Unit | Bldg | Zoning | Persons of Density | Net Density | Beds | Baths | SF | Deck SF | Note |
| 1403 | RC-403 | 38 | Lodge | 0.75 | • | 1 | 1 | 1,151 | 99 | Very dark residence due to roof lines |
| 1404 | RC-404 | 38 | Lodge | 0.75 | | 1 | 1 | 820 | 98 | |
| Sub-Total | | | | 1.5 | | 2 | 2 | 1,971 | 197 | |
| 1403/1404 Re | configured | | Condo | 3 | (1.50) | 3 | 2.5 | 1,971 | 197 | |
| Variance | | | | | | | | | | |
| 1405 | RC-405 | 38 | Lodge | 0.75 | | 1 | 1 | 929 | 98 | Very dark residence due to roof lines |
| 1406 | RC-406 | 38 | Lodge | 0.75 | | 1 | 1 | 1,150 | 57 | |
| Sub-Total | | | | 1.5 | | 2 | 2 | 2,079 | 155 | |
| 1405/1406 Re | configured | | Condo | 3 | (1.50) | 3 | 2.5 | 2,079 | 155 | An offer is in for these units, with the buyer looking to complete the work |
| Variance | | | | | | | | | | |
| 1407 | RC-407 | 38 | Lodge | 0.75 | | 1 | 1 | 771 | 0 | Very dark residence due to roof lines |
| 1408 | RC-408 | 38 | Lodge | 0.75 | | 1 | 1 | 789 | 98 | , and the second |
| Sub-Total | | | | 1.5 | | 2 | 2 | 1,560 | 98 | |
| 1407/1408 Re | configured | | Condo | 3 | (1.50) | 2 | 2.0 | 1,560 | 98 | Lowest priced two-bedroom residence in the project |
| Variance | | | | | | | | | | |
| 1502 | RC-502 | 38 | Lodge | 0.75 | | 1 | 1.5 | 980 | 56 | Takes over 1505 upper loft as second bedroom |
| 1502 Reconfig | gured | | Condo | 3 | (2.25) | 2 | 2.5 | 1,480 | 56 | This is an estimate of the SF |
| Variance | | | | | | | | | | |
| 1503 | RC-503 | 38 | Condo | 3 | | 3 | 3 | 2,022 | 93 | Gives up third bedroom to 1504 |
| 1503 Reconfig | gured | | Condo | 3 | 0.00 | 2 | 2 | 1,517 | 93 | This is an estimate of the SF |
| Variance | | | | | | | | | | |
| 1504 | RC-504 | 38 | Lodge | 0.75 | | 1 | 1 | 1,280 | 93 | Takes over 1503 third bedroom to become two bedroom + loft residence |
| 1504 Reconfig | gured | | Condo | 3 | (2.25) | 2 | 2 | 1,786 | 93 | This is an estimate of the SF |
| Variance | | | | | | | | | | • |
| 1505 | RC-505 | 38 | Lodge | 0.75 | | 1 | 1.5 | 1,038 | 56 | Gives up loft space to 1502 |
| 1508 | RC-506 | 38 | E Lodge | 0.5 | | 0 | 0 | 312 | 39 | No change - will be sold with 1505 |
| Sub-Total | | | | 1.25 | | 2 | 2 | 1,350 | 95 | • |
| 1505/1508 Re | configured | | No Change | 1.25 | 0.00 | 1 | 1.5 | 850 | 95 | This is an estimate of the SF |

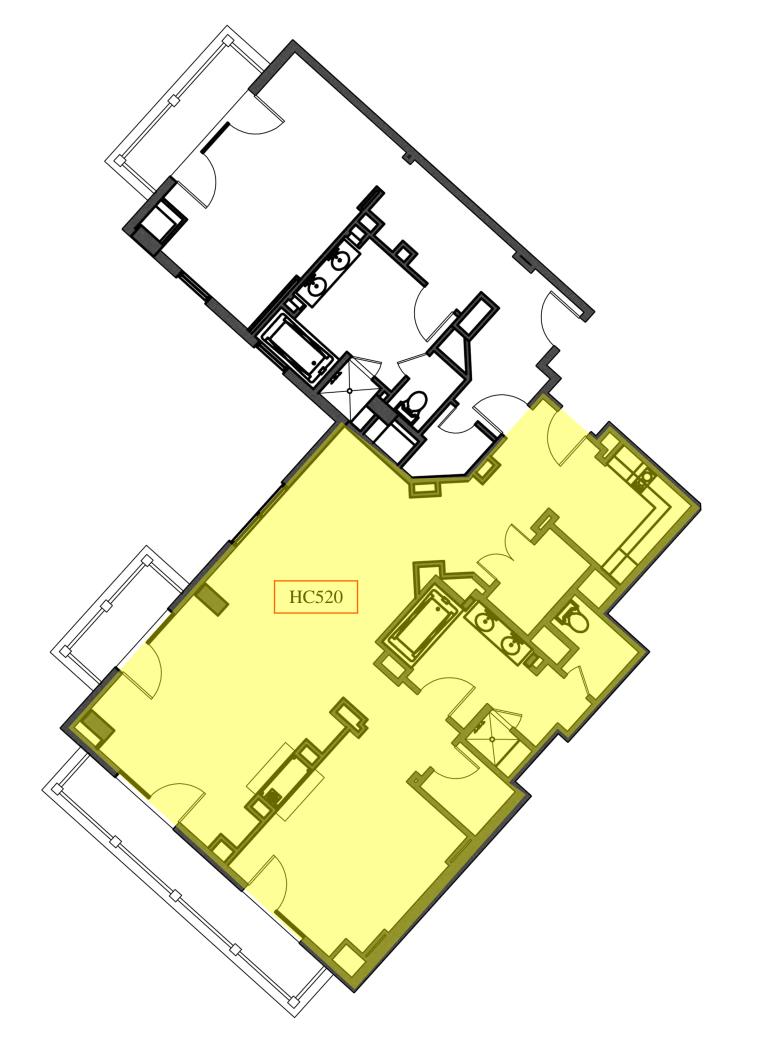
Variance

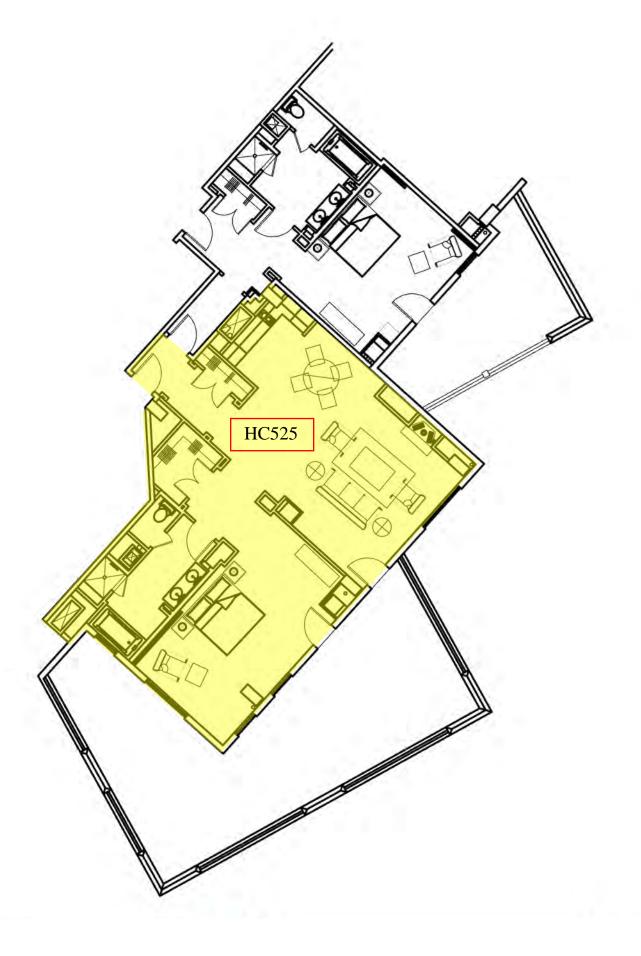
Total (9.00)

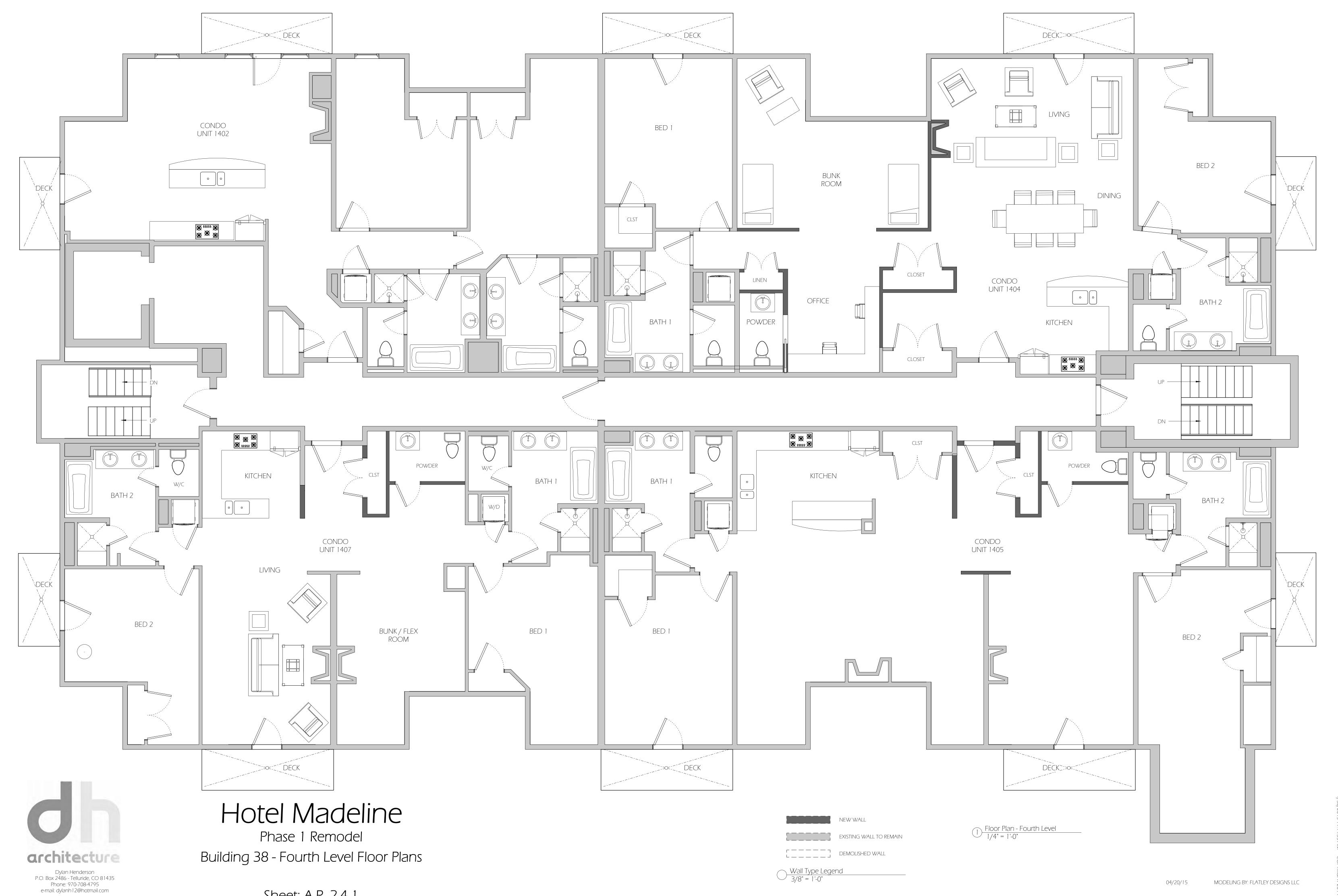




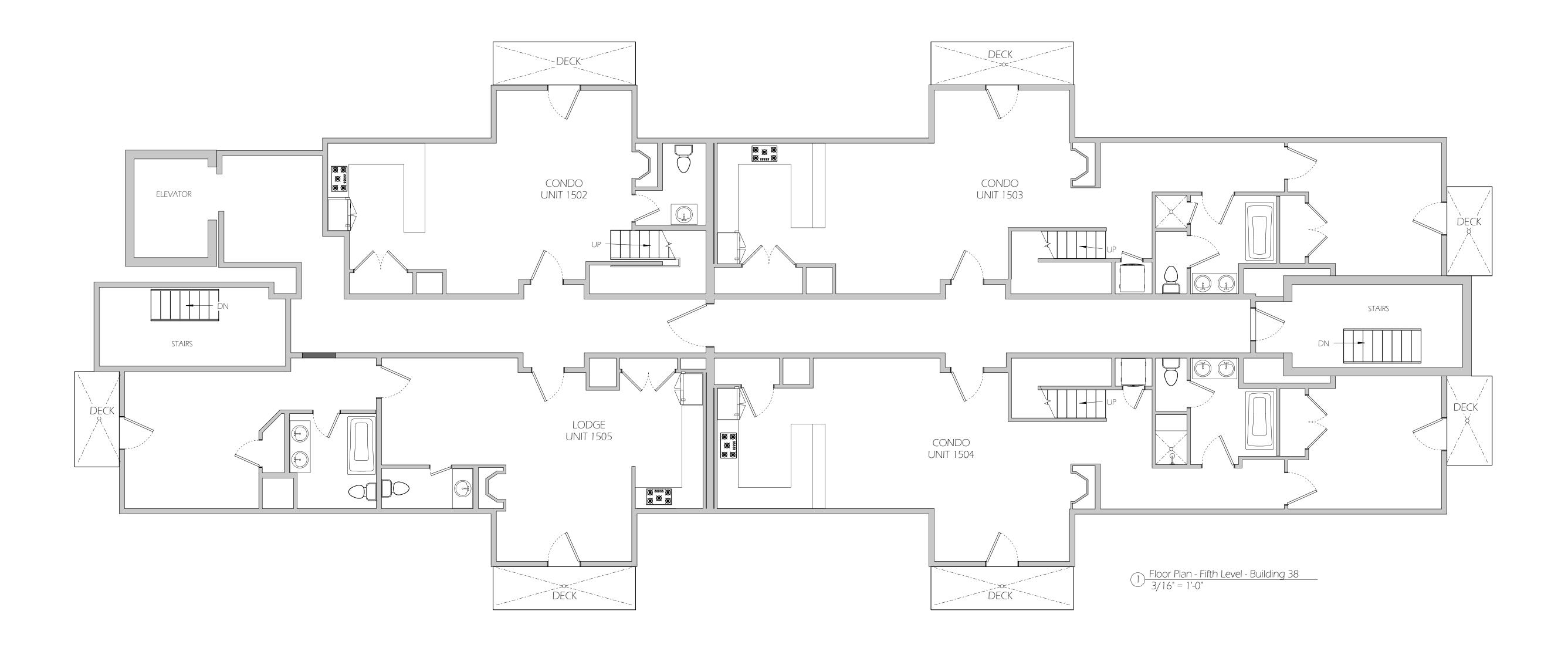


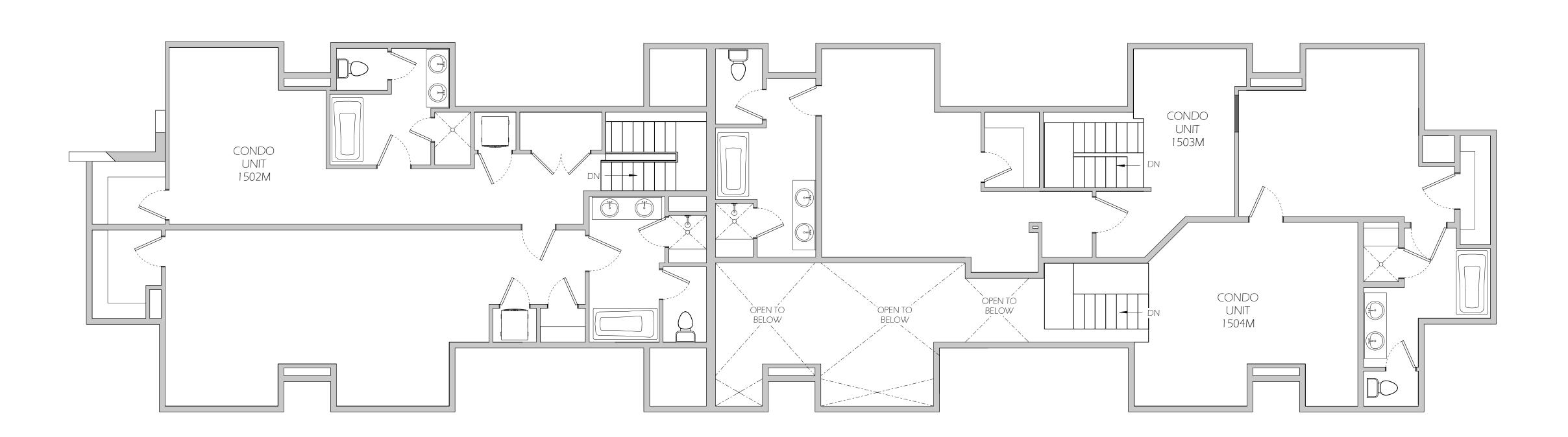






Sheet: A.P. 2.4.1





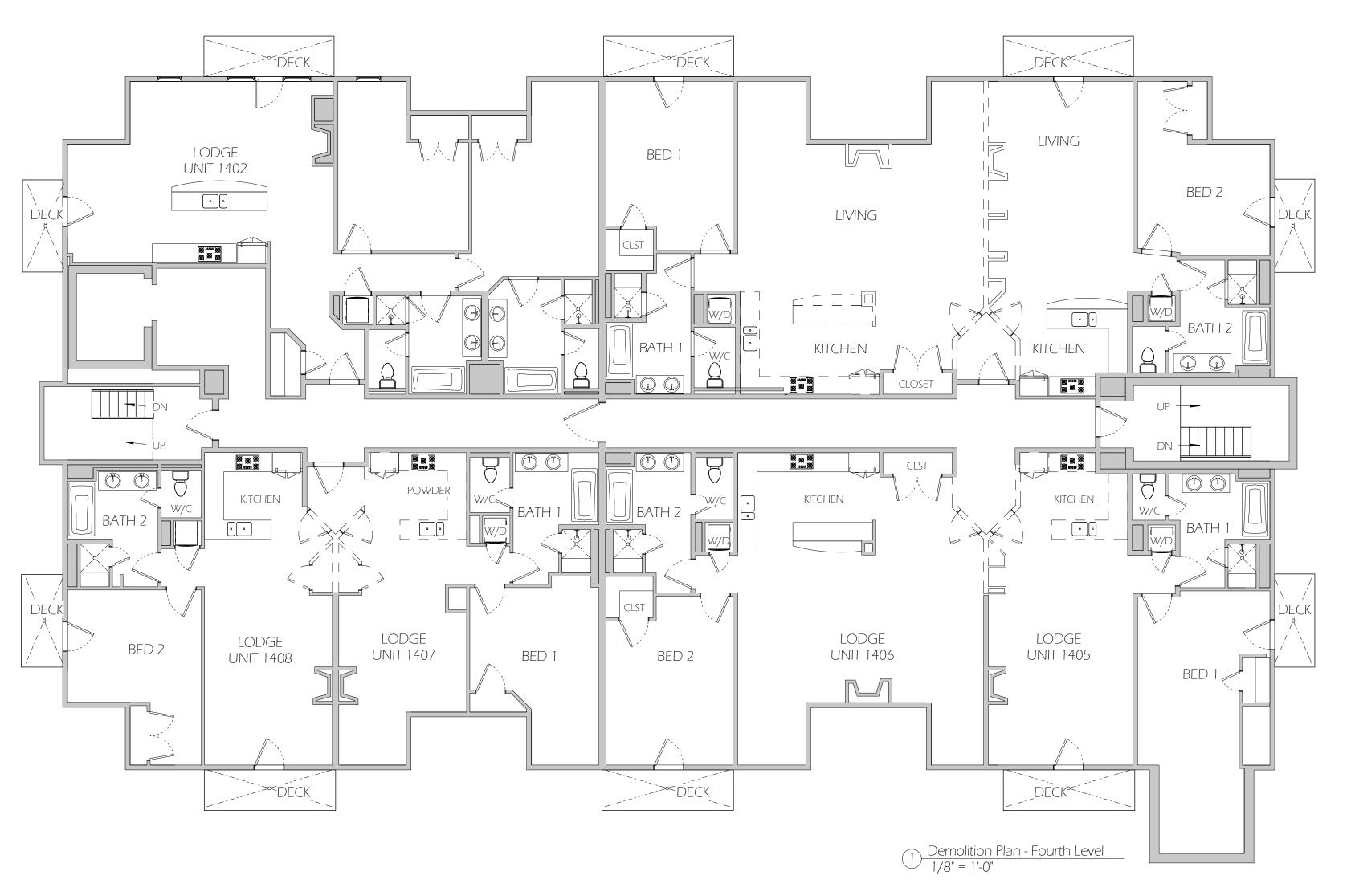
Floor Plan - Sixth Level - Building 38
3/16" = 1'-0"

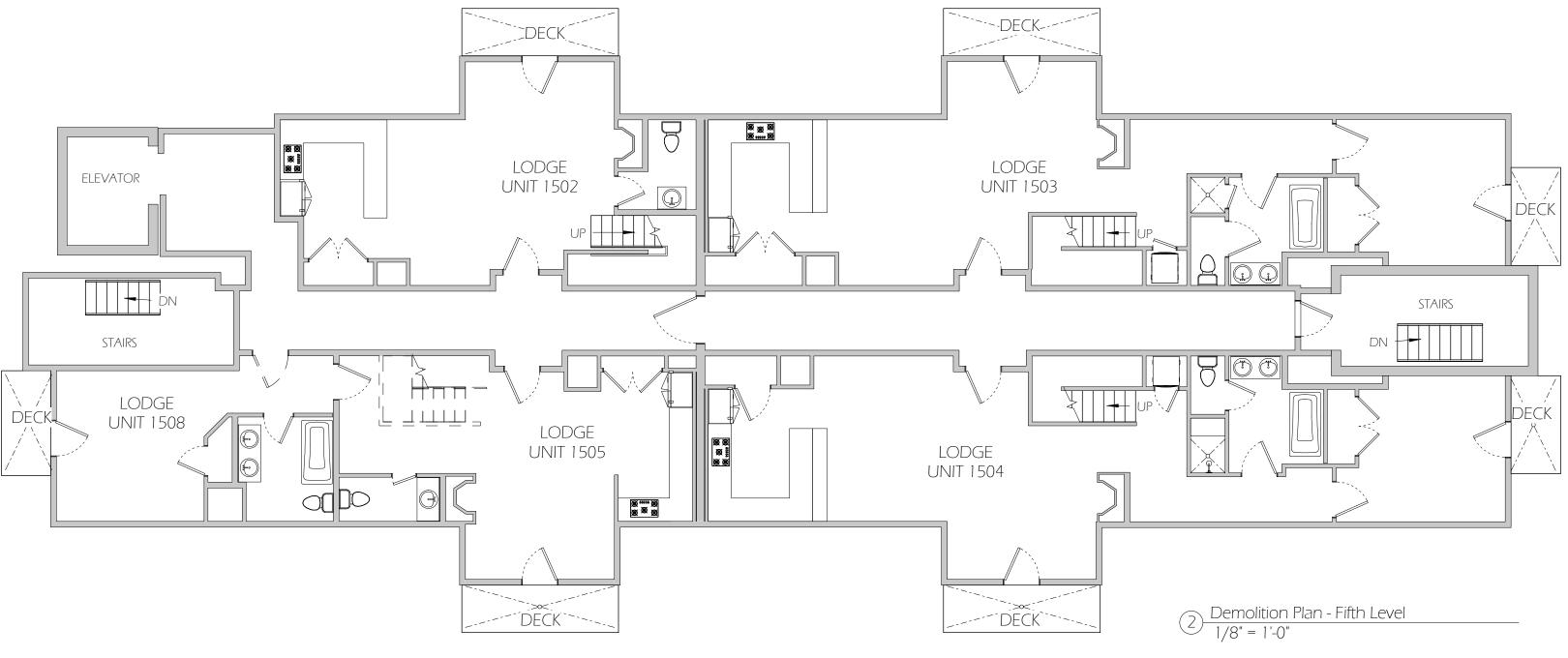
Hotel Madeline

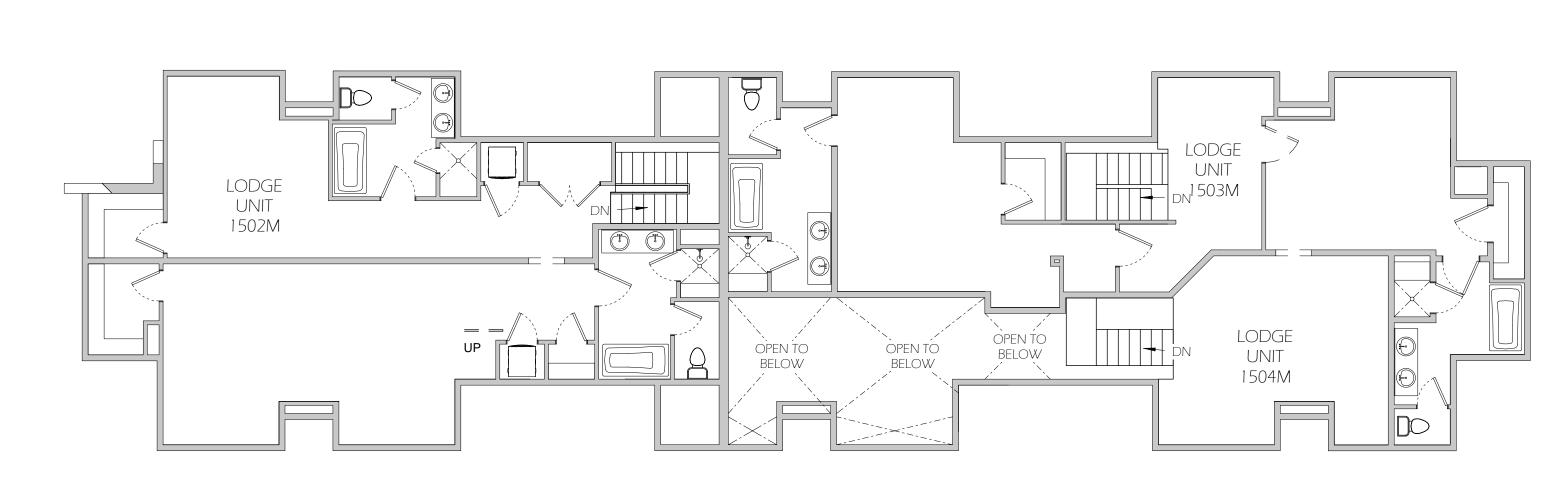
Phase 1 Remodel
Building 38 - Fifth & Sixth Level
Floor Plans

Sheet: A.P. 2.4.2









Demolition Plan - Sixth / Mezzanine Level
1/8" = 1'-0"

Hotel Madeline

Phase 1 Remodel

Building 38 - Fourth, Fifth, & Sixth Level Demolition Plans





COMMUNITY DEVELOPMENT DEPARTMENT PLANNING DIVISION

455 Mountain Village Blvd. Mountain Village, CO 81435 (970) 728-1392

TO: Design Review Board

FROM: Dave Bangert, Town Forester

FOR: DRB Public Hearing on May 7, 2015

DATE: April 15, 2015

RE: Consideration of a Minor Revisions application to allow for a landscape berm to

be constructed in the northern General Easement on Lot 355

APPLICATION OVERVIEW:

The purpose and intent of this memo is to have the Design Review Board review and act upon a Minor Revisions application to allow for the construction of a landscape berm in the northern General Easement located on Lot 355.

PROJECT GEOGRAPHY

Legal Description: Lot 355, Telluride Mountain Village, Filing 26 **Address:** 129 Rocky Road, Mountain Village, Colorado

Applicant/Agent: Tommy Hein Architects
Owner: Joel & Marie Purdom
Zoning: Single Family Residential

Existing Use: Vacant, Single Family Residential

Proposed Use: Single Family Residential

Lot Area: 1.35 acres

Adjacent Land Uses:

North: Single Family Residential
 South: Single Family Residential
 East: Single Family Residential

West: Active Open Space, Marmot Ski Trail

<u>ATTACHMENTS</u>

Exhibit A: Applicant's Site plan and Civil drawings

• Exhibit B: Applicants narrative

RECORD DOCUMENTS

- Town of Mountain Village Community Development Code as amended (CDC)
- Town of Mountain Village Home Rule Charter as amended
- Minor Revisions Application as maintained by the Community Development Department.

BACKGROUND

As part of a Minor Revisions application the applicant is proposing to construct a landscape berm to the north side of the lot to reduce the visual impact of the North Façade from the road. This berm is proposed in the northern General Easement and will require specific approval from the Design Review Board.

CRITERIA FOR DECISION

- 1. The proposed development meets the Design Regulations;
- 2. The proposed development is in compliance with the Zoning and Land Use Regulations;
- 3. The proposed development complies with the road and driveway standards;
- 4. The proposed development is in compliance with the other applicable regulations of this CDC:
- 5. The development application complies with any previous plans approved for the site still in effect:
- 6. The development application complies with any conditions imposed on development of the site through previous approvals; and
- 7. The proposed development meets all applicable Town regulations and standards.

ANALYSIS

17.3.14. E General Easement Setback

- **E.** The following development activities are permitted in the general easement setback or other setbacks subject to the applicable review process and Design Regulations:
 - 1. Review authority approved accessways for direct access, including driveways, walkways, and ski trails and ski lifts for ski area access.
 - a. Accessway impacts to the general easement shall be minimized to the extent practical, such as a perpendicular crossing of the easement setback area.
 - b. Accessways shall not exceed the minimum Town standards for construction, such as the minimum width.
 - 2. Utilities:
 - a. To the extent practical, all utilities shall follow a driveway alignment.
 - 3. Address monuments;
 - 4. Natural landscaping without any man-made materials or hardscape;
 - 5. Fire mitigation and forestry management without substantial earthwork;
 - 6. Construction staging provided:
 - a. The area proposed for such staging is devoid of naturally occurring trees or other naturally occurring vegetation; or
 - b. The DRB is approving disturbance in the general easement for another proposed improvement such as a driveway, utility cut, or skier access,

2

and the area can be used for staging until the approved improvement is constructed; and

- 7. Other uses as provided for in the definition of general easement.
- F. The DRB may waive the general easement setback or other setbacks and allow for prohibited activities provided:
 - 1. The applicant has demonstrated that avoiding grading and disturbance in the general easement setback would create a hardship, and there is not a practicable alternative that allows for reasonable use of the lot;
 - 2. The disturbance in the general easement setback is due to natural features of the site, such as steep slopes, wetlands and streams;
 - 3. No unreasonable negative impacts result to the surrounding properties;
 - 4. The general easement setback or other setback will be revegetated and landscaped in a natural state;
 - 5. The Public Works Department has approved the permanent above-grade and below-grade improvements;
 - 6. The applicant will enter into an encroachment agreement with the Town with the form and substance prescribed by the Town; and
 - 7. Encroachments into the general easement setback or other setbacks are mitigated by appropriate landscaping, buffering and other measures directly related to mitigating the encroachment impacts.

The applicant is proposing a berm be added to the north side of the lot to create a buffer to the road. This will shield occupants in the home from the users of the road as well as shield the road from any off site glare from within the residence. The berm grading will be minimized on site to save to save trees and reduce additional clearing.

Staff is in support of this application and has met on site with the architect, engineer and the Director of Public Works to work out the positioning of this berm. Public Works is in favor of this design. Staff will have a condition that the owners of Lot 355 enter into a Revocable General Easement encroachment agreement with the Town for this landscape berm in the General Easement.

RECOMMENDATION

Staff recommends the DRB approve the Minor Revisions application with the following motion:

"I move to approve a Minor Revisions application for a landscape berm in the northern General Easement on Lot 355, with the findings and conditions contained in the staff memo of record.:"

Findings:

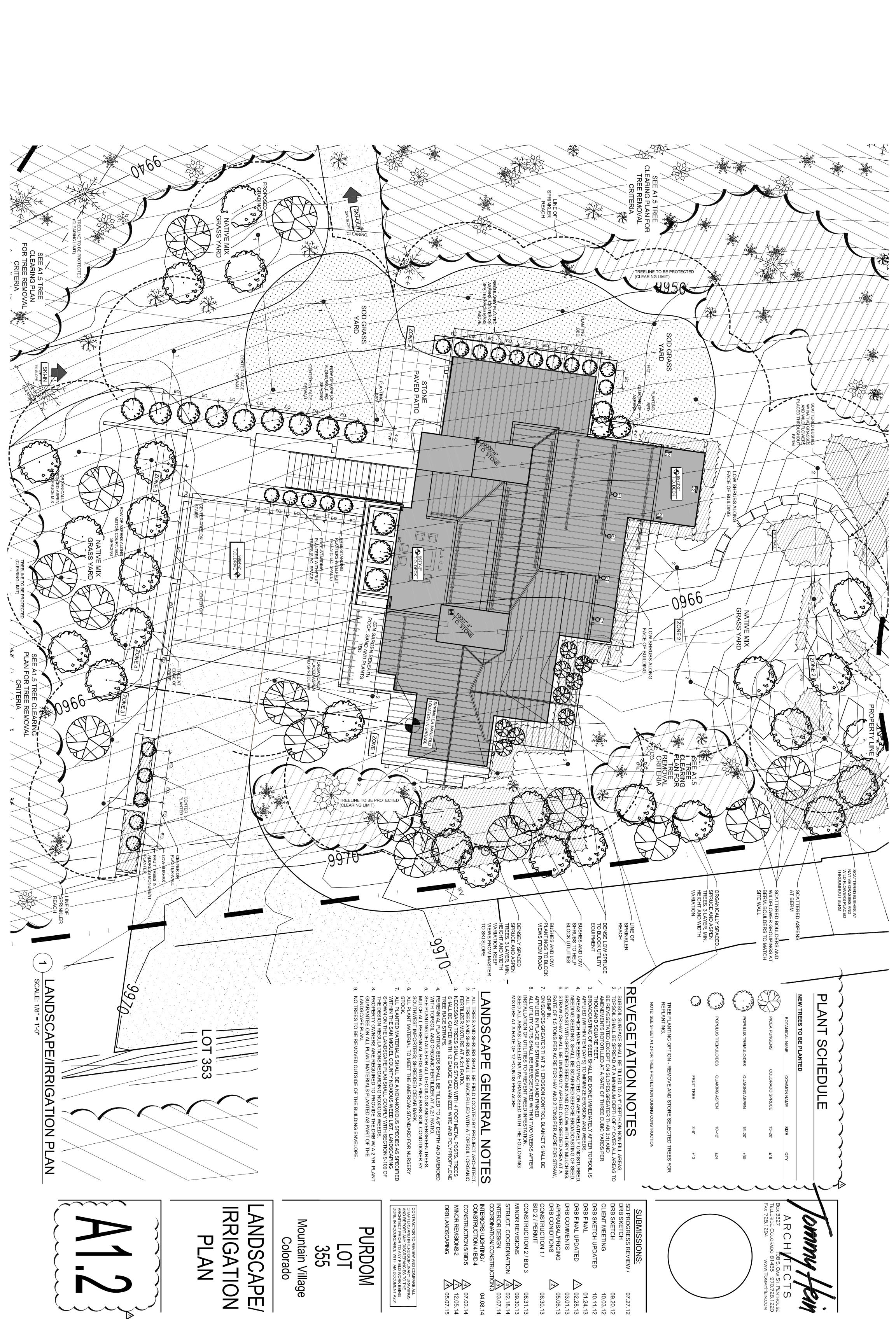
- 1. The proposed development meets the Design Regulations;
- 2. The proposed development is in compliance with the Zoning and Land Use Regulations;
- 3. The proposed grading in the general easement meets the criteria contained in CDC Section 17.3.14 as follows:

- a. The proposed landscaping berm cannot be constructed to provide the needed buffering without disturbance in the general easement.
- b. No unreasonable negative impacts result to the surrounding properties;
- c. The general easement setback or other setback will be revegetated and landscaped in a natural state;
- d. The Public Works Department has approved the permanent above-grade and below-grade improvements.
- e. The applicant will enter into an encroachment agreement with the Town with the form and substance prescribed by the Town.
- f. The landscaped berrn is mitigating visual impacts.
- 4. The disturbance in the general easement is due to the cul-du-sac turnaround adjacent to the property.
- 5. The proposed development complies with the road and driveway standards;
- 6. The proposed development is in compliance with the other applicable regulations of this CDC;
- 7. The development application complies with any previous plans approved for the site still in effect:
- 8. The development application complies with any conditions imposed on development of the site through previous approvals; and
- 9. The proposed development meets all applicable Town regulations and standards.

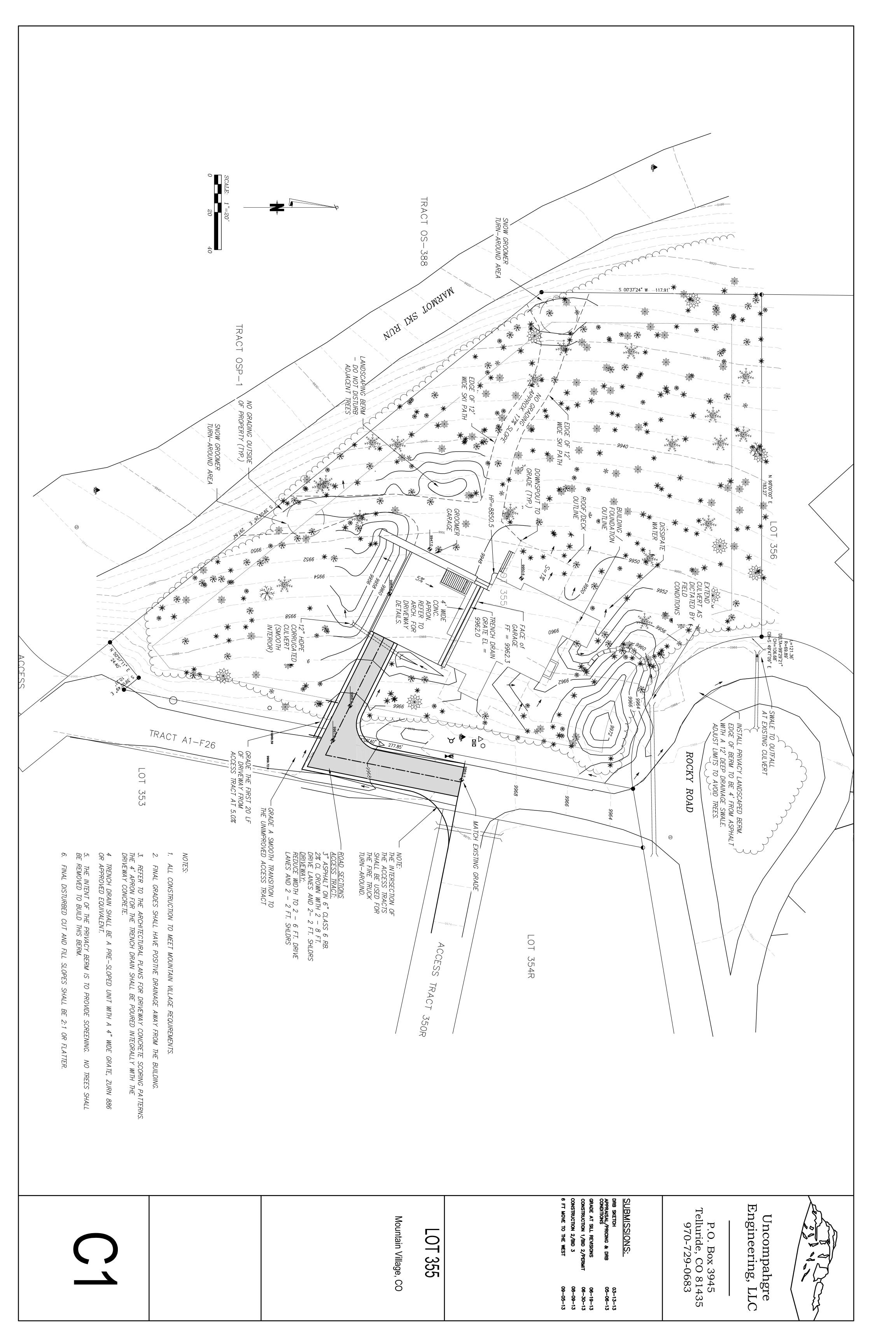
Condition:

1. The owners of Lot 355 shall enter into a Revocable General Easement encroachment agreement with the Town for the landscape berm in the General Easement prior to the issuance of a certificate of occupancy.

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355

129 Rocky Road, Mountain Village **MINOR REVISIONS TO DRB FINAL** May 15, 2015

On March 14, 2013 DRB voted to grant final approval for the design of the 355 residence with conditions. This was followed by a building permit granted on July 16, 2013.

BERM AT NORTH SIDE OF SITE

A berm has been added to the north side of the lot in order to create a buffer from an otherwise open vista to the road. This will aid, not only in shielding occupants from the users of the road, but will also work to shield the road from light flooding from within the residence. The berm grading will be minimized on site to save significant trees, and to reduce additional clearing to a minimum.

Best_ Tommy Hein Architects Skyler Bonser – Project Architect



COMMUNITY DEVELOPMENT DEPARTMENT PLANNING DIVISION

455 Mountain Village Blvd. Mountain Village, CO 81435 (970) 728-1392

TO: Design Review Board

FROM: Dave Bangert, Town Forester

FOR: DRB Public Hearing on May 7, 2015

DATE: April 21, 2015

RE: Consideration of a Design Review Process application to allow an encroachment

into the General Easement for a monument on the southern side of the driveway that matches the existing address monument and assorted landscaping on both

sides of the drive that will extend into the RROW on Lot 204.

APPLICATION OVERVIEW:

The purpose and intent of this memo is to have the Design Review Board review and act upon a Design Review Process application to allow for an encroachment into the General Easement for a monument on the southern side of the driveway that matches the existing address monument and assorted landscaping on both sides of the drive that will extend into the RROW located on Lot 204.

PROJECT GEOGRAPHY

Legal Description: Lot 204, Telluride Mountain Village, Filing 6 **Address:** 108 Stevens Drive, Mountain Village, Colorado

Applicant/Agent: Bruce Derrick **Owner:** Bruce Derrick

Zoning: Single Family Residential Existing Use: Single Family Residential Single Family Residential

Lot Area: 0.841 acres

Adjacent Land Uses:

North: Single Family Residential
 South: Single Family Residential
 East: Single Family Residential
 West: Active Open Space

ATTACHMENTS

• Exhibit A: Applicant's Site plan and photos

• Exhibit B: Applicants narrative

RECORD DOCUMENTS

- Town of Mountain Village Community Development Code as amended (CDC)
- Town of Mountain Village Home Rule Charter as amended
- Design Review Process Application as maintained by the Community Development Department.

BACKGROUND

As part of a Design Review Process application the applicant is proposing to construct a monument on the southern side of the driveway that matches the existing address monument and assorted landscaping on both sides of the drive that will extend into the RROW on Lot 204. This monument and landscaping is proposed in the southern General Easement and will require specific approval from the Design Review Board. The proposed Road Right of Way encroachment will have to be approved by the Mountain Village Town Council.

CRITERIA FOR DECISION

- 1. The proposed development meets the Design Regulations;
- 2. The proposed development is in compliance with the Zoning and Land Use Regulations;
- 3. The proposed development complies with the road and driveway standards;
- 4. The proposed development is in compliance with the other applicable regulations of this CDC:
- 5. The development application complies with any previous plans approved for the site still in effect:
- 6. The development application complies with any conditions imposed on development of the site through previous approvals; and
- 7. The proposed development meets all applicable Town regulations and standards.

ANALYSIS

17.3.14. E General Easement Setback

- **E.** The following development activities are permitted in the general easement setback or other setbacks subject to the applicable review process and Design Regulations:
 - **1.** Review authority approved accessways for direct access, including driveways, walkways, and ski trails and ski lifts for ski area access.
 - a. Accessway impacts to the general easement shall be minimized to the extent practical, such as a perpendicular crossing of the easement setback area.
 - b. Accessways shall not exceed the minimum Town standards for construction, such as the minimum width.
 - 2. Utilities:
 - a. To the extent practical, all utilities shall follow a driveway alignment.
 - 3. Address monuments;
 - **4.** Natural landscaping without any man-made materials or hardscape;
 - **5.** Fire mitigation and forestry management without substantial earthwork;
 - **6.** Construction staging provided:
 - a. The area proposed for such staging is devoid of naturally occurring trees or other naturally occurring vegetation; or
 - b. The DRB is approving disturbance in the general easement for another proposed improvement such as a driveway, utility cut, or skier access, and the area can be used for staging until the approved improvement is constructed; and

- **7.** Other uses as provided for in the definition of general easement.
- F. The DRB may waive the general easement setback or other setbacks and allow for prohibited activities provided:
 - 1. The applicant has demonstrated that avoiding grading and disturbance in the general easement setback would create a hardship, and there is not a practicable alternative that allows for reasonable use of the lot;
 - 2. The disturbance in the general easement setback is due to natural features of the site, such as steep slopes, wetlands and streams;
 - 3. No unreasonable negative impacts result to the surrounding properties;
 - 4. The general easement setback or other setback will be revegetated and landscaped in a natural state;
 - 5. The Public Works Department has approved the permanent above-grade and below-grade improvements;
 - 6. The applicant will enter into an encroachment agreement with the Town with the form and substance prescribed by the Town; and
 - 7. Encroachments into the general easement setback or other setbacks are mitigated by appropriate landscaping, buffering and other measures directly related to mitigating the encroachment impacts.

The applicant's narrative states that the proposed encroachments and new monument will provide visually appealing symmetry to the drive entrance and mirror the existing boulder wall on the north side of the drive. The applicant believes the proposed encroachment will not impose on his neighbors to the south as the new monument will not be visible from their property. The Director of Public Works has no issues with the proposed encroachments into the General Easement and RROW.

Staff is in support of this application and has met on site with the owner to go over the layout and design of the project. Staff recommends that any DRB approval include a condition that the owners of Lot 204 enter into a Revocable General Easement encroachment agreement with the Town for this new monument and landscaping in the General Easement.

RECOMMENDATION

Staff recommends the DRB approve this Design Review Process application with the following motion:

"I move to approve a Design Review Process application for a monument and landscaping in the southern General Easement on Lot 204, with the findings and conditions contained in the staff memo of record."

Findings:

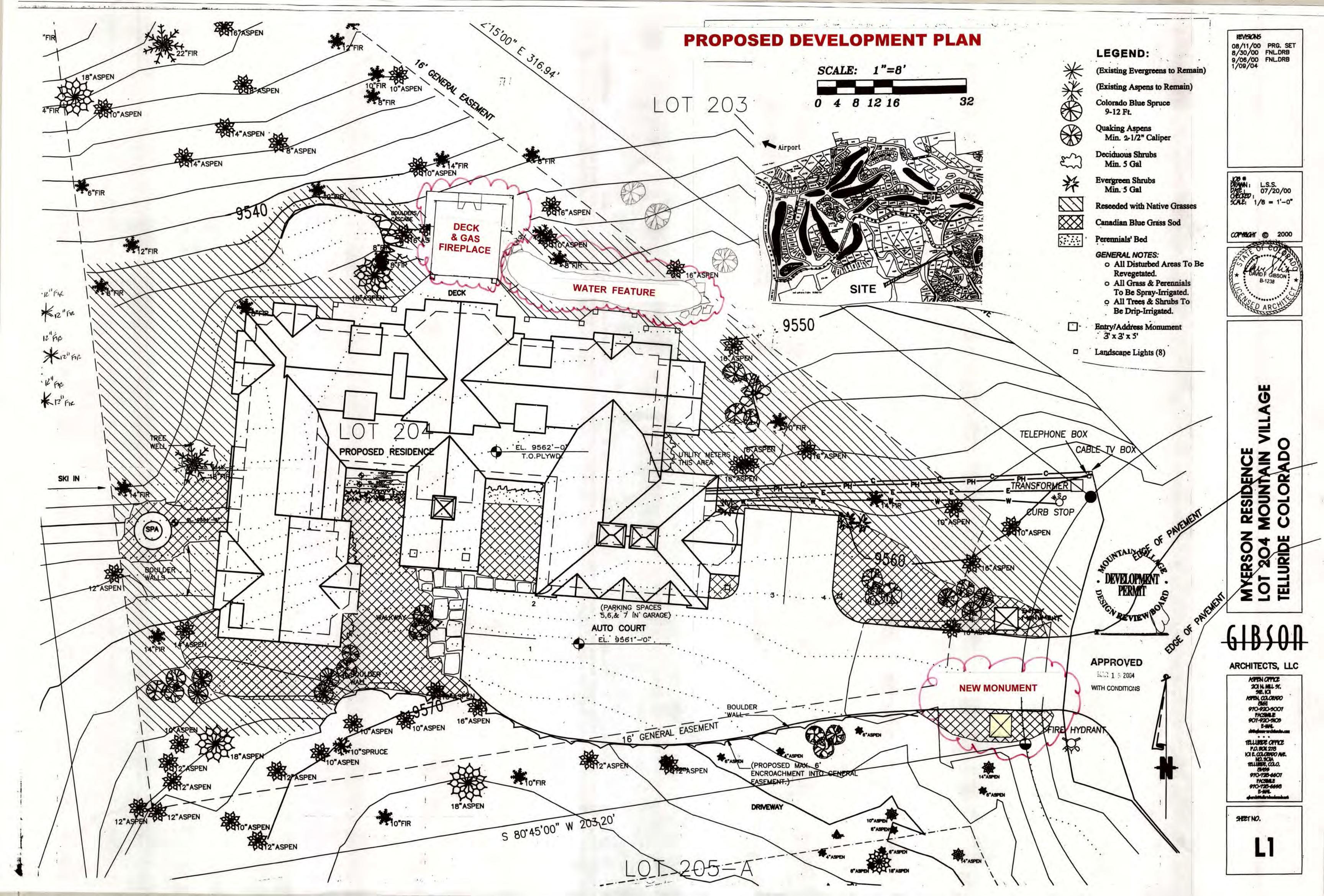
- 1. The proposed development meets the Design Regulations;
- 2. The proposed development is in compliance with the Zoning and Land Use Regulations:
- 3. The proposed grading in the general easement meets the criteria contained in CDC Section 17.3.14 as follows:

- The proposed landscaping and monument cannot be constructed to provide the needed buffering without disturbance in the general easement.
- b. No unreasonable negative impacts result to the surrounding properties;
- c. The general easement setback or other setback will be revegetated and landscaped in a natural state;
- d. The Public Works Department has approved the permanent above-grade and below-grade improvements.
- e. The applicant will enter into an encroachment agreement with the Town with the form and substance prescribed by the Town.
- 4. The proposed development complies with the road and driveway standards;
- 5. The proposed development is in compliance with the other applicable regulations of this CDC;
- 6. The development application complies with any previous plans approved for the site still in effect;
- 7. The development application complies with any conditions imposed on development of the site through previous approvals; and
- 8. The proposed development meets all applicable Town regulations and standards.

Conditions:

- The owners of Lot 204 will seek approval from the Mountain Village Town Council for the RROW encroachment at the May 21, 2015 meeting
- 2. The owners of Lot 204 shall enter into a Revocable General Easement/RROW encroachment agreement with the Town for the landscaping in the General Easement prior to the issuance of a certificate of occupancy.

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April 24, 2015

Mr. Dave Bangert
Associate Planner/Forrester
Town of Mountain Village
455 Mountain Village Blvd, Suite A
Mountain Village, CO 81435

Re: DRP Application for 108 Stevens Drive

Dear Dave:

In connection with the above application I have submitted, I am writing to offer a few thoughts on my request for the additional address monument.

While the placement of the address monument will involve a minor encroachment into the General Easement and the RROW, I feel strongly it is warranted for several reasons:

- -Though I would really prefer gates, TMV does not allow those. Aesthetically, as evidenced by the photo super-imposing the monument, the new monument will provide visually appealing symmetry to the drive entrance.
- -It will also allow me to mirror the existing boulder wall (on the north side of the drive) which will further beautify the drive entrance.
- -Further, I believe this will not impose on my neighbors to the south (the Perez's 112), since the berm along our common P/L and GE is so high this monument will not be visible from their property.

Regards,

Bruce W. Derrick

Cc: file



COMMUNITY DEVELOPMENT DEPARTMENT PLANNING DIVISION

455 Mountain Village Blvd. Mountain Village, CO 81435 (970) 728-1392

TO: Design Review Board

FROM: Savannah Jameson, Planner II

FOR: DRB Public Hearing on May 7, 2015

DATE: April 17, 2015

RE: Consideration of a Design Review Process Development Application for Lot 1001

and Tract OS-1R1 Roof and Fence Variation

PROJECT GEOGRAPHY

Legal Description: Lot 1001 and Tract OS-1R1 Mountain Village Filing No. 1 455 Mountain Village Blvd (Village Court Apartments)

Applicant/Agent: Steven Spencer

Owner: Town of Mountain Village

Zoning: Multi-Family, Ski Resort Active Open Space

Existing Use: Multi-Family Use/Community Garden, Dog Park and Recreation Area

Proposed Use: No change in use

Adjacent Land Uses:

North: Open Space

South: Open Space and Civic

East: Open SpaceWest: Open Space

Lot Size: 8.394

PROJECT SUMMARY

| CDC Provision | Requirement | Proposed |
|---------------------------|-------------------------------------|----------|
| Maximum Building Height | 40' maximum (35'+5' for gable roof) | 15.6" |
| General Easement Setbacks | | |
| North | 16' | 23' |
| South | 16' | 19' |
| East | 16' | 22' |
| West | 16' | 145' |
| Roof Pitch | | |
| Primary | 6:12 to 12:12 | 3:12 |
| Exterior Material | | |
| Hardy Board Panel | Specific Approval | 75% |
| Rusted Corrugated Metal | Specific Approval | 25% |
| Fencing Material | Variance | |

ATTACHMENTS

Exhibit A: Applicant NarrativeExhibit B: Design Review Plans

RECORD DOCUMENTS

- Town of Mountain Village Community Development Code as amended (CDC)
- Town of Mountain Village Home Rule Charter as amended
- Design Review Application as maintained by the Community Development Department

BACKGROUND

The proposal is for the construction of a community building/clubhouse, for use as an amenity and recreation area at Village Court Apartments (VCA). The property is located on parcel 1001 zoned Multi-Family and Tract OS-1R1, Ski Resort Active Open Space. A conditional use permit for a community garden, dog park, and recreation area on Tract OS-1R1 was approved by Town Council at a public meeting on February 20, 2014. In accordance with the Town of Mountain Village Design Regulations, the Towns' applicant, Steven Spencer, has applied for a Variance to Mountain Village Design Regulations ("Design Regulations") Section17.5.9(D)(2)(d) Fence Materials and Section 17.5.6(2)(a).Roof Pitch).

As part of a plan to improve safety and offset maintenance costs, the applicant is requesting to construct a composite fence. Composite fencing is designed to mirror the look and feel of natural wood grain without splintering, which is an important consideration for a recreation area. Plus composite fencing is an extremely strong, attractive and environmentally friendly choice for fencing that offers long-term durability and low maintenance benefits. Mountain Munchkins had a composite fence approved and constructed in 2010. This fence would match in style the Mountain Munchkins fence.

The proposed roof pitch at 3:12 is compatible with and emulates the adjacent storage building roof. The low pitch is appropriate for this small ancillary building.

CRITERIA FOR DECISION

- 1. The proposed development meets the Design Regulations;
- 2. The proposed development is in compliance with the Zoning and Land Use Regulations;
- 3. The proposed development complies with the road and driveway standards:
- 4. The proposed development is in compliance with the other applicable regulations of this CDC:
- The development application complies with any previous plans approved for the site still in effect:
- 6. The development application complies with any conditions imposed on development of the site through previous approvals; and
- 7. The proposed development meets all applicable Town regulations and standards.

ANALYSIS

The proposed addition complies with the Design Regulations and the Design Review Process as outlined in the findings set forth in the attached resolution. The following are the outstanding matters that have to be corrected or addressed:

Design Variations

The applicant is seeking the following two design variations pursuant to CDC Section 17.4.16:

- 1. Use of composite fencing material in lieu of Stone, stucco, metal or wood as outlined in CDC Section17.5.9(D)(2)(d); and
- 2. A roof pitch of 3:12 in lieu of 6:12-12:12 as outlined in CDC Section 17.5.6(2)(a).

Section 17.4.16(D) states that the following criteria shall be met for the review authority to approve a design variation development:

- 1. The design variation is compatible with the design context of the surrounding area, and provides for a strong mountain vernacular design.
- 2. The design variation is consistent with the town design theme;
- 3. The strict development application of the Design Regulations(s) would prevent the applicant or owner from achieving its intended design objectives for a project;
- 4. The design variation is the minimum necessary to allow for the achievement of the intended design objectives;
- 5. The design variation is consistent with purpose and intent of the Design Regulations;
- 6. The design variation does not have an unreasonable negative impact on the surrounding neighborhood; and
- 7. The proposed design variation meets all applicable Town regulations and standards.

Cost or inconvenience alone shall not be sufficient grounds to grant a design variation.

Other CDC Considerations

CDC Section 17.5.9(D)(2)(d) Walls, fences and gates shall be constructed from stone, stucco, metal or wood to meet the town design theme, and shall require the specific approval of the review authority. While wood is a common fencing material, the composite fencing emulates the look and feel of wood, which is consistent to the intent of the Design Regulations. The composite fencing will be compatible with the Mountain Munchkins fence.

CDC Section 17.5.6(2)(a) Primary roof pitches shall be 6:12 to 12:12 except for: Town shops, recreational facilities, community facilities, public works buildings or buildings with an industrial zoning designation on the site may have lesser slopes approved as a specific approval of the DRB. The proposed roof at 3:12 is compatible with and emulates the adjacent storage building roof. The low pitch is appropriate for this small ancillary building.

Exterior Wall Materials

The north, south, and west facades proposes the exterior materials to be 75% 4" hardy board and 25% rusted corrugated metal; sustainable building materials which require specific DRB approval. Staff is recommending approval of the sustainable building materials

RECOMMENDATION

Staff recommends the DRB approve the Design Review Process development application with the following motion:

"I move to approve a resolution for a Design Review Process development application for the construction of the clubhouse on Lot 1001 and Tract OS-1R1, with the findings and conditions as set forth in the resolution."

RESOLUTION OF THE DESIGN REVIEW BOARD OF MOUNTAIN VILLAGE, COLORADO, AUTHORIZING AND APPROVING THE DESIGN REVIEW PROCESS DEVELOPMENT APPLICATION FOR A ROOF AND FENCE VARIATION ON LOT 1001 AND TRACT OS-1R1

Resolution No. 2015-0507-06

RECITALS:

- A. The Town of Mountain Village and TSG Ski and Golf, LLC ("Owner") are the owners of certain real property described as Lot 1001 and Tract OS-1R1, Mountain Village Filing No. 1.
- B. The Town of Mountain Village has submitted a Class 2 Design Review Process application requesting approval for a roof and fence variation on Lot 1001 and Tract OS-1R1. ("Application").
- C. The Design Review Board (DRB) considered this application, along with evidence and testimony, at a public meeting held on May 7, 2015. Upon concluding their review, the DRB voted_____to approve the Application.
- D. The DRB considered the Application submittal materials, all other relevant materials, public letters and public testimony, and approved the Application with conditions as set forth in this Resolution.
- E. The Owners have addressed, or agreed to address, all conditions of approval of the Application imposed by the DRB.
- F. DRB based their approval of this Application on the following findings, as stated required by section CDC Section 17.4.11(D):
 - 1. With compliance of the conditions set forth below, the proposed development meets the Design Regulations because, without limitation, the development is compliant with the Town design theme, building siting design requirements, building design requirements,
 - 2. The DRB finds that the variations to use composite fencing material in lieu of stone, stucco, metal or wood as outlined in CDC Section17.5.9(D)(2)(d) and a roof with a 3:12 pitch in lieu of 6:12-12:12 as outline in CDC Section 17.5.6(2)(a) meet the criteria for approving variations as set forth Section 17.4.16(D).
 - a. The design variation is compatible with the design context of the surrounding area, and provides for a strong mountain vernacular design;
 - b. The design variation is consistent with the town design theme;
 - c. The strict development application of the Design Regulations(s) would prevent the applicant or owner from achieving its intended design objectives for a project;
 - d. The design variation is the minimum necessary to allow for the achievement of the intended design objectives;
 - e. The design variation is consistent with purpose and intent of the Design Regulations;
 - f. The design variation does not have an unreasonable negative impact on the surrounding neighborhood; and
 - g. The proposed design variation meets all applicable Town regulations and standards.

Now, Therefore, Be It Resolved that the DRB hereby approves the Application and authorizes the DRB Chairman to sign the Resolution subject to the conditions in Section 1.

Section 1. Development Application Conditions

- 1. The development shall comply with the following required surveys and inspections as set forth in CDC Section 17.5.5(J):
 - **A.** The Planning Division shall conduct site inspections to ensure the development is proceeding in accordance with the approved plans.
 - **B.** Prior to the issuance of either a certificate of occupancy or a temporary certificate of occupancy, the Planning Division shall inspect the site to ensure the development is constructed in accordance with the approved plans, including but not limited to all exterior materials, windows, exterior lighting, landscaping, drainage and massing.
- 2. All representations of the applicant, whether within the submittal or at the DRB hearing, are conditions of this approval.

Section 2. Effective Date and Length of Validity

- This approval shall be effective seven (7) calendar days from the date of the DRB approval, on May 14, 2015 unless an appeal is filed in accordance with the CDC appeal procedures. If an appeal is filed pursuant to the appeal procedures, building permits or other development permits shall not be issued until the appeal is heard by the Town Council and it takes action to uphold or modify the approval.
- 2. This approval shall be valid for eighteen (18) months from the effective date of approval and shall lapse on November 7, 2016 unless a Renewal Process development application is approved by the Town pursuant to the CDC.

Section 3. Void Approval

A resolution or subsequent approval issued by the Town in error or which does not comply with the provisions of this CDC or Town-adopted codes, ordinances and regulations is null and void. A permit, certificate or license issued in reliance upon any materially false statement in the development application, supporting documents or oral statements made on the record shall be null and void.

Be It Further Resolved that the Application may be developed as submitted in accordance with Resolution No. 2015-0507-06

Approved by the Design Review Board at a public meeting May 7, 2015.

| | Town of Mountain Village, Design Review | Board |
|--------------------------------|---|-------|
| | By: | |
| | Bill Hoins, Chairman | |
| Attest: | | |
| | | |
| Ву: | | |
| Chris Hawkins, Director of Con | | |



DESIGN REVIEW PROCESS APPLICATION

Community Development Department Planning Division

455 Mountain Village Blvd. Mountain Village, CO 81435 (970) 728-1392

| | DESIGN REVIEW | V PRO | CESS APPLICATION | | | |
|---------------------------|---|--------|------------------|----------------|--|--|
| | APPLICA | NT INF | ORMATION | | | |
| Name: | | | E-mail Address: | | | |
| Mailing Address: | | | Phone: | | | |
| City: | | State | : | Zip Code: | | |
| Mountain Village Business | License Number: | | | | | |
| | PROPER | TY INF | ORMATION | | | |
| Physical Address: | | | Acreage: | | | |
| Zone District: | Zoning Designations: Density Assigned to the Lot or Site: | | | e Lot or Site: | | |
| Legal Description: | | | I | | | |
| Existing Land Uses: | | | | | | |
| Proposed Land Uses: | | | | | | |
| | OWNE | R INFO | RMATION | | | |
| Property Owner: | | | E-mail Address: | | | |
| Mailing Address: | | | Phone: | | | |
| City: | | State | : | Zip Code: | | |
| | DESCRIP | TION (| OF REQUEST | | | |
| | | | | | | |
| | | | | | | |
| | | | | | | |

DEVELOPMENT NARRATIVE

Project Location: Lot 1001 and Lot OS-1R1, Village Court Apartments, Town of

Mountain Village

Project Narrative:

The proposal is for the construction of a clubhouse, for use as a recreation area at Village Court Apartments (VCA). The property is located on parcel 1001 zoned Multi-Family and Tract OS-1R1, Ski Resort Active Open Space. A conditional use permit for a community garden, dog park, and recreation area on Tract OS-1R1 was approved by Town Council at a public meeting on February 20, 2014.

The clubhouse will be a one story 500 square foot building with a 20'X25' deck. The exterior materials will be75% Hardy Board and 25% Rusted Corrugate Metal. A composite fence 3' in height will be located along the adjacent sidewalk, allowing for a recreational area to be located at the back of the clubhouse.

In accordance with the Town of Mountain Village Design Regulations, the Towns' applicant, Steven Spencer, has applied for two variances to Mountain Village Design Regulations; Section 17.5.9.D.2(d)(Fence Materials) and Section 17.5.6.A.2(a)(Roof Pitch).

Criteria for Decision

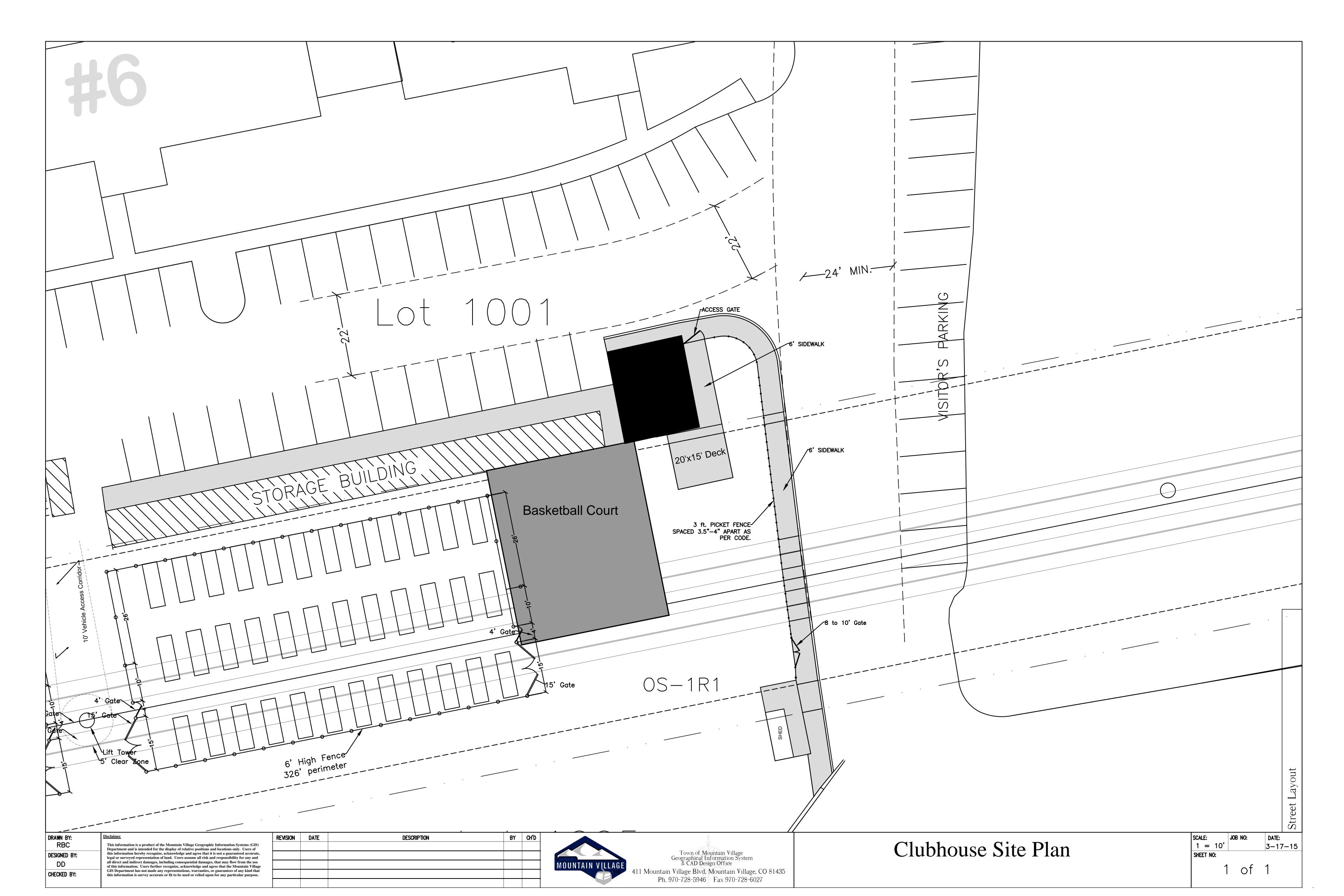
17.5.9.D.2(d) Walls, fences and gates shall be constructed from stone, stucco, metal or wood to meet the town design theme, and shall require the specific approval of the review authority.

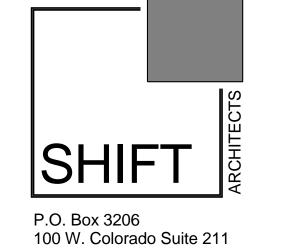
While wood is a common fencing material, the composite fencing emulates the look and feel of wood, which is consistent to the intent of the Design Regulations. The composite fencing will be compatible with the Mountain Munchkins fence.

17.5.6.A.2(a) Primary roof pitches shall be 6:12 to 12:12 except for: Town shops, recreational facilities, community facilities, public works buildings or buildings with an industrial zoning designation on the site may have lesser slopes approved as a specific approval of the DRB.

The proposed roof at 3:12 is compatible with and emulates the adjacent storage building roof. The low pitch is appropriate for this small ancillary building.







P.O. Box 3206 100 W. Colorado Suite 211 Telluride, Colorado 81435 p 970-728-8145 kristine@shift-architects.com www.shift-architects.com

DATE: 2015.03.11

RETAINING WALL 3' - 0" 0' - 9" 10' - 6" HOT WATER HEATER 3' - 5 3/4" 2' - 0" 6' - 0 1/4"

2 Level 1 1/4" = 1'-0"

| EXISTING SHED | |
|---------------|--|
| 3"/1'-0" | |
| | |
| | |

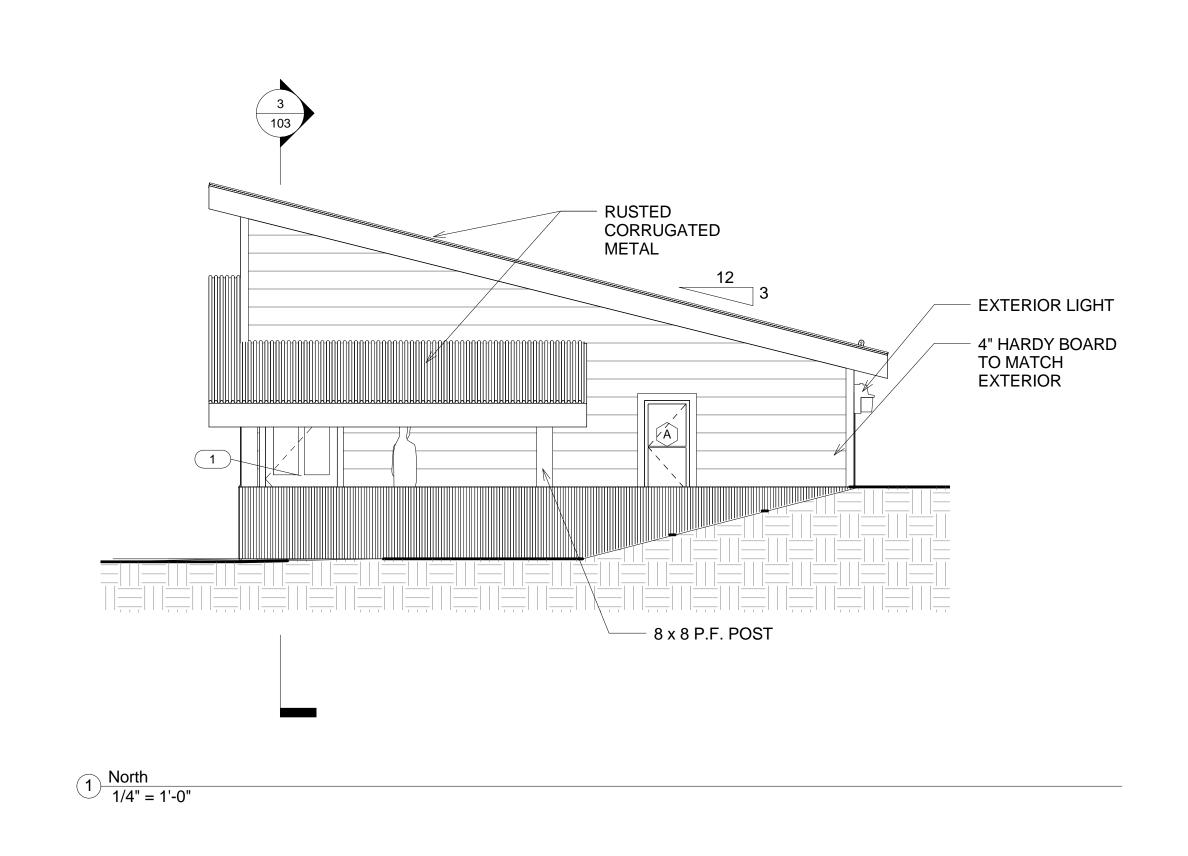
| | | | | Door Sched | ule | | | |
|-----------|-------|------------------|-------|------------|---------|----------|-------------|--------------------|
| Type Mark | Count | Manufactur er | Model | Width | Height | Comments | Description | Family |
| 1 | 1 | | | 3' - 0" | 6' - 8" | | | Single-Glas |
| 2 | 1 | | | 3' - 0" | 6' - 8" | | | Single-Flus h |
| 1 | 1 | | | 3' - 0" | 6' - 8" | | | Single-Glas s 3 |
| 6 | 1 | | | 6' - 0" | 7' - 0" | | | Sliding-2 panel |

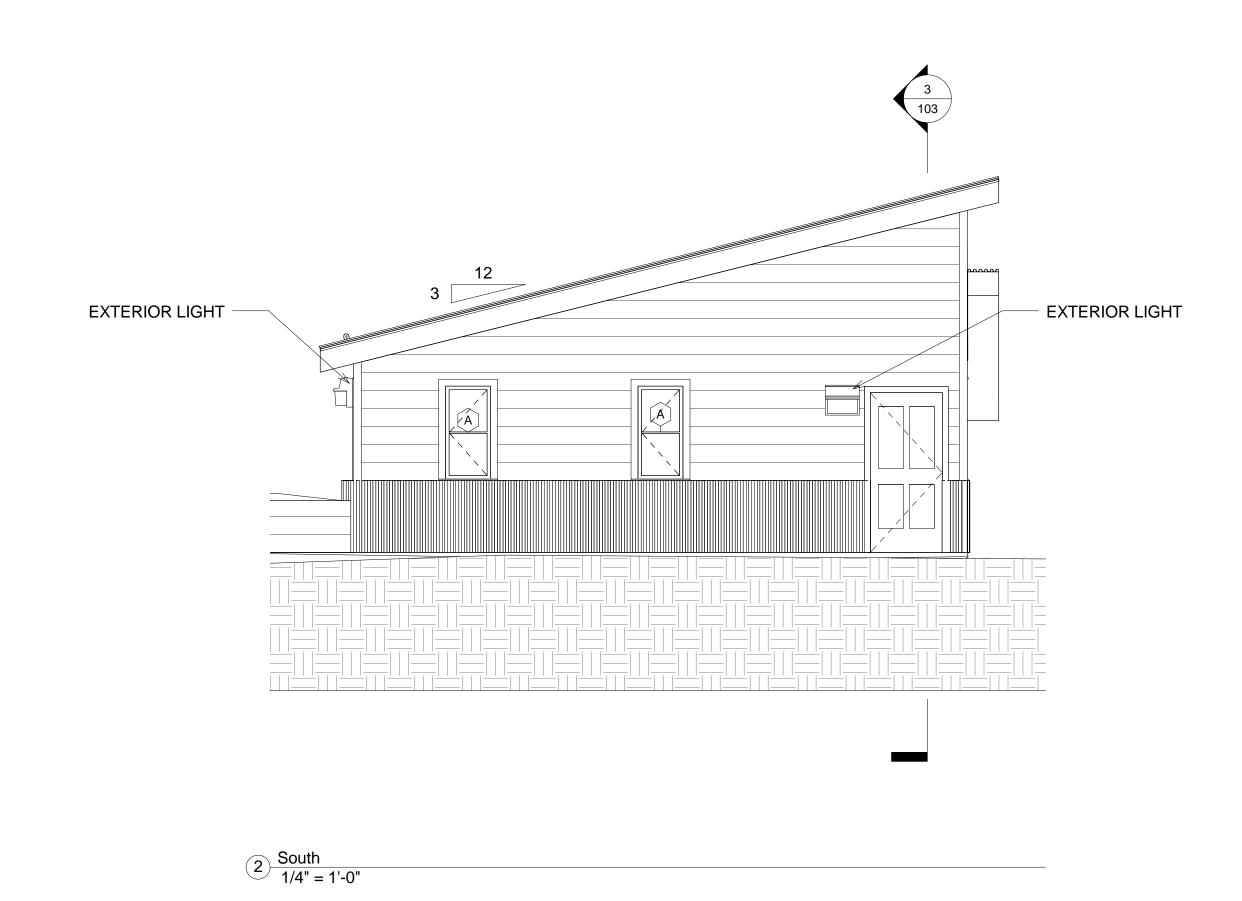
1 Roof & SITE 1/8" = 1'-0"

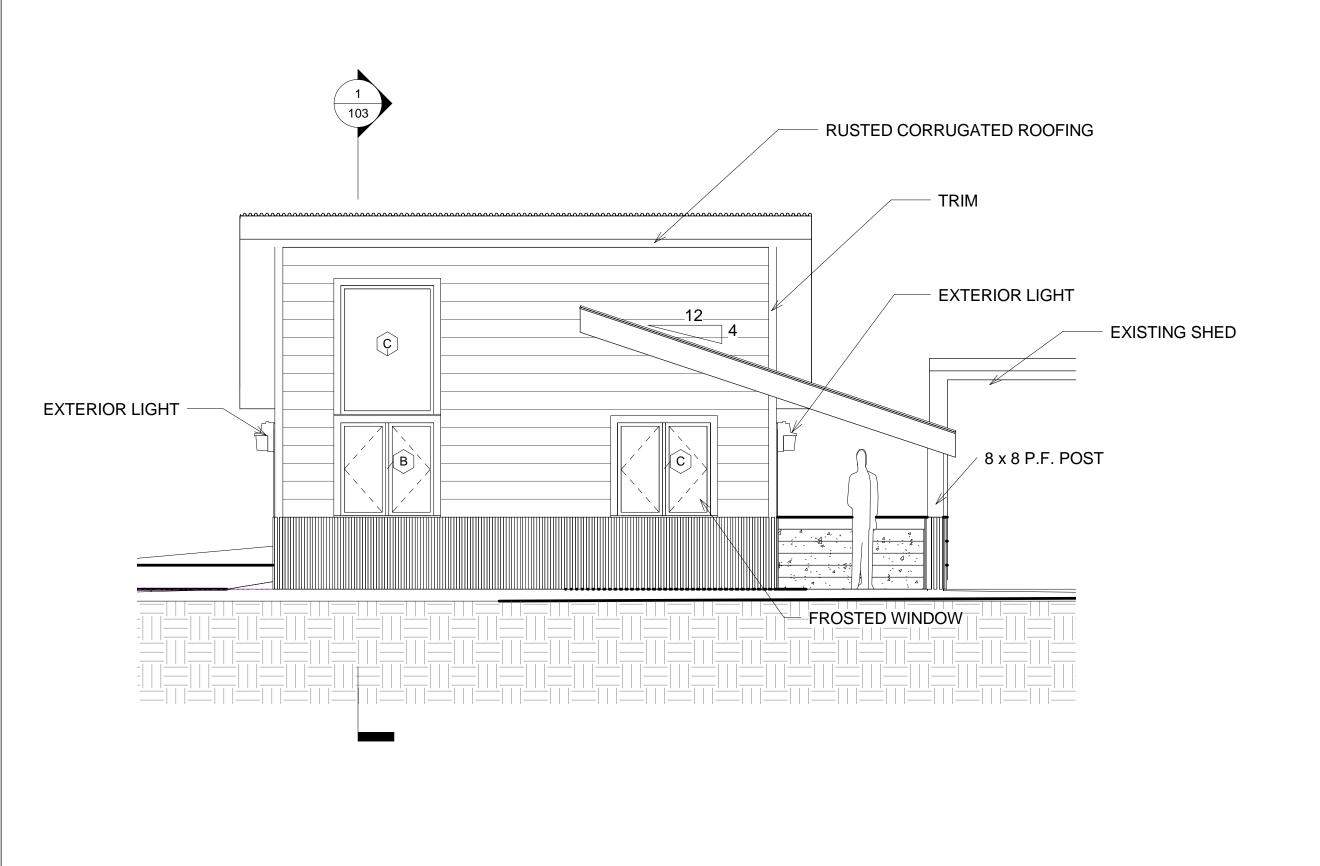
| | | | Win | dow Schedule | | | | |
|-----------|-------|---|-------------|------------------|-------|---------|---------|----------|
| Type Mark | Count | Family | Description | Manufactur er | Model | Height | Width | Comments |
| A | 1 | Casement 2 Divided with Trim.0001 | | | | 4' - 0" | 2' - 0" | |
| A | 1 | Casement 2 Divided with Trim.0001 | | | | 4' - 0" | 2' - 0" | |
| В | 1 | Casement Dbl with Trim | | | | 4' - 0" | 4' - 0" | |
| A | 1 | Casement 2 Divided with Trim.0001 | | | | 4' - 0" | 2' - 0" | |
| В | 1 | Casement Dbl with Trim | | | | 4' - 0" | 4' - 0" | |
| С | 1 | Fixed with Trim | | | | 5' - 6" | 4' - 0" | |
| С | 1 | Casement Dbl with Trim | | | | 4' - 0" | 4' - 0" | |

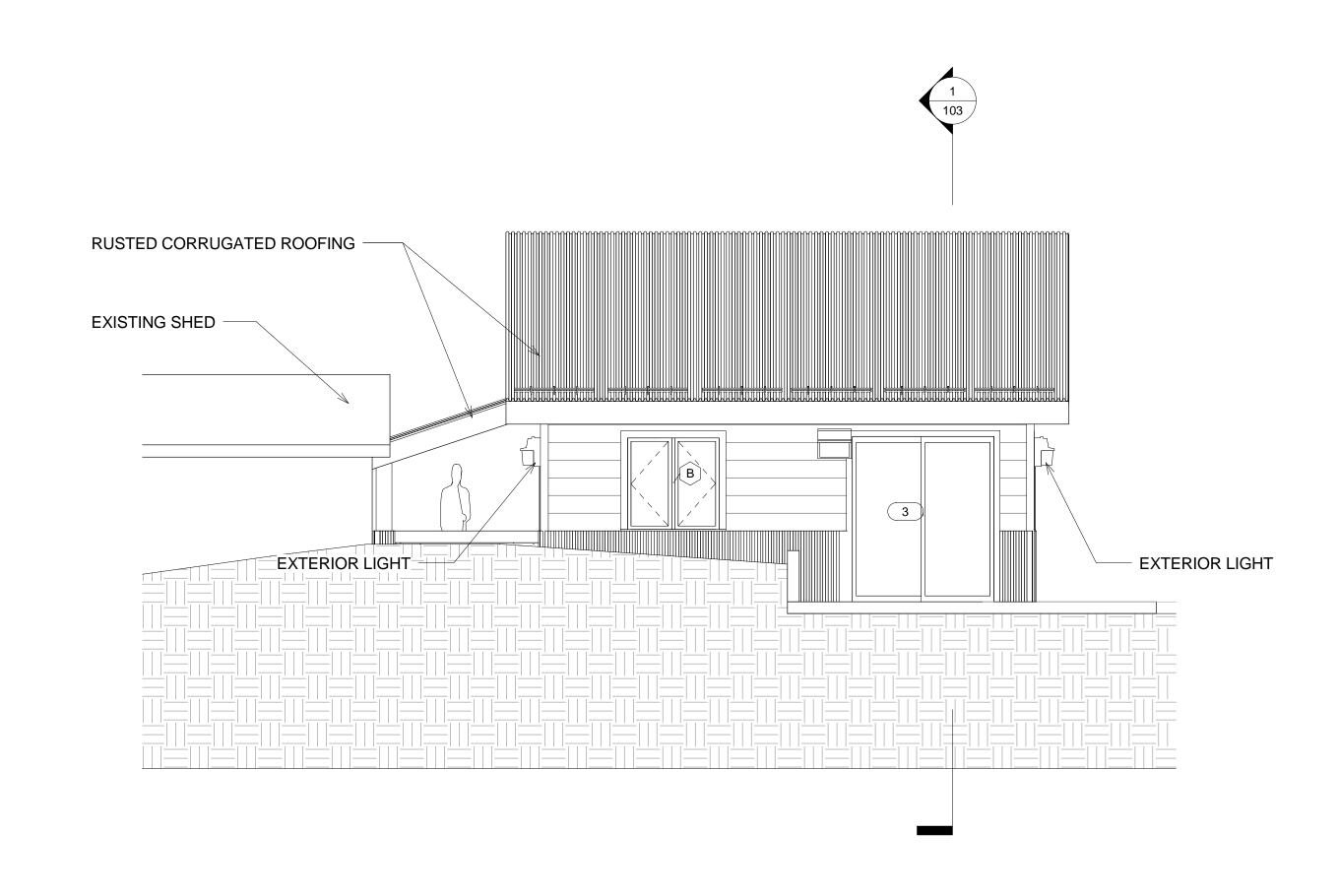
SITE PLAN

SHEET NUMBER









P.O. Box 3206
100 W. Colorado Suite 211
Telluride, Colorado 81435
p 970-728-8145
kristine@shift-architects.com
www.shift-architects.com

DATE: 2015.03.11

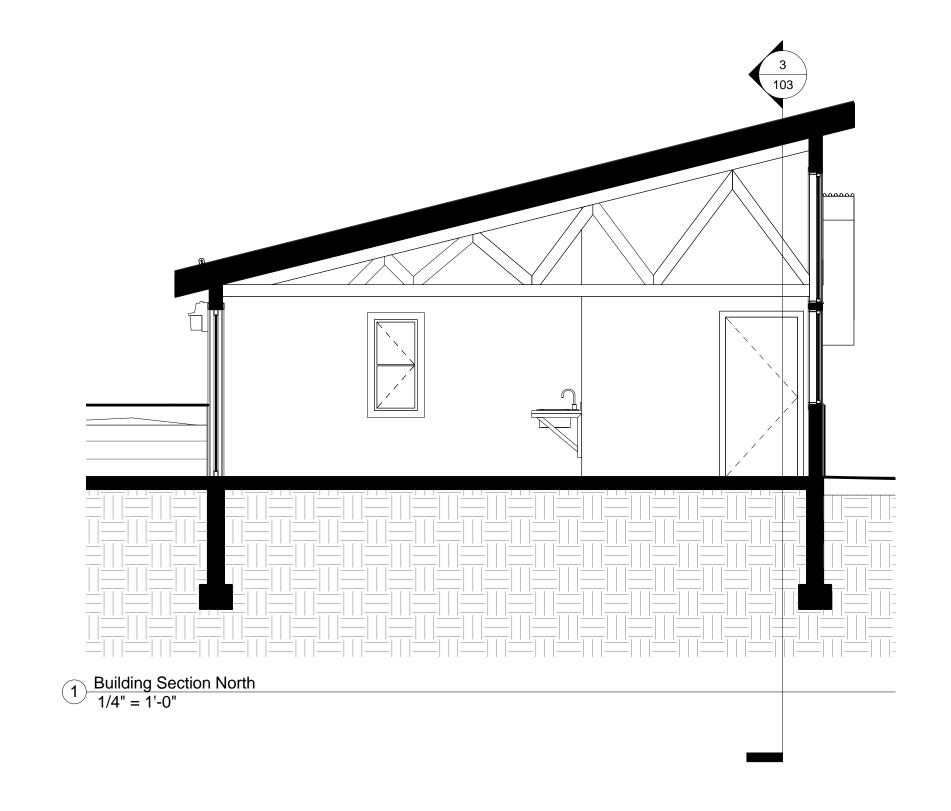
VCA Shed

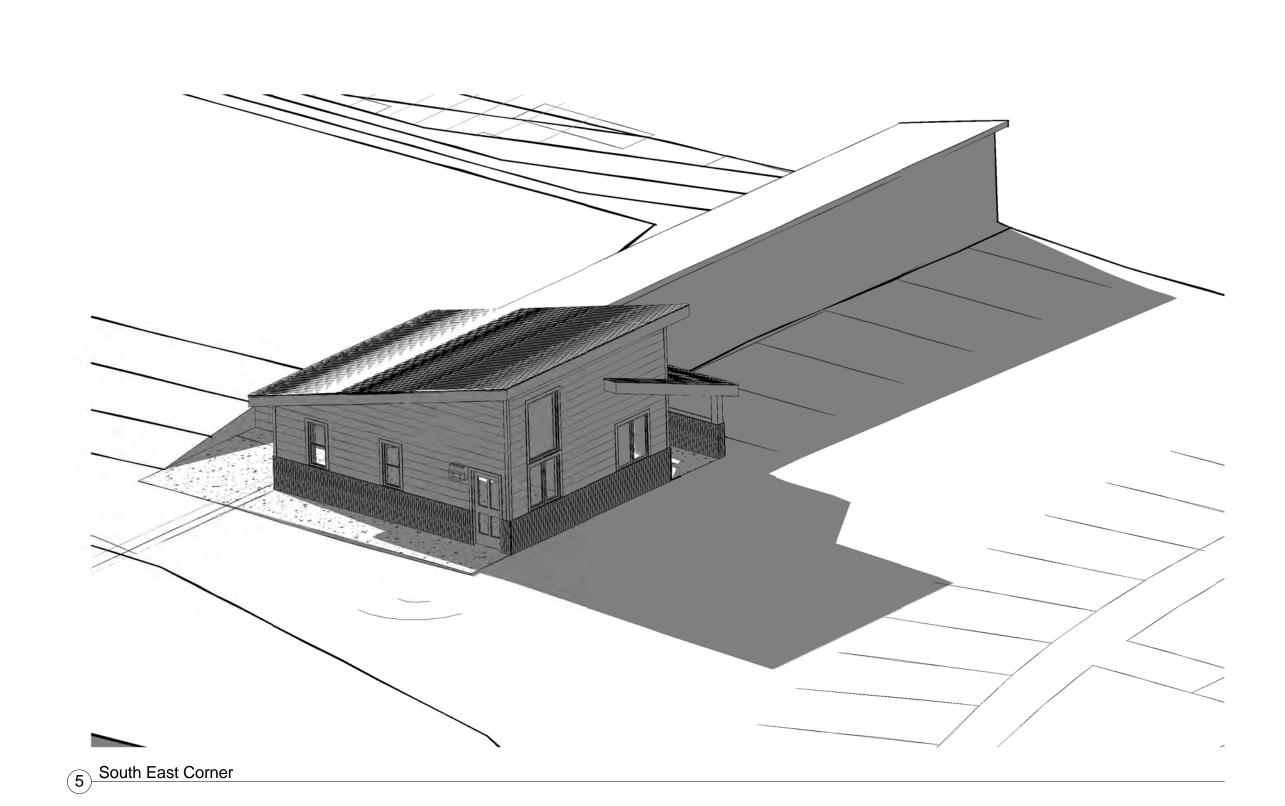
ELEVATIONS

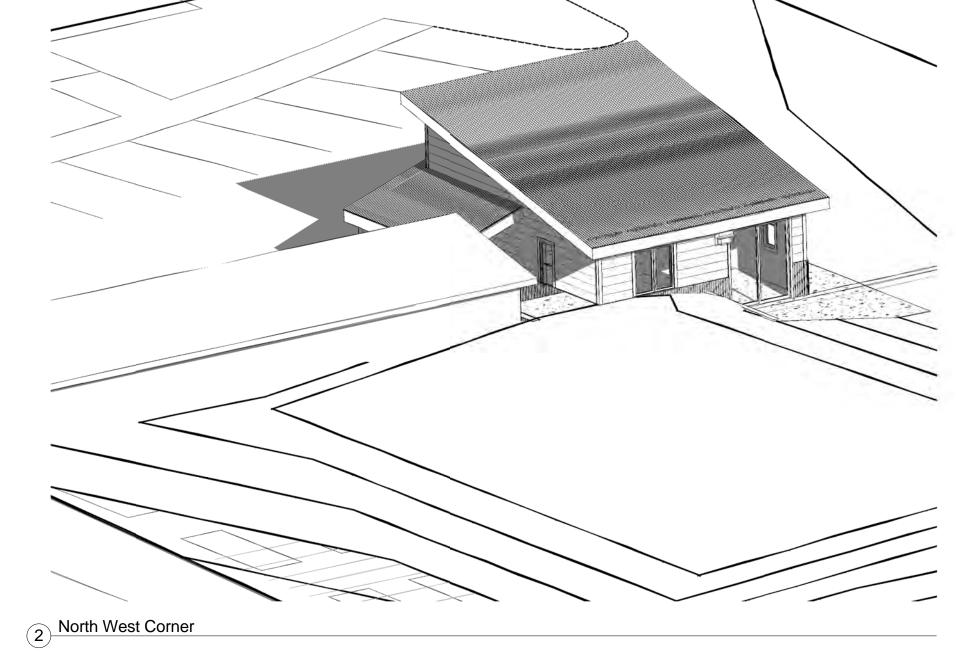
SHEET NUMBER

West 1/4" = 1'-0"





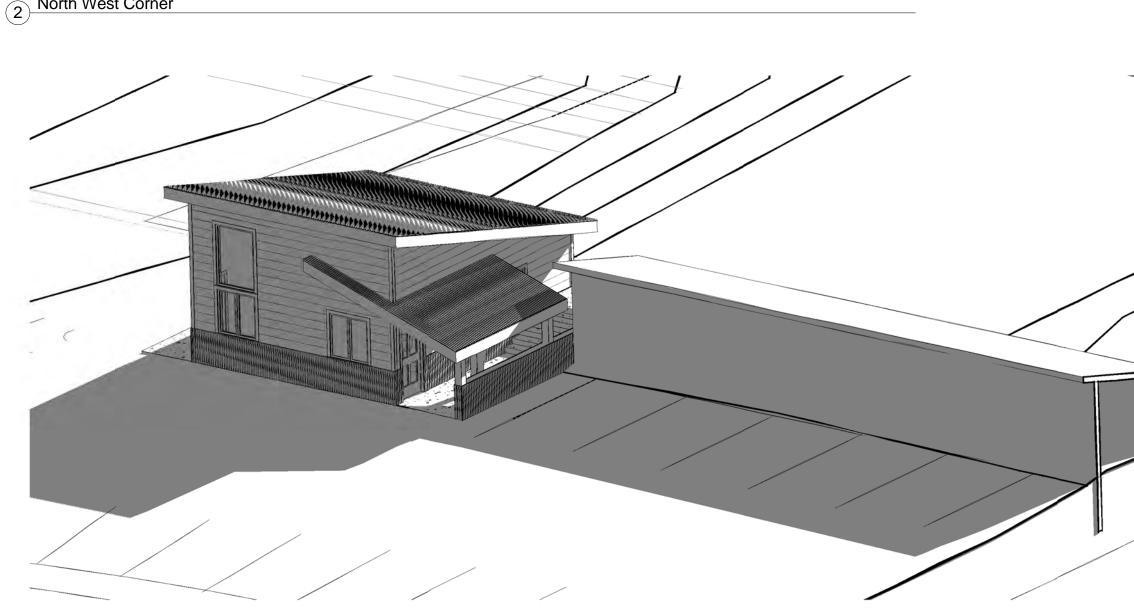




Building Section West

1/4" = 1'-0"

4 North East Corner



Shed

SECTION & RENDERING

SHEET NUMBER





COMMUNITY DEVELOPMENT DEPARTMENT
PLANNING DIVISON
455 Mountain Village Blvd.
Mountain Village, CO 81435
(970) 728-1392

DATE: May 7, 2015

TO: Design Review Board

FROM: Savannah Jameson, Planner II

FOR: DRB Work Session Agenda Item #10

RE: Conceptual Work Session Regarding Synthetic Roofing Material in the Mountain

Village

PROJECT GEOGRAPHY

Legal Description: Mountain Village

Address: Various

Applicant/Agent: Town of Mountain Village

Owner: Various
Zoning: Village Center
Existing Use: Village Center

ATTACHMENTS

• Exhibit A: Proposed synthetic roofing tiles

RECORD DOCUMENTS

- Town of Mountain Village Community Development Code as amended (CDC)
- Town of Mountain Village Home Rule Charter as amended
- Design Review Application as maintained by the Community Development Department

BACKGROUND

Synthetic roofing materials are evolving over time and staff feels that it is very important for the DRB to review new products streaming into the market over time. Several issues are also driving the need for property owners to consider synthetic roofing material. These include but are not limited to Westile going out of business so there is no replacement roofing in the Village Center and the Town prohibiting shake shingles. We continue to see more requests for synthetic slate roofing. Since roofs are a big part of the design of a building they can collectively impact the character of the Town and an area. Staff therefore believes it is a good idea for the DRB to periodically revisit appropriate roofing materials.

In 2014 staff was contacted by Pro Services, a roofing contractor that routinely does roofing work in the Village Center, and was informed that the roofing tile manufacturer, Westile has gone out of business and the concrete tiles are no longer available. DRB approved a Design Review Process application with a variation to allow for staff review for the use of synthetic roofing tiles for repair work in the Village Center at their meeting on July 3, 2014. However, this was a site specific approval for the Village Center specific to one applicant and not all roofing

contractors. Moreover, the applicant never moved forward with using synthetic tiles to repair roofs.

RELEVANT CODE SECTIONS

17.5.6.C.5. Roof Material

- a. All roofing material shall be of a type and quality that will withstand high alpine climate conditions.
- b. The review authority may require Class A roofing materials as a fire mitigation measure.
- c. Permitted roof material outside the Village Center include:
 - i. Rusted standing seam metal;
 - ii. Zinc:
 - iii. Minimum 1/2" slate; and
 - iv. 18 22 gauge black or rusted corrugated metal.
- d. Village Center roofing material shall be concrete tile or synthetic materials that emulate concrete tile of the color burnt sienna except for special copper accent roofs that shall require specific approval of the DRB.
- e. The following roofing materials may be approved by the DRB as a specific approval that is processed as a class 3 development application if the DRB finds the roofing material is consistent with the town design theme and the applicable Design Regulations:
 - i. Copper;
 - (a) Copper shall only be considered when it is proposed with a brown or verde patina finish where visible except for the Village Center where a verde patina finish is required.
 - (i.) The copper finish shall be completed prior to issuing a certificate of occupancy.
 - ii. Galvanized corrugated or standing seam metal (not rusted);
 - iii. Standing seam;
 - iv. Synthetic roofing material that accurately emulates wood shake, concrete and slate tile or any other roofing material permitted or existing in Mountain Village.
 - (a) Synthetic roofing material shall be:
 - (i.) Durable
 - (ii.) High strength, both material and shape;
 - (iii.) Low absorption or permeability;
 - (iv.) High freeze/thaw damage resistance;
 - (v.) Color throughout the tile (not surface applied); and
 - (vi.) High-quality design that fits within the architectural context of the building and the architectural context of the surrounding area.
- f. The following requirements are applicable to all roofing:
 - i. Metal roofing surface shall not reflect an excessive amount of light when viewed against direct sunlight.
 - ii. Unless the DRB grants a specific approval for a non-rusted metal roof, corrugated and standing seam roofing materials shall be pre-treated to produce rusting prior to placement on the roof, and prior to the issuance of a certificate of occupancy.
- g. The installation or re-installation of wood shakes, glazed tile and asphalt shingles is prohibited, except for the repair or replacement of wood shake areas that are 25% or less of the total roof surface area.

- h. Roof flashing, Gutters Downspouts and Similar Hardware:
- i. In the Village Center, all exposed metal flashing, gutters, and downspouts and other roof hardware shall be copper except when either structural requirements dictate the use of stronger materials such as for snow fences.
 - ii. In all other areas, other metal guttering besides copper may be approved by the review authority to allow it to match roofing material, such as the use of rusty steel guttering on a rusty metal roof.
 - iii. When steel or iron are used, they shall be either rusted to match the roof or finished with a baked on enamel paint or, subject to the prior approval of the review authority, a silicon modified alloy or special epoxy paint system of a color approved by the review authority.

Staff Recommendation

Staff has included with the memo a spreadsheet of various roofing products and will present several synthetic roofs and photos of recent projects at the DRB meeting to facilitate a discussion. The end goal of the worksession is to provide general non-binding direction on synthetic roof materials. Issues for the DRB to consider include:

- 1. How natural does a synthetic product look?
- 2. How durable are synthetic roof material in high alpine conditions where snow, ice and ultraviolet light can cause impacts?
- 3. Do synthetic roofs in the Village Center cause the need for more snow retention and protection since they will shed snow and ice very rapidly?
- 4. Should Double Roman roof tile replacements with a synthetic material come before the DRB or should the CDC be amended to allow for staff review up to a certain percentage of roof area?

| Concrete | Warranty | Fire Rating | Hail Rating | Weight | Stock | Colors | Tiles/sqft* | Color- throughout |
|-----------------------------|---------------------------------------|-------------|-------------|-----------------|------------------|-----------|-------------|----------------------|
| | Limited Lifetime, Fully Transferable, | <u> </u> | <u> </u> | <u> </u> | | | • | |
| Boral Villa 900 Impact | Non-Prorated | Class A | Class 4 | 970lb/100 sq ft | Denver | 2 colors | 8 | 7 Yes |
| | Limited Lifetime, Fully Transferable, | | | | | | | |
| Boral Villa 900 | Non-Prorated | Class A | Class 3 | 900 lb/100sqft | Denver | 7 colors | 8. | 5 Yes |
| Hanson Hacienda | Limited Lifetime, Fully Transferable | Class A | Class 3 | 920 lb/100sqft | California | 5 colors | 8 | 6 Yes |
| Crown Tuscany | Limited Lifetime, Fully Transferable | Class A | Class 3 | 910 lb/100 sqft | Florida | 7 colors | 8 | 6 Yes |
| Synthetic Concrete Tiles | | | | | | | | |
| Distinctions Synthetic Tile | 50 Year Limited Warranty | Class A | Class 4 | 240lb/ 100sqft | British Columbia | 6 Colors | | |
| Titan Double Roman Tile | 50 Year Limited Warranty | Class A | Class 4 | 198lb/100sqft | Wisconsin | 10 colors | 90 | O Yes |
| Quarrix Composite Roof Tile | 50 Year Limited Warranty | Class C | Class 4 | N/A | Denver | 6 colors | 92 | 2 Yes |
| Synthetic Cedar Tiles | | | | | | | | |
| CE DUR | Limited Lifetime, Fully Transferable | Class A | Class 4 | 170lb/100sqft | Denver | 5 colors | N/A | A Yes |
| Cost | | | | | | | | |
| Concrete Tiles | \$4.50 - \$9.00 a square foot* | | | | | | | |

^{*}Unusually steep slopes, skylights, and multiple dormers may add anywhere from \$1 to \$5 per square foot

\$7.00 - \$9.00 a square foot*

Synthetic Tiles



COMMUNITY DEVELOPMENT DEPARTMENT PLANNING DIVISION

455 Mountain Village Blvd. Mountain Village, CO 81435 (970) 728-1392

TO: Design Review Board

FROM: Chris Hawkins, Director of Community Development

FOR: Public Hearing on May 7, 2015

DATE: April 30, 2015

RE: Major PUD Amendment to Extend the Length of Validity and Vested Property Rights for a Site Specific Development Plan for Lot 109R from December 8, 2015 to December 8, 2020

PROJECT GEOGRAPHY

Legal Description: Lot 109R

Address: 632-642 Mountain Village Blvd.

Owner/Applicant: MV Colorado Development Partners, LLC

Agent: Law Offices of Thomas G Kennedy **Zoning:** Village Center Zone District

Existing Use: Vacant; North Village Center Parking Lot

Approved Use Pursuant to PUD Development Agreement: 66 efficiency lodge units; 38 lodge units, 20 condominium units, 1 employee apartment and 20,164 sq. ft. of commercial

Site Area: .825 acres

Adjacent Land Uses:

North: Vacant 89 LotsSouth: Shirana Condos

• East: Westermere & Palmyra Condos

West: See Forever & The Peaks

ATTACHMENTS

1. Applicant Narrative

2. Approved PUD Plan Set

RECORD DOCUMENTS

- Town of Mountain Village Community Development Code (as amended)
- Town of Mountain Village Home Rule Charter (as amended)
- Design Review Application as maintained by the Community Development Department.

BACKGROUND

The Town Council approved the final PUD development application on December 8, 2010 Resolution Number 2010-1208-31 as recorded at Reception Number 415339. The PUD development agreement was recorded on March 18, 2011 at Reception Number 416997 (PUD Agreement). The PUD Agreement established the length of validity until December 8, 2015. The applicant is requesting an extension for five years in order to allow the owner to continue monitoring the market conditions and complete the project when appropriate.

ANALYSIS

The only requested amendment to the PUD Agreement is the extension of the approved final PUD plan and the associated vested property rights. Section 12.16 of the PUD Agreement allows the developer to seek an extension to the PUD. The Community Development Code does not have a PUD extension process with the major PUD amendment process the only avenue for seeking an extension.

The creation of the Mountain Village Hotel PUD included the creation of Lot 109R that is now a platted lot, with the density assigned to this lot via the Town's approval of the final PUD plan and the associated PUD Agreement. The Town received Lot 644 in The Meadows in exchange for land it conveyed to the developer that is now a part of Lot 109R. The density assigned by the PUD is also assigned to the site. Thus, the developer and the Town have received benefits that cannot be reversed, with Lot 109R platted to fit the density and development allowed by the PUD Agreement. Staff would also note that it took several years, numerous public hearings and lots of resources to create the PUD and the associated site specific development plan. So staff believes that extending the PUD is warranted, especially since it will help further the goals and actions in the Mountain Village Comprehensive Plan.

STAFF RECOMMENDATION

Staff recommends that the DRB approve the requested PUD amendment with the following motion:

"I move to recommend the Town Council approve a major PUD amendment for Lot 109R to extend the PUD Agreement and the associated vested property rights a period of five vears."

Addendum to PUD Extension/Amendment

April 1, 2015

MV Colorado Development Partners, LLC, a Texas limited liability company or its successor in interest ("Owner") secured certain approvals ("Town Approvals") by the Town of Mountain Village ("Town"), authorizing the Owner to pursue a mixed use development project ("Project") on Lot 109R, Town of Mountain Village. The documents reflect the Town Approvals include, without limitation, the following documents:

- 1. Town Council PUD Approval Resolution
- 2. Replat
- 3. Development Agreement
- 4. Final PUD Plan

Copies of the Town Approvals Documents are attached to this Application.

The Town Approvals were granted through December 8, 2015 ("Town Approvals Expiration Date"), at which time they would expired unless Owner has either: (a) obtained a building permit and commenced construction of the Project Condominium; or (b) applied for and obtained an approval to extend this Agreement and the Town Approvals.

The within application is being submitted by Owner to amend the Town Approvals, including the Final PUD approval for the Project, for the purpose of extending the Town Approvals Expiration Date from December 8, 2015 to December 8, 2020 and the period of extended vesting for the Project through December 8, 2020 for reasons set forth herein.

Owner secured the project at the height of the recent economic downturn. As the economy continues to recover, the Owner notes that not all sectors are recovering at the same pace. The Project includes a substantial accommodation/hotel element, which economic sector has not recovered to a level that supports the ability for Owner to secure necessary financing to insure the success of the Project.

The Owner seeks the extension of time to allow it to continue to monitor market conditions and, when appropriate, complete the Project.

Respectfully Submitted,

MV Colorado Development Partners, LLC, a Texas limited liability company

Printed Name: John Wagner

Title: Vice President

Chris Hawkins

From: neal elinoff < nealelinoff@gmail.com>
Sent: Wednesday, April 15, 2015 4:45 PM

To: Chris Hawkins

Cc: Dominic Spagnuolo; Greg Ritter; david eckley; Steve Schulz; MICHAEL VAZQUEZ; Lee Pressler regarding the PUD Amendment hearing at DRB (May 7) and council (May 21) for Lot 109R

Dear Chris,

I'm one of the homeowners whose views got destroyed when MV permitted the development of See Forever Village large apartment condos. Currently and at that time, there was a Ridge Development restriction. Indeed, that's why the original homeowners, the ones that paid a heavy price for unobstructed views, purchased on the ridge to begin with. We knew that any development could not be built high enough to be seen from the valley floor.

John Abrams got a variance and as you know, the blight of empty, "See Forever Village" apartments can be seen from the valley floor because of that variance.

Now all that we have left are partial views up the ski hill. For 8 homeowners who purchased unobstructed view homes, we currently have major view obstructions and if the project under review is built on the entirety of the land, the only views remaining will be of exterior walls with brightly lit interior walkways and stairwell lights that would double up with the new project. Indeed, we'd have no views at all. We might as well have built in a canyon instead of a ridge.

Kindly view these issues during the day and especially at night so that you can see our blighted view corridors as they currently are and as they will be impacted to a final degree if this project is built as the developer is suggesting. When you do so, you'll see the negative impacts that old and potentially new developments will blight our property with and destroy our value.

Sincerely,

Neal Elinoff president

Elinoff & Co. Gallerists and Jewelers 204 West Colorado Ave. PO Box 2846 Telluride, CO 81435

work: 970-728-5566; fax: 970-728-5950; cell: 970-708-0679



Mountain Village Hotel BOKA Powell Project No. 08131.100

Supplemental Information Final PUD Application Town Council Review

Issue Date: November 18, 2010



Table of Contents

Mountain Village Hotel Supplemental Information Final Submission

| Tab 1 | Exterior Material Information |
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| Tab 2 | Material Key Rendering |
| Tab 3 | MEP Narrative |
| Tab 4 | Lighting Fixture Information |
| Tab 5 | Snow Guard Detail / Typical Bathroom Vent |
| Tab 6 | Landscape Water Usage Chart |
| Tab 7 | Paving Specification |
| Tab 8 | Bollard Information |
| Tab 9 | TFD Exhibits |
| Tab 10 | Drainage Summary |
| Tab 11 | Geotechnical Reports |
| Tab 12 | Westemere Exhibits |



EXTERIOR FINISHES

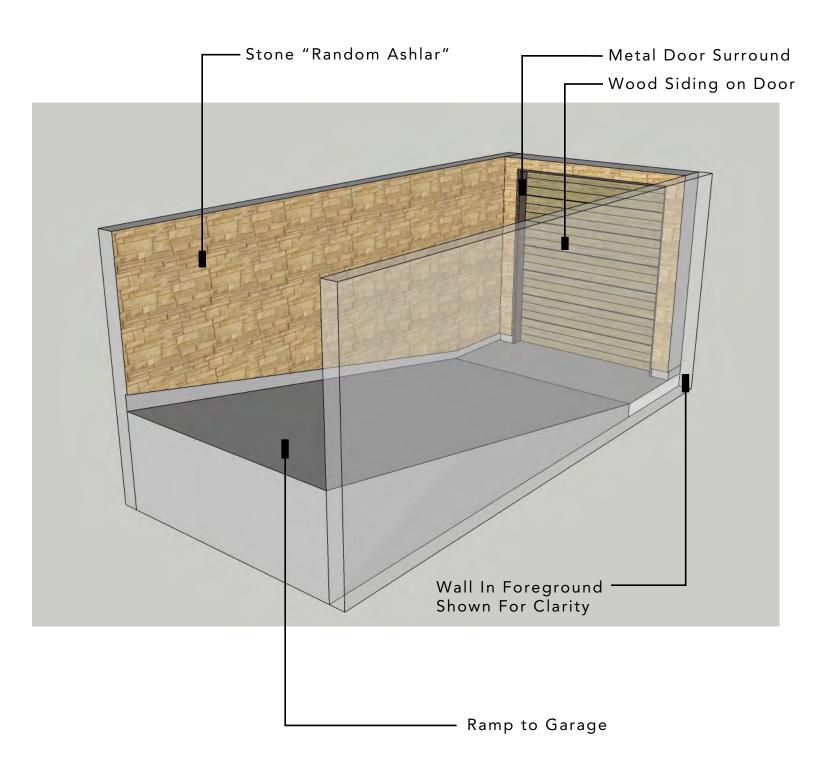




Materials 8.31.2010 PROJECT NO: 08131.100











November 18th, 2010 27

John E. Orfield | Principal BOKA Powell, LLC Architecture+Interiors+Planning+Graphics 8070 Park Lane, Suite 300 | Dallas, Texas 75231 T 972.701.9000 | F 972.991.3008 | C 214.289.0078 jorfield@bokapowell.com | www.bokapowell.com

Attn: John

Re: Mountain Village Hotel Mechanical, Electrical and Plumbing system narratives.

HVAC PERFORMANCE CRITERIA 1.

DESIGN INTENT

The performance criteria are intended to define the level of performance of the HVAC system. The parameters used herein shall form the basis of the HVAC system design, equipment selection and system sizing.

B. CODES AND STANDARDS

The engineering calculations are based on the latest recommendations of ASHRAE and good engineering practices consistent with industry practice.

The codes applicable to the design are as follows:

LEED silver (USGBC) International Building Code (2009) International Electrical Code (2008) International Fire Code (2009) International Mechanical Code (2009) International Plumbing Code (2009) International Residential Code (2009)

The standards applicable to the design are as follows:

- American Society of Heating, Refrigeration and Air Conditioning Engineers (ASHRAE) handbooks
- Sheet Metal and Air Conditioning Contractors National Association (SMACNA)
- American Society of Mechanical Engineers (ASME)



2. HVAC SYSTEM DESCRIPTION

A. INTRODUCTION

- The HVAC system for the Mountain Village Hotel will consist of a centralized chilled water and boiler plant serving the guest room fan coil units and air handling units located through the hotel. The boiler plant will also provide hot water for domestic use, laundry and snow melt systems.
- 2. The system is based on 7 stories above grade and three garage levels below grade including utility and back-of-house spaces. The Porte Cochere entrance will incorporate two insulated, automatic doors leading to a tempered (45°F to 50°F) garage. Retail spaces, a business center, a bar and lounge, guest services, pool services, kitchen and dining, offices, spa and exercise areas will occupy the Plaza Level. The main electric room in the lower garage level. The laundry and kitchen support areas will be on the ground level as well as an entrance lobby. Mechanical equipment rooms will be primarily in the southeast corner of the two parking levels and in the north west corner of the basement level garage.

B. CHILLED WATER PLANT

- 3. The chilled water plant will consist of (2) water cooled chillers (220 to 250 tons) with a variable primary pumping system distributing chilled water to the air handling units and guest room fan coil units. The chilled water plant will be located in the basement garage level chiller machinery room. The plant will also incorporate a water side economizer (2 150 ton cooling towers) to benefit from free cooling during low wet bulb conditions.
- 4. The chillers will be a water cooled electric centrifugal or screw type with a minimum full load efficiency of 0.55 kw/ton at 72°F entering condenser water temperature. The chilled water supply temperature will be 45°F and the return temperature 57°F. The condenser water supply temperature will be 72°F and the return temperature 87°F. The chillers will use R134a refrigerant (non-CFC) and the machinery room will be equipped with a refrigerant leak detection and ventilation system in compliance with ASHRAE Standard 15.
- 5. Chilled water will be distributed by two (2) variable flow chilled water pumps, each selected at approximately 70% of peak flow. The pumps will be vertical centrifugal type, approximately 450 GPM at 120ft TDH each, operated through dedicated variable frequency drives. Each pump will be furnished with a suction diffuser. An automatic bypass control valve shall maintain minimum flow through the chillers.
- A free cooling cycle will provide chilled water through a plate and frame heat exchanger when the wet bulb temperature falls below 40°F. The heat exchanger will be sized to provide approximately 150 tons of free cooling.
- The chilled water loop will have an air separator on the return water back to the chillers, a vertical bladder type expansion tank and a chemical pot feeder for corrosion control.
- 8. In addition to hydronic fan coils and air handling units the chiller will serve water cooled walk-in coolers and freezers, water cooled ice machines, water cooled electric, elevator



machine, and data room fan coils. By linking all cooling systems to the chilled water loop there will be no need for unsightly air cooled condensing units on the project.

- 9. The cooling tower plant will consist of two open cooling towers with flume box. The fans will be centrifugal, forced draft type, with variable speed drives. Tower construction will include a stainless steel basin with galvanized steel structure and deck. The basin will not require electric basin heaters as the tower will may be used as a garage ventilation fan, located in the basement parking level. The cooling towers will be matched to provide condenser water for 50% of the chiller requirements. In this way the two cooling towers will provide 100% redundancy for the tower free cooling cycle to provide chilled water during cool weather which will peak around 150 tons (including the many back-of-house water cooled air conditioning systems.
- 10. Condenser water will be circulated between the cooling towers and the chillers by two (2) condenser water pumps, one operating and one standby. Each pump will be designed for approximately 240 GPM flow rate at 60 ft TDH discharge pressure. Each pump will be furnished with a suction diffuser.
- 11. A dedicated condenser water valve shall route condenser water through the cold side of the plate and frame heat exchanger when the outside wet bulb conditions are low enough to take advantage of free cooling.
- 12. A three way bypass valve will be used in conjunction with the cooling tower fan speed to control condenser water temperature.
- 13. A chemical treatment system consisting of a corrosion inhibitor and biocides with a conductivity sensor and control panel will be provided for the cooling tower. The chemical drums and control panel will be located in the chiller room.
- 14. A packaged cooling tower water sand filter located in the mechanical room adjacent to the cooling towers will filter particulate accumulated in the cooling tower basins.

C. BOILER PLANT

- 15. The heating hot water plant will consist of three to six condensing, fully modulating, natural gas fired water tube boilers, rated for a total nominal output of approximately 8500 MBH at altitude. The boilers will be located in the upper garage level mechanical room. Sealed combustion air will be delivered through an engineered system supplying air in and flue gas out. The boilers will nominally run at 160°F supply temperature and 110°F return water temperature for maximum condensing and efficiency. The heating plant will run without glycol and pump through a set of space heating heat exchangers as well as a set of domestic hot water generating heat exchangers. The space heating fluid shall be a solution of 50% polypropylene glycol.
- 16. The hot water plant will use a primary secondary pumping scheme with constant speed primary hot water pumps serving each boiler, constant volume secondary hot water pumps serving the domestic hot water generators and variable speed secondary hot water pumps serving the space heating piping distribution system.



- 17. The primary hot water pumps will each be vertical centrifugal type and will be manifolded into a common header. The primary loop will be decoupled from the secondary loop.
- 18. Two (2) secondary vertical centrifugal hot water pumps will be manifolded into a common header to distribute hot water to the space heating coils in the central station air handling units, reheat coils, unit heaters; and the snow melt heat exchanger. These two pumps shall be sized for 70% of full load and will run in tandem on a variable speed pumping loop for maximum operating efficiency. The domestic hot water heat exchangers serving the kitchen, the laundry, and the guest rooms shall each have their own pair of constant volume circulators dedicated to their respective loads with 100% installed redundancy (one pump is stand-by out of each of the three pairs).
- 19. The heating hot water loop will have an air separator on the supply water from the boilers, a vertical bladder type expansion tank and a chemical pot feeder for corrosion control. The loop will be controlled through a three-way valve to reset the system water temperature from 85°F to 125°F as the outside air temperature falls from 55°F to 0°F. Public area fan coils and make-up air units that need higher temperature heating water will be boosted by one of two or three strategically located instantaneous sealed combustion hot water heaters.

D. COMMON AREAS

- 20. Central make-up air units strategically located throughout the building will provide fresh air to common areas and retail spaces. Individual spaces will be served by hydronic fan coils (two or four pipe, depending on whether radiant floors are incorporated into the design).
- 21. Central make-up air may be ducted or delivered through a tunnel ceiling construction to each guest room for superior comfort and indoor air quality. Should continuous ventilation be included in the final design, toilet exhaust systems will likely be routed through air-to-air heat recovery heat exchangers for maximum energy efficiency.
- 22. The Dining area will be served by a central station air handling unit. The HVAC system shall be as follows:
 - (a) One (1) variable air volume central unit with a capacity of approximately 10,000 cfm supply air.
 - (b) The unit shall consist of a supply fan with VFD, chilled water cooling coil, hot water heating coil, 85% efficient filters, 100% outside air section with dampers, exhaust/relief fan with VFD and 100% exhaust air section with dampers.
 - (c) The exhaust/relief fan shall be sized to match the supply fan capacity for use in smoke control. The unit fans will be connected to the emergency power system.
 - (d) The unit will supply six to ten VAV boxes with hot water reheat coils where needed for zone control. The air distribution will be through various ceiling diffusers.
- 23. The Lobby, Bar and Public restroom areas will be served by a central station air handling unit. The HVAC system shall be as follows:
 - (a) One (1) variable air volume central unit with a capacity of approximately 7,000 cfm supply air.

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- (b) The unit shall consist of a supply fan with VFD, chilled water cooling coil, hot water heating coil, 85% efficient filters, 100% outside air section with dampers, exhaust/relief fan with VFD and 100% exhaust air section with dampers.
- (c) The exhaust/relief fan shall be sized to match the supply fan capacity for use in smoke control. The unit fans will be connected to the emergency power system.
- (d) The unit will supply five to ten VAV boxes with hot water reheat coils for zone control. The air distribution will be through various ceiling diffusers.
- 24. The Spa and fitness center will be served by multiple fan coil units. 100% fresh air and exhaust will be provided for the locker, shower and exercise areas.

E. GUEST LODGE, EFFICIENCY LODGE AND CONDOMINIUMS

- 25. The guest rooms will be conditioned by four (4) pipe chilled water fan coil units located in the area or a (2) piped combined with in-floor radiant heat. It is assumed that outside air for ventilation (65 cfm) will be through a centrally ducted or tunnel ceiling delivery system in the corridors with 4" round ducts penetrating each unit through a 6" x 6" grill just in side the unit door, OR through windows 4% of floor area to met the ventilation requirements. Return air will be from the space served through a return air grille at the face of the unit enclosure. Some areas may need horizontal soffit mounted ductwork to serve more than one room with a single fan coil unit.
- 26. The guest room fan coil units will consist of horizontal soffit mounted units with integral access doors. Units will have a capacity of 350 to 600 cfm, and will include a factory supplied sound attenuation package, valve package and integrated (to the front desk) thermostat.
- 27. A networked property management system (PMS) is expected to be incorporated into the design such as Epitome by Softbrands or INNCOM's e4 Smart Digital Thermostat
- 28. Guest room kitchens shall be limited to 36" ranges with exhaust requirements of no more than 400 cfm.
- 29. Penthouse units with steam showers will receive toilet exhaust systems.
- 30. The maid and service rooms on each level appropriately will be conditioned by four (4) pipe chilled water/heating hot water fan coil units located in the area served.
- 31. The guest room bathrooms will be exhausted by common risers collected in an attic duct system leading to make-up air units pre-conditioned with air-to-air heat exchangers recovering heat (or cooling) from the exhaust airstreams. Each bathroom will be supplied with a wall mounted exhaust grille leading to a sub duct. The exhaust quantity will be 35 cfm at each bathroom. Should ventilation in the Guest Rooms be based on operable windows only, the restroom exhaust fans will need to be individually switched and may lead to looking at individual 4" duct runs up and out through some roof detail, or worst case, out the sidewalls.
- 32. The guest corridors will be conditioned by make-up air units mounted on the roof or buried in the building in mechanical rooms. The make up air unit shall consist of a supply fan, an

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exhaust fan, a heat recovery coil or coils, hot water heating coil, chilled water cooling coil, filters and intake section. A central supply air duct will distribute air in the corridor at each level. Fire/smoke dampers shall be provided at each supply grille at each level.

- 33. The telephone and electrical rooms at each floor will be supplied with two pipe cooling only fan coils.
- 34. The passenger elevator machine room and the freight elevator machine room on the Tower roof will each be conditioned using a wall mounted packaged water cooled heat pump.

F. KITCHEN

- 35. The kitchen is assumed to have three (3) cooking areas, two (2) dish washing areas, walk in coolers and freezers and office space.
- 36. Each cooking area will have a kitchen hood with a dedicated exhaust fan and make-up air unit. The exhaust fan will be a roof mounted centrifugal exhauster with ventilated curb sized for 5000 cfm. The make-up air unit will be a factory packaged supply fan, gas furnace heating section and filter sized for 5000 cfm and designed to maintain a minimum of 60 °F supply air temperature. The exhaust fan will be ducted to the kitchen hood using welded carbon steel ductwork with clean outs per code. The make-up air from the unit will also be ducted to the kitchen hood with galvanized sheet metal ductwork. The balance of the make-up air to the kitchen hood will come from the surrounding areas.
- 37. The kitchen exhaust fans will have extended stacks with a 30" x 20" chases through the building to terminate at the roof.
- 38. Each dishwasher area will have a ventilation hood with a dedicated exhaust fan. The fans will be roof mounted centrifugal utility sets sized for 1000 cfm each. The exhaust fan will be ducted to the dishwasher hood using stainless steel ductwork.
- 39. The kitchen work area and support spaces will be conditioned using a four pipe fan coils coils at each zone for temperature control. Gas fired, heating only make-up air units will be paired with each kitchen hood.
- 40. Miscellaneous spaces. The basement back of house and mechanical spaces will ventilated by mechanical exhaust and heated using hot water unit heaters located in the spaces served. The areas will include the following:
 - (a) Mechanical rooms.
 - (b) Maintenance shop.
 - (c) Receiving dock
 - (d) Compactor room.
 - (e) Pool equipment room.
- 41. The main electrical room in the lower garage level will be conditioned by a dedicated heat pump. The unit will be located immediately adjacent to the electrical room.



- 42. The chiller machinery room in the basement will be ventilated by a dedicated exhaust fan controlled through a refrigerant leak detection control panel and sensors designed to shut down the chiller plant and ventilate the room upon the detection of refrigerant over set point. Make up air will be provided to the chiller room through a unit serving the kitchen and dining area as well
- 43. The hot water boiler room in the basement will be utilize sealed combustion intakes and flues negating the need for large combustion air louvers into this room.
- 44. The laundry room in the basement will be supplied with conditioned air from the central make up air unit serving kitchen and dining areas. The laundry room will be exhausted through the ironer exhaust fan and the laundry dryer exhaust fans, both located in the laundry room and ducted through a dedicated shaft to vents on the roof.
- 45. The main telephone room will be conditioned using a heat pump. The unit will operate 24 hours per day and will be connected to the emergency power system.
- 46. Electric cabinet unit heaters will be provided at entrance vestibules and at the bottom of each exit stairwell.
- 47. Hot tub areas will be ventilated, dehumidified and heated using a factory packaged dehumidification unit with air cooled condenser. The unit will be mounted in the pool house mechanical room and will be sized for 5000 cfm. The ductwork serving hot tub areas will be PVC coated sheet or aluminum metal duct. Air from the adjacent spa will be transferred to the indoor pool to maintain a negative air flow to the pool area.

G. FIRE AND LIFE SAFETY

48. No smoke control systems are anticipated at this time.

H. SNOWMELT

- 49. The areas indicated on the snow melt plan will be furnished with a snow melt system.
- 50. The system will use embedded PEX tubing and will be sized to cover an area of approximately 25,000 square feet. The snowmelt system will simply be an extension of the radiant floor piping system, using the same pumps, heat exchangers, and temperature resetting mixing valve to deliver 85°F to 125°F fluid. The heat transfer fluid will be 50% propylene glycol. The snowmelt system will provide automatic snow/ice detection and will maintain an idling temperature in the snow-melting slab during very cold weather.

CONTROLS

- 51. A DDC BAS system will be installed with equipment provided by Alerton, JCI, Invensys, or
- 52. This system will control and monitor the following:
 - (a) Central chilled water plant and cooling towers.



- (b) Central hot water boiler plant.
- (c) Common area central station and make-up air handling units and zone VAV boxes.
- (d) Back of house fan coil units.
- (e) Snowmelt systems.
- (f) Lighting controls
- 53. A separate system will monitor CO levels in the van parking area and control the exhaust fan serving this space.
- 54. Guest room fan coil units and exhaust fans, cabinet heaters, unit heaters and miscellaneous exhaust systems will have stand alone controls or will run on a 24/7 basis. These systems are not included in the DDC/BAS system.
- 55. The packaged heat pump systems serving the elevator machine rooms, tele data room and the AV room will have network thermostats.

PLUMBING SYSTEM DESCRIPTION 3.

A. INTRODUCTION

1. The natural gas, domestic water, and sanitary systems for the building will be provided within and up to a point 5 ft from the building. All site piping will be by others.

B. DOMESTIC WATER

- 2. A 3" or 4" domestic cold water service will be brought in from 5 ft. outside the building near the loading dock. A backflow preventer will be provided on the service where it enters the building.
- The domestic water will be distributed to the first floor plumbing fixtures directly from the main service.
- 4. Domestic Hot Water for the guest rooms and common areas will be generated using heat exchangers located in the basement boiler room. The hot water generators will each be sized to handle 100% of the load. An in-line pump will be installed to circulate water through the heat exchanger and the tanks to ensure that hot water will always be available for use. The exchangers will use heating water from the main gas fired hot water boilers. The domestic hot water skid will include (2) heat exchangers each with a recovery capacity of approximately 1400 GPH and two (2) 1200 gallon storage tanks. The water will be generated and stored at 140°F.
- 5. Domestic Hot Water for the kitchen will be generated using double wall heat exchangers located in the basement boiler room. The hot water generators will each be sized to handle



100% of the load. An in-line pump will be installed to circulate water through the heat exchanger and the tanks to ensure that hot water will always be available for use. The exchangers will use heating water from the main gas fired hot water boilers. The kitchen domestic hot water skid will include (2) heat exchangers each with a recovery capacity of 35 GPM and one (1) 1000 gallon storage tank. The water will be generated and stored at 140°F.

- 6. The majority of the laundry services will be outsourcers. The reminding laundry requirements will be accomplished with washer hot water being generated by a gas fired hot water heater located in the basement boiler room. The hot water heater will each be sized to handle 100% of the reminding laundry requirement load (taking inlet water from the 140 °F house supply). An in-line pump will be installed to circulate water through the heat exchanger and the tank to ensure that hot water will always be available for use. The water will be generated at 165F unless an ozone based cleansing system is installed.
- 7. The pipe material shall be Type 'L' copper, or Charlotte CPVC, or Uponor AquaPEX insulated with 3/4" fiberglass for all hot water and hot water circulating lines.
- 8. The water quality is assumed to be less than 7 grains per gallon hardness. No water softeners are planned at this time.

C. SANITARY SEWER

- 9. A sanitary sewer connection will be made into the building from 5 ft. outside the footprint of the building.
- 10. Sanitary sewer piping system for the guest rooms and for the public spaces will be a mix of combination waste and vent and conventional waste and vent systems.
- 11. The sanitary sewer fixtures in the basement will be routed to a sewage ejector located in the maintenance area. The ejector will be a duplex pump with fiberglass basin and the discharge will connect to the main sanitary lateral at grade level.
- 12. One (1) 3000 gallon restaurant grease interceptor will be located on the north side of the building adjacent to the kitchen or at the lower level loading dock area.
- 13. Condensate drains will be collected from the fan coil units at the base of each riser and routed to a floor sink or approved waste receptor connected to the sanitary sewer system. Condensate from air handling units and fan coil units will be routed to the nearest floor sinks or approved waste receptor connected to the sanitary sewer system.

D. NATURAL GAS

- 14. Natural gas piping will be routed from the meter near the loading dock to each point of use for the following equipment:
 - 1. Hot water boilers.
 - 2. Make up air unit gas furnaces.



- Kitchen equipment.
- 4. Laundry equipment.
- 15. The piping above grade shall be black iron, schedule 40 steel, Gas-Tite, Trac Pipe, or copper. A main gas meter (by the utility) will be provided at the building entrance.
- 16. The natural gas will be distributed at a pressure of 2 psig to the equipment.

E. FUEL OIL

- 17. The emergency generator is assumed to be supplied with an integral diesel fuel oil storage tank. An external fuel oil tank with pumps will be needed if extended operating capabilities longer than a few hours are desired.
- 18. The hot water boilers are assumed to be natural gas fired only. No back up fuel system is provided.

F. PLUMBING FIXTURES

Penthouse units expected to have steam showers.

4. ELECTRICAL

A. DESIGN INTENT

This document is intended to define the parameters for the overall electrical service, the electrical distribution thru-out the building(s) and lighting, exterior, interior and controls.

B. CODES AND STANDARDS

The codes applicable to the design are as follows:

- 2008 National Electrical Code (NEC)
- 2006 International Energy Conservation Code (IECC)
- LEED certified or LEED Silver certified.
- Local building code amendments

C. MAIN ELECTRICAL SERVICE

A new 120/208V. 3 phase 4000 amp electrical service from San Miguel Power Association with an exterior pad-mounted transformer and possibly a utility connection cabinet to be located with coordination with landscape design. This service will be sized to provide adequate power for the entire facility with a 10% to 20% spare capacity based on owner input and cost effectiveness.

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There will be one utility central meter for the entire hotel with separate utility meters for each of the retail spaces.

D. ELECTRICAL ROOMS

The main electrical room in the parking garage to be 1 hour fire rated and a minimum of 12' x 30' with two exit doors at opposite ends. To be located as close to the utility transformer and the generator.

Plaza level to be fed from main electrical room in parking garage and panel boards place for convenience and cost restraints.

The electrical rooms for floors 1 thru 5 to be vertically stacked and a minimum of 8' x 8' with a single door. Routing of conduit from main electrical room to these electrical rooms should be considered in the final design of the plaza level.

Tenant retail spaces to have their own meter in the main electrical room with a panel board sized to the relative size of the space.

E. LIGHTING

 Exterior lighting: Please see attached lighting plans for the description of proposed exterior lighting.

These fixtures will be a combination of incandescent, LED and PL fluorescent lamping; compact fluorescent will need to be provided with starters rated for cold weather.

Walkway and courtyard lighting away from main plaza to be kept to a minimum needed for safety and shall comply with the same restrictions for plaza lighting.

Garage lighting is to be LED low profile fixture.

2. Interior lighting

Guest room lighting can be a combination of incandescent, LED and compact fluorescent.

Due to the restrictions of the International Energy Conservation Code, IECC 2006 the use of incandescent lighting in the common areas is severely restricted due to high wattage for light output.

It is our recommendation to replace all incandescent and compact fluorescent down lights with a LED down light. The LED down light is slightly more expensive than a commercial compact fluorescent but has the ability to be dimmed with any commercially available dimmer. The average life for the LED is 50,000 hours, with compact fluorescent at 12,000 hours, so maintenance is reduced. The light quality of the LED (92 CRI) is superior to any compact fluorescent (82 CRI).

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Egress/emergency lighting

Exit signs for front of house to be an edge lit LED high quality fixture.

Exit signs for back of house to be LED lit white impact plastic generic fixture.

For egress lighting in the front of house, atrium, dining, lobby and guest room corridors is to be emergency ballasts in some of the normal light fixtures.

Egress lighting in back of house, service corridors, kitchen, covered parking and swimming pool are to be frog eye wall packs.

Lighting Controls

Guest rooms shall have local switching for all lighting and the overall control for all hard wired fixtures and switched receptacles at the unit entrance per IECC 2006. In the larger units a small lighting control would be recommended for convenience and salability.

Guest floor corridor lighting shall be on continuously.

Stairwell fixtures to have individual occupancy sensors combined with dual level lighting so the stair well runs a 50% light level until occupied.

Public spaces to maintain a minimum level of lighting at all time with extra and/or decorative lighting to be controlled through either a dimming rack or lighting control panel. This can be used to take advantage of day-lighting and can be set to automatic or controlled locally.

All back of house offices, storage, etc. to utilize dual level switching or occupancy sensors.

Garage lighting can be controlled thru a step dimming system and occupancy sensors that allows a 50% power and light reduction of each fixture until the space is occupied then the space is brought to full brightness.

F. GENERATOR

Generator is preliminarily sized at 230 KW to provide power to run four hydraulic ram elevators and some mechanical loads as yet to be determined. If additional loads are required by owner or code to be on the generator i.e.; change to traction elevators. adding lighting or mechanical loads then the size of the generator may change

Generator to be installed in garage with a minimum clear space including generator and working clearances to be 15'L x 10'W x 7-1/2'H.

Generator package shall include a critical grade silencer, diesel belly tank with supply for two hours of operation and day tank for testing.

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If the run time for the generator is to exceed two hours then a separate dual walled diesel fuel tank in close proximity to the generator will be required.

Two automatic transfer switches are to be provided, one switch for the elevator load and the other switch for any additional loads, to avoid the elevator selective coordination issues.

Joseph Gaumond, LEED AP

Principal

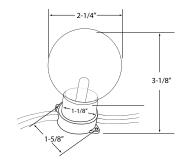
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FESTIVAL LIGHT STRING

Festive and easy to use, low voltage Festival **Light String** allows runs up to 120 feet. Long life, high performance 24V xenon lamp assemblies are attached to 12-gauge wire at a preset spacing of 12" or 24". Wire, sockets, lamps and clear globes are included and optional colored globes may be purchased separately and replaced on the job site. The 24V system requires use with a magnetic transformer to maintain proper voltage.









FEATURES

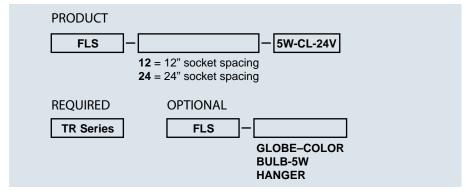
- Durable, safe, low voltage lighting that adds a festive look
- Clear or colored polycarbonate globes provide a secure watertight seal around bulb
- Sockets firmly attached to 12-gauge wire
- Field cuttable for custom applications (24V is non-rated) use with c/UL/us rated magnetic transformers
- Socket may be mounted to wire using a plastic, v-shaped hanger (sold separately) or surface mounted with socket bases' mounting holes
- 12" and 24" socket spacing options



BILL OF MATERIALS

- ☐ Determine socket spacing (12" or 24")
- ☐ Determine quantity of light strings needed (length for 12" spacing is 60 feet; length for 24" spacing is 120 feet
- ☐ Determine total wattage and transformer location(s)
- ☐ Order light string(s) and 24V magnetic transformer(s)
- ☐ Consider whether hangers will be needed

ORDERING INFORMATION



ACCESSORIES

| _ | ? | FLS-GLOBE- (COLOR) | Colored globes for Festival Light String, 2-1/4" diameter, PVC Specify Color: BL ue, FR osted, GR een, OR ange, RE d, YE llow and CL ear replacement |
|--------|----------|-----------------------|---|
| - | \wedge | FLS-HANGER | V-shaped plastic hanger that attached to socket base for hanging on cable |
| - | | FLS-BULB-5W | Replacement lamp, 5 watts, 24V, 8,000 hours average life, wedge base |
| pg 120 | • | TR Series | Magnetic transformer, enclosed 12V or 24V, dimmable, with multiple knockouts and boost tap 150 watts (one 15 amp circuit) or 300 watts (one 25 amp circuit) For runs up to 25 feet, c/UL/us |

TECHNICAL SPECIFICATIONS

Voltage 24V, 60Hz AC operation

Construction Durable PVC plastic globes mounted over durable long lasting xenon socket assemblies on 12AWG copper wire

Bulk reel 60 foot string (12" socket spacing) or 120 foot string (24" socket spacing)

H = 3-1/8" (socket with globe), Dia = 2-1/4", W = 1-5/8" (socket base mounting holes) Dimensions

Wattage 5 watts per lamp

Maximum Run 60 feet for 12" socket spacing; 120 feet for 24" socket spacing

Packaging Both 12" and 24" socket spacing product include wire, socket, lamps, and clear globe (colored globes must be

ordered separately) pre-assembled in a box

Lamp specs 8,000 hour rated life

Dimmable with most standard incandescent dimmers Dimmers

Diffuser Installed clear polycarbonate globes are standard; colored globes are sold separately













STEPLYTES-LED









RSC2 ALUMINUM

RSS2 POLYCARBONATE RSV2 ALUMINUM

HOUSING - Die-cast aluminum back box contains less than 0.315 copper to prevent compain in concrete or masonry applications.

TRIM RING - Die-cast aluminum from contains less than 0.3% copper to resist corrodon. Cast bronze future has polycarbonate color-in pragrated, dark bronze finish (PSC2 and RSV2 only).

FINISH - Alaminum parts have zinc-nich primer and thermodet powdertoat for durability Cast bronze his natural Inith (RSC2 and RSV2 only).

CAST LOUVER FACEPLATE - Die-cast repperfree aurnirum with 15° cutoff, with frosted tempered glass Cast bronze louver is natural cast bronze with frosted tempered grass (RSC/ and RSV2 only).

OPAL FACEPLATE - Opal UV stabilized medical molded polycarbonate with frosted glass silkoned on inside for durability.

LOUVER SCREEN FACEPLATE - Clear UV stabilized injection molded polycarbonatie with 45° ioux and screen insert.

LED - Type I Limson Inch curput LEDs powered at 350 nA DC to operate for 50,000 hours.

REFLECTOR - Specular aluminum formed reflector.

ELECTRICAL - Electronic 120 volt. AC to 12 volt regulated DC power supply.

WIRING - Supply wires connect directly to the quick connect on the integral power supply.

ADA COMPLIANT - Compliant when used in indoor and cuicloonwall applications.

UL dUL Listed

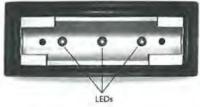


RSC2 CAST BRONZE



RSV2 CAST BRONZE





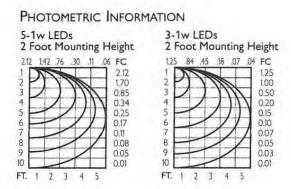
ACCESSORIES

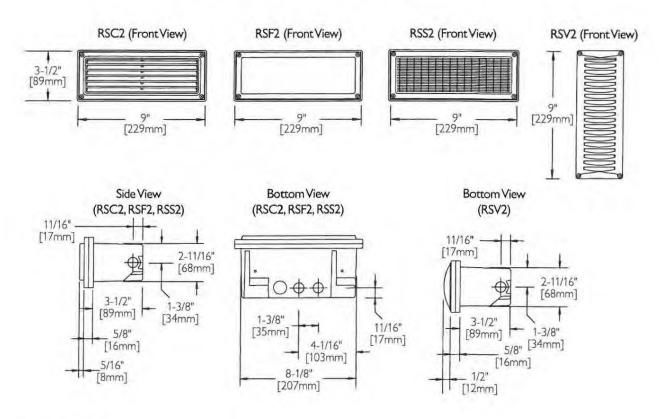
| Catalog Number | Description | |
|----------------|--|--|
| MAKM | Masonry Construction Mounting Kit | |
| MAKNS | New Stud Wall Construction Mounting Kit | |
| MAKES | Existing Stud Wall Construction Mounting Kit | |

Note: All units supplied with mounting hardware for concrete pour installation. If other mounting kits are required, order accessory above.



STRING LIGHTING - FIXTURE "B"





ORDERING GUIDE

| Catalog Number | Finish (A) Black (B) White (H) Bronze (N) Natural Cast Bronze | Sym Optics K5 or L5 | Lamping D3 (3 tw LEDs) D5 (5 tw LEDs) | Voltage (E) 120 |
|----------------|---|---------------------------------|---|--------------------|
| RSC2 | (Specify) | K5 | (Specify) | E |
| RSF2 | (A,H) | L5 | (Specify) | E |
| RSS2 | (A,H) | K5 | (Specify) | E |
| RSV2 | (Specify) | K5 | (Specify) | E |
| RS2D-AHO* | Black | Housing Only for Pre-Shipment | | |
| RS2D-BHO* | White | LED Housing Only for Pre-Shipme | nt | |
| RS2D-HHO* | Bronze | LED Housing Only for Pre-Shipme | ent | |
| RS2D-NHO* | Natural Cast Bronze | LED Housing Only for Pre-Shipme | nt | |

^{*}When housing only is ordered for pre-shipment, add suffix "LH" (Less Housing) to complete catalog number to order fixture without housing.



PAVER LIGHTING - FIXTURE "C"





TM and manufacture by MSL-SOLARPATH



Select product button for full information

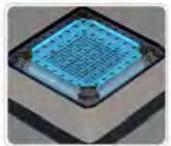
La Download Product Catalog



SolaTile II

SolaTile II is the newest generation of Solar Plus Sun Solutions light emitting tiles.

It utilizes an open surface photovoltaic array laid in a recessed chamber. LED lights, driven by clean capacitor power, illuminate a full-face illuminating plate. When the plate is lit, the entire surface of the Sola Tile II glows and the recessed solar array is hidden. Through advanced laser etching, the highlighting of emblems, logos, demarcations, symbols, pictures, and lettering on the unit surface are made possible - branding and customization possibilities are unlimited.



- Landscaping
- . Biking Paths
- Patios
- Boradwalks
- Pavers
- · Pool Decks
- Fouritains
- · Pool bottoms
- · Stairways
- Building Exteriors
- Walkways
- And many more...



Technical Specifications

Technology and Operation

Solar panel type

Illumination technology

Energy storage

Charging time

Operation at full charge

Mono-crystalline

Ultra-bright L.E.Ds

Ultra-capacitors

2 (sunny day) to 8 (cloudy day) Hours

Up to 18 hours



Lighting Cut Sheets 11.18.2010

PAVER LIGHTING - FIXTURE "C"

Solatile II

| Colors | Red, Green, Blue, Orange, Amber, White |
|------------------------------|--|
| Control of the second second | And the same of the same and th |

Operating temperature 140 F to -13 F / 60 C to -25 C

Automatic On/Off level 150 lux - 350 lux

| Color | Luminance (Nit) | | | Uniformity | | |
|--------|-----------------|------|------|------------|-----|--|
| Red | 6.8 | 6.5 | 21.6 | 20.5 | 90% | |
| Green | 17.3 | 16.4 | 54.2 | 51.4 | 90% | |
| Blue | 4.7 | 4.5 | 14.8 | 14.1 | 90% | |
| Orange | 6.3 | 5.9 | 19.9 | 18.7 | 90% | |
| Amber | 3.4 | 3.2 | 10.8 | 10.1 | 90% | |
| White | 13.2 | 12.5 | 41.5 | 39.3 | 90% | |
| | | | | | | |

Malina II on Con.,

Weight

Case Material UV-Treated polycarbonate (main body)

ABS (bottom cover)

L. Square - 8"W x 8"L x 2.54"H / W 20.32cm x L 20.32cm x H 5.45cm

S. Square - 4"W x 4"L x 2.54"H / W 10.16cm x L 10.16cm x H 6.45cm

Rectangular - 4"W x 8"L x 2.54"H / W 10.16cm x L 20.32cm x H 6.45cm

Round - Ф4" x 2.54"H / Ф10.16cm x H 6.45cm

L. Square - 5.5lbs / 2.5kg
S. Square - 1.3lbs / 0.6kg

Rectangular - 2.2lbs / 1kg

Round - 1.2lbs / 0.5kg

 Modulus of rupture
 21.8 Nimm2 (=222 kgf/cm2)

 Compressive strength
 2.74 N/mm2 (=28 kgficin2)

 Breaking load
 34.888 N (=3.560 kgf)

 Waterproof
 IP 68 - 100% (submersable)

me: - Fixture Type: - Model No: - Lamp Info:

APP LICA TIO N

Small aperture medium distribution downlight is suitable for commercial, retail and institutional applications that require an energy saving, long life LED lamp source, high lumen output and excellent color rendering characteristics.

PRODUCT DATA

REFLEC TO R: The parabolic reflector redirects spill light from the lamp, and elimi nates high angle glare. A one half-inch overlap flange is standard. Supplied standard with Specular Clear Alzak, other reflector finishes are available in order to provide maximum flexibility to the designer.

DEC ORA TIVE ACCESS ORY: Round disc is fabricated from soda lime glass (ICE4) or borosilicate glass (ICE4BS) and is suspended from reflector by satin stainless hard ware. Glass features a frosted center ring with center opening.

HOUS ING: Heavy gauge galvanized steel housing provides a secure mounting-plat form for the electrical components and protects the optical assembly. Standard plaster flange allowsone inch ceiling thickness with custom depth available. LED module is accessible from below.

MOUNTING: Universal Mounting brackets adjust vertically 5.5" and accepts C-channel or rigid bars (see optional accessories).

ELEC TRICAL: 120 to 277 VAC, 50-60 Hz.

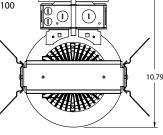
JUNC TIO N B OX: Heavy gauge galvanized junction box pre-wired with grounding pigtail. Easy access covers. Multiple conduit knockouts listed for through branch circuit wiring.

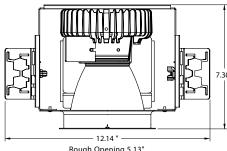
LED MODULE & DR IVER: LED Module and Driver are manufactured by Philips Lighting. Remote phosphor technology insures color consistency from fixture to fixture. DIMMING: Standard product is compatible with 0-10 volt dimming controls.

| LIGHT OUTPUT (Im) | COLOR TEMP. (K) | POWER (W) | EFFICACY (Im/W) |
|-------------------|--------------------|--------------|--------------------|
| 1100 | 3000 | 17.4 | 63.1 |
| 1100 | 3500 | 16.7 | 66.1 |
| 1100 | 4000 | 15.9 | 69.1 |

Specifications based on Fortimo LED DLM1100 by Philips Lighting after 100 hours.

 Expected lamp life to be 50,000 hours with 70% lumen maintenance wheambient temperatures do not exceed 45°C. Lower ambient temperatures yield longer lamp life.







For Wet Location Under Covered Ceiling

Catalog Number – 4VLED

Example: 4VLED11004K

| Fixture Series | Lumens | Input/Voltage | Options | | | | | | | |
|----------------------|---|----------------------|------------------------------|--|--|--|--|--|--|--|
| 4VLED | 1100 4K (4,000K) 1100 35K (3,500K) 1100 3K (3,000K) | 120/277V Standard | EM = Emergency Power Pack | | | | | | | |
| Ontional Accessories | | | | | | | | | | |

Optional Accessories

Hanger Bars (set of 2)

#101782 = #520 Caddy Bars #9152 = 52" C-Channel #9127 = 27" C-Channel 27BH = 27" Solid Bar

For Optional Reflector Finishes & Decorative elements, consult special section of catalog.

| Fixture Type | Job Reference/Location |
|--------------|------------------------|
| Lamp Type | Approval |

COVENTY Architectural Series

SERIES 4VLED1100

Recessed Downlight, 1100 Lumen LED Open Reflector



Available with suspended decorative glass disc.

Rough Opening 5.13"

4VLED

Example: 4VLEDSCLPF

Trim Trim Finish w/polished flange 4VLED - Downlight Reflector SCLPF - Soft Specular Clear Alzak HAZPF - Haze Alzak ICE4 - Decorative Glass Accessory For white painted flange, drop "PF" from Catalog #. For other finishes, consult reflector section.

Manufactured and tested to UL#1598 and CSA standards. Note: Suitable for damp location.

Fixtures are not designed for direct contact with insulation.

All Pathway products meet or exceed requirements as established by the National Electrical Code. Specifications subject to change without notice. Alzak is a registered trademark of Alcoa.



Lighting Cut Sheets 11.18.2010

Chris W. Barnes Reg. No.: 400465 Project Name: - Fixture Type: - Model No: - Lamp Info:



SERIES 4VLED1100 - 4" Aperture 1100 Lumen LED Open Downlight

PHOTOMETRIC DOWNLIGHT DISTRIBUTION

Catalog No. - 4VLED1100/4VLEDSCLPF Lamp(s) – Fortimo DLM 1100 Lumens – 1100

Spacing Criteria - 1.05 Efficiency - 85.5%

CONE OF LIGHT

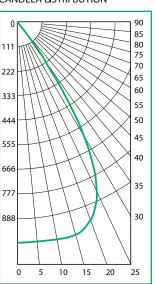
Mounting Ht. FC (Nadir) Diameter 9'.8" 8.0' 33 10.0' 17.8 12'.3" 12.0' 11.1 14'.7" 14.0' 7.6

Initial Footcandles at a 30" Work Plane Beam diameter calculated at 50% of maxi

ZONAL LUMENS SUMMARY

| ZONE | LUMENS | % LAMP | % LUMINAIRE |
|--------|--------|--------|-------------|
| 0- 30 | 746 | 67.8 | 79.3 |
| 0- 40 | 922 | 83.8 | 98.1 |
| 0- 60 | 936 | 85.1 | 99.6 |
| 0- 90 | 940 | 85.5 | 100.0 |
| 90-120 | 0 | 0.0 | 0.0 |
| 90-130 | 0 | 0.0 | 0.0 |
| 90-150 | 0 | 0.0 | 0.0 |
| 90-180 | 0 | 0.0 | 0.0 |
| 0-180 | 940 | 85.5 | 100.0 |

CANDELA DSTRI BUTION



CANDELA

| VERTICAL ANGLE | 0 |
|-------------------|-------|
| 0.00 | 999 |
| 5.00 | 997 |
| 10.00 | 999 |
| 15.00 | 998 |
| 20.00 | 956 |
| 25.00 | 847 |
| 30.00 | 606 |
| 35.00 | 258 |
| 40.00 | 53.8 |
| 45.00 | 5.81 |
| 50.00 | 2.47 |
| 55.00 | 1.46 |
| 60.00 | 1.17 |
| 65.00 | 0.830 |
| 70.00 | 0.670 |
| 75.00 | 1.14 |
| 80.00 | 1.54 |
| 85.00 | 1.73 |
| 90.00 | 1.46 |

COEFFICIENTS OF UTILIZATION

| PCC | | 3. | 3 | | | | 7 | | | .5 | | | .3 | | | .1 | | 0 |
|-----|-----|-----|-----|-----|----|----|----|----|----|----|----|----|----|----|----|----|----|----|
| PW | .7 | .5 | .3 | .1 | .7 | .5 | .3 | .1 | .5 | .3 | .1 | .5 | .3 | .1 | .5 | .3 | .1 | 0 |
| RCR | | | | | | | | | | | | | | | | | | |
| 0 | 101 | 101 | 101 | 101 | 99 | 99 | 99 | 99 | 95 | 95 | 95 | 91 | 91 | 91 | 87 | 87 | 87 | 85 |
| 1 | 97 | 95 | 93 | 91 | 95 | 93 | 91 | 89 | 89 | 88 | 87 | 86 | 85 | 84 | 83 | 82 | 81 | 80 |
| 2 | 92 | 88 | 85 | 82 | 91 | 87 | 84 | 81 | 84 | 82 | 80 | 82 | 80 | 78 | 79 | 78 | 76 | 75 |
| 3 | 88 | 83 | 79 | 76 | 86 | 82 | 78 | 75 | 80 | 76 | 74 | 77 | 75 | 73 | 76 | 73 | 72 | 70 |
| 4 | 84 | 78 | 73 | 70 | 82 | 77 | 73 | 70 | 75 | 72 | 69 | 73 | 70 | 68 | 72 | 69 | 67 | 66 |
| 5 | 80 | 73 | 69 | 65 | 79 | 72 | 68 | 65 | 71 | 67 | 64 | 70 | 66 | 64 | 68 | 65 | 63 | 62 |
| 6 | 76 | 69 | 64 | 61 | 75 | 68 | 64 | 61 | 67 | 73 | 60 | 66 | 62 | 60 | 65 | 62 | 59 | 58 |
| 7 | 73 | 65 | 60 | 57 | 72 | 65 | 60 | 57 | 64 | 59 | 56 | 63 | 59 | 56 | 62 | 58 | 56 | 55 |
| 8 | 69 | 62 | 57 | 53 | 68 | 61 | 56 | 53 | 60 | 56 | 53 | 59 | 56 | 53 | 58 | 55 | 53 | 51 |
| 9 | 66 | 58 | 53 | 50 | 65 | 58 | 53 | 50 | 57 | 53 | 50 | 56 | 52 | 50 | 56 | 52 | 50 | 49 |
| 10 | 63 | 55 | 50 | 47 | 62 | 55 | 50 | 47 | 54 | 50 | 47 | 54 | 50 | 47 | 53 | 49 | 47 | 46 |
| DEC | | 20 | | | | | | | | | | | | | | | | |

PFC = 0.20

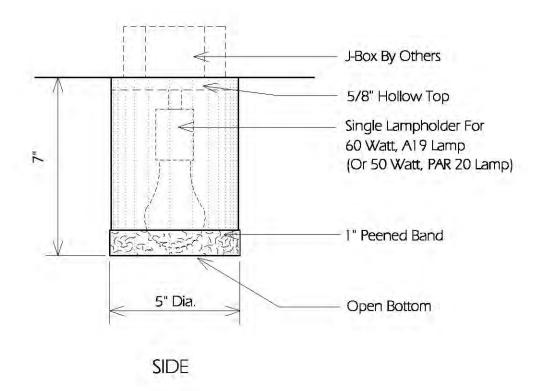
AVERAGE LUMINANCE

| 20 | | |
|-------|-------|--|
| Angle | CD/M² | |
| 45° | 800 | |
| 55° | 248 | |
| 65° | 191 | |
| 75° | 429 | |
| 85° | 1934 | |



PENDANT LIGHTING - FIXTURE "F"





TWO HILLS STUDIO®

Fine Lighting & Decorative Metal Work www.twohillsstudio.com 2706 SOUTH LAMAR BOULEVARD AUSTIN, TEXAS 78704 512-707-7571 FAX 512-707-7524

CEILING FIXTURE CF10

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ALL RIGHT'S RESERVED

Fixture Is Appropriate For: Wet Locations As Shown

Wet Locations With Glass Or Solid Top A

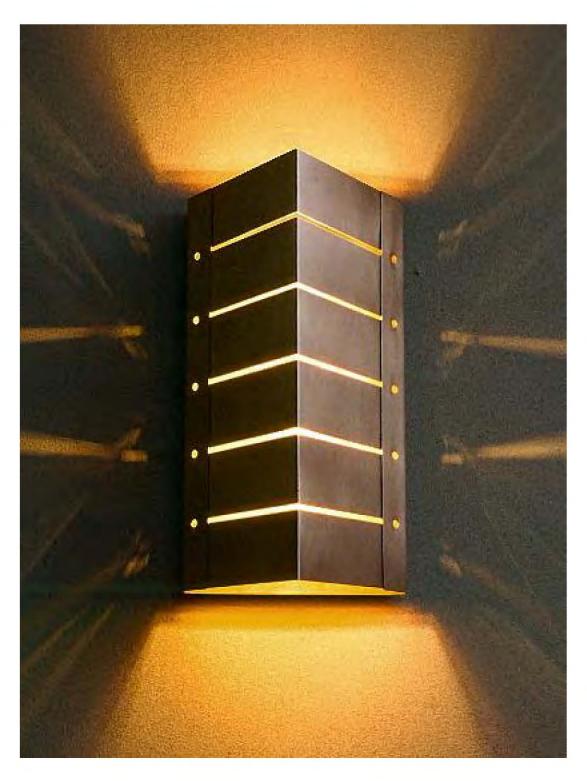
Damp Locations As Shown

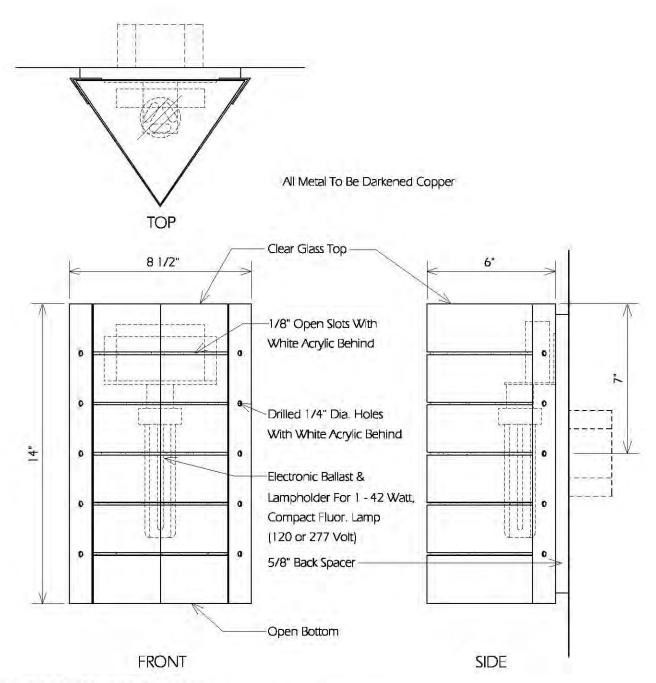
Dry Locations As Shown

U.L. Approved



WALL SCONCE - FIXTURE "G"





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FIXTURE TYPE 'G' 6 REQ'D. MOUNTAIN VILLAGE HOTEL BOKA Powell Architects

COPYRIGHT 2010 TWO HILLS STUDIO, INC. Drawing Date: 11-3-10 Revised Date:

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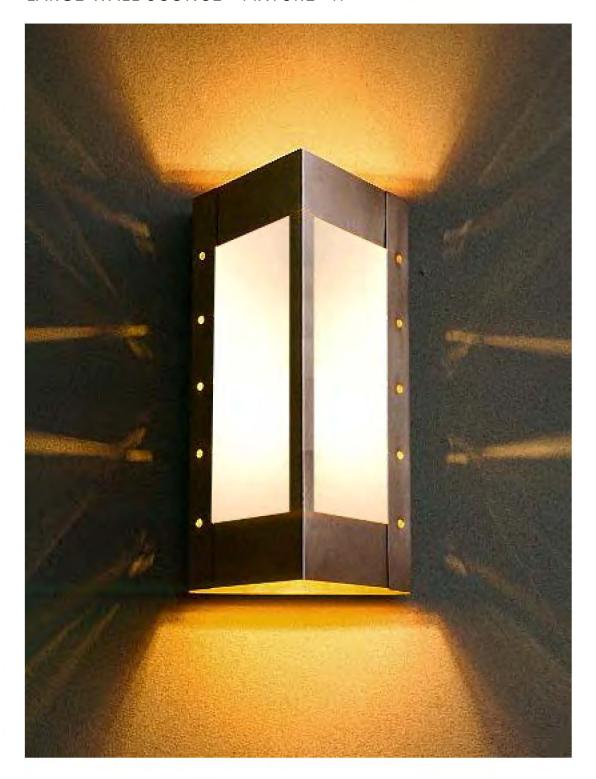
Fixture Is Appropriate For:

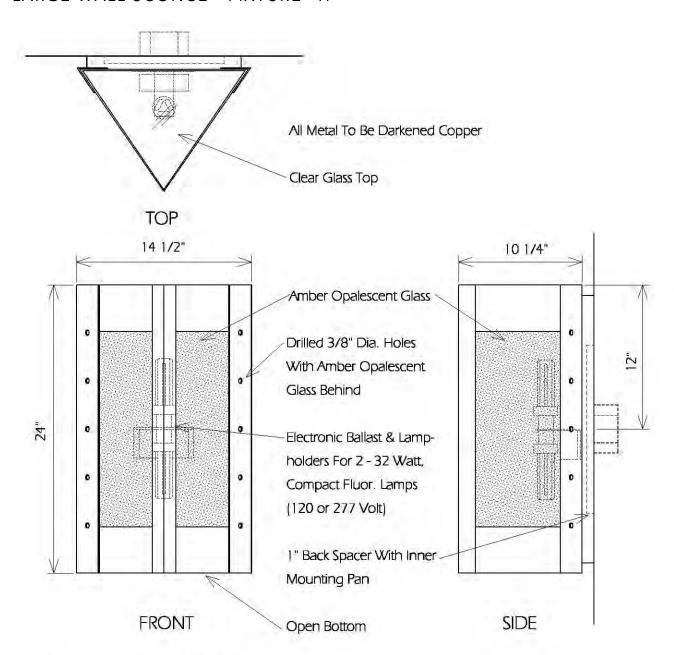
- Wet Locations As Shown
- Damp Locations As Shown
- Dry Locations As Shown
- U.L. Approved



Lighting Cut Sheets 11.18.2010

LARGE WALL SCONCE - FIXTURE "H"





TWO HILLS STUDIO®

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FIXTURE TYPE 'H' 4 REQ'D. MOUNTAIN VILLAGE HOTEL BOKA Powell Architects

COPYRIGHT 2010 TWO HILLS STUDIO, INC. Drawing Date: 11-3-10 Revised Date: 11-4-10

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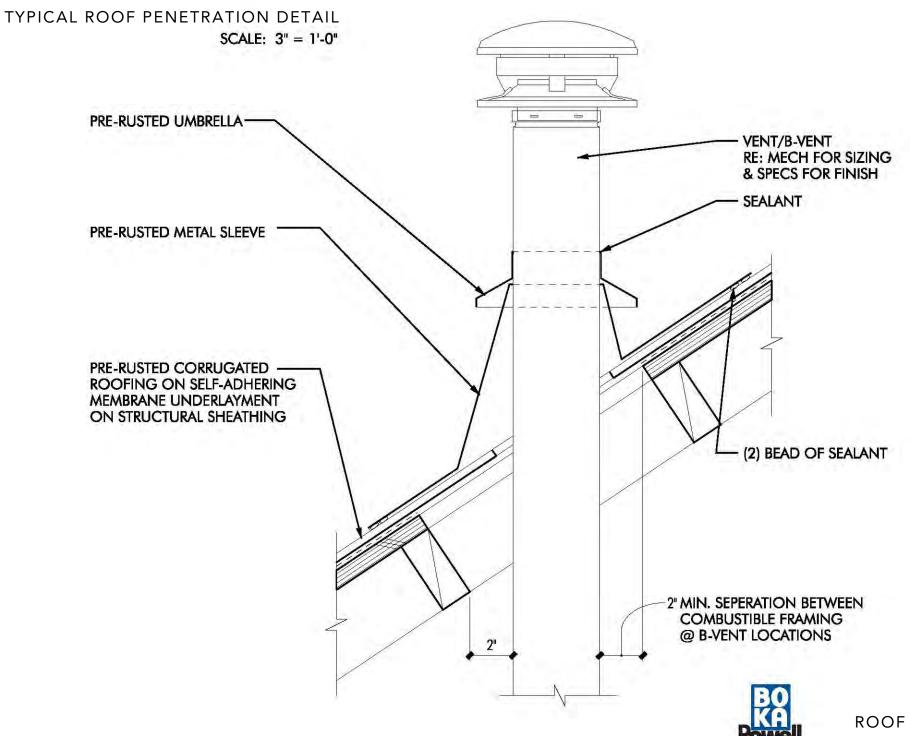
Fixture Is Appropriate For:

- Wet Locations As Shown
- Damp Locations As Shown
- Dry Locations As Shown
- ____ U.L. Approved



Lighting Cut Sheets 11.18.2010



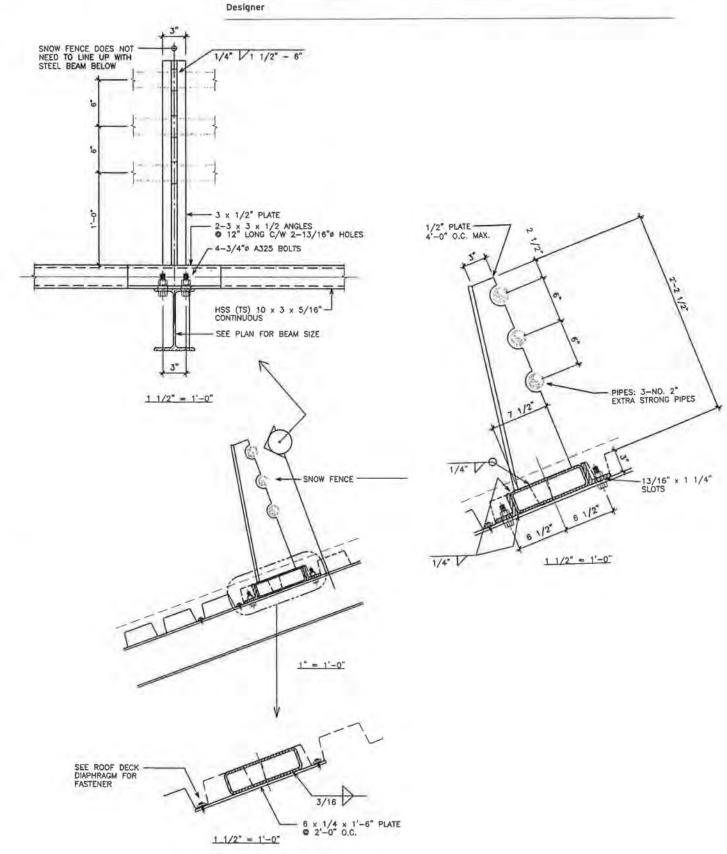


11.18.2010

Chris W. Barnes Reg. No.: 400465



| Project | Job Number | | |
|---------|------------|----|--|
| Date | Page | of | |



SNOW FENCE CONNECTION DETAIL

(SEE ARCH'L DRAWINGS FOR LOCATION)

Project Name: Mountain Village Hotel

Project Location: Mountain Village, CO

Date: 25-Aug-10

Seasonal Water Use and Peak Flow Projections Prepared By HydroSystems-KDI, Inc.

| | Irrigated | | Applicat | ion | Monthly | Maxin | num |
|---------------------|--------------------|-----------------|-----------------|-----|---------|-----------|-------|
| Description | Irrigated Acres | Month | Rate | | | Required | lFlow |
| | Acres | | (in./mon | | | (GPN | |
| Spray Irrigated | | April | 0.83 | | 8,133 | | |
| Native Seed | | May | 1.60 | | 15,587 | | |
| | | June | 2.50 | | 24,387 | | |
| | 0.36 | July | 3.19 | | 31,161 | | 3.3 |
| | | August | 2.78 | | 27,107 | | |
| | | Sept. | 1.67 | | 16,266 | | |
| | | October | 0.83 | | 8,133 | | |
| annual sub-total | | | 13.40 | /yr | 130,775 | /yr | |
| | | | | | 0.40 | acrefeet | |
| Drip Irrigated | | April | 1.00 | | 699 | | |
| Trees | | May | 1.92 | | 1,339 | | |
| | | June | 3.00 | | 2,095 | | |
| | 0.03 | July | 3.83 | | 2,677 | | 0.3 |
| | | August | 3.33 | | 2,329 | | |
| | | Sept. | 2.00 | | 1,397 | | |
| | | October | 1.00 | | 699 | | |
| annual sub-total | | | 16.08 | /yr | 11,234 | /yr | |
| | | | | | 0.03 | acrefeet | |
| Drip Irrigated | | April | 0.83 | | 1,359 | | |
| Shrubs & Perennials | | May | 1.60 | | 2,604 | | |
| | | June | 2.50 | | 4,074 | | |
| | 0.06 | July | 3.19 | | 5,206 | | 0.5 |
| | | August | 2.78 | | 4,528 | | |
| | | Sept. | 1.67 | | 2,717 | | |
| | | October | 0.83 | | 1,359 | | |
| annual sub-total | | | 13.40 | /yr | 21,846 | /yr | |
| | | | | | 0.07 | acrefeet | |
| Site Totals | 0.45 | irrigated acres | - | | 163,855 | gals/yr | |
| | | | | | 0.50 | acrefeet/ | yr |
| | | Using a | 3/4" <i>Tap</i> | | 4 | GPM - p | eak |

Projections are based on a six day per week, six hour per day, peak season watering schedule.

Peak season application for Spray Irrigated Native Seed is 0.72" per week.

Peak season application for Drip Irrigated Shrubs & Perennials is 0.72" per week.

SECTION 02515 UNIT PAVING

PART ONE: GENERAL

1.01 <u>DESCRIPTION</u>:

- A. Work Included: Furnish all labor, materials, equipment and services required for the supply and installation of precast concrete paving and edge units on a prepared sand laying course and a drainage sub-base course where indicated on the drawings:
 - 1. Colored concrete unit pavers.
 - 2. Bedding and joint sand.

1.02 STANDARDS:

- A. ASTM C 936, Standard Specification for Solid Concrete Interlocking Paving Units.
- B. ASTM C 979, Specification for Pigments for Integrally Colored Concrete

1.03 DELIVERY, STORAGE AND HANDLING:

- A. Pavers shall be delivered and stored at the work site on pallets, metal strapped in cubes, capable of transfer by fork lift or clamp lift, packaged by the paver manufacturer.
- B. Unload pavers at job site in such a manner that no damage occurs to the product.
- C. Sand shall be protected against rain, wind and snow when stockpiled on work site. Form of protection shall be secured in place.
- D. Coordinate delivery and paving schedule to minimize interference with normal use of buildings adjacent to paving.
- E. Do not install sand or pavers during heavy rain or snowfall.
- F. Do not install sand and pavers over frozen base materials.
- G. Do not install frozen sand.

PART TWO: PRODUCTS

2.01 UNIT PAVERS:

- A. As supplied by Pavestone 9401 E. 96th Ave. Henderson, CO 80630 303.287.3700 or approved equal. Provide colored samples for approval by the Landscape Architect prior to delivering the material to the site.
- B. Type 1 Pavestone, "Venetian" Stone Giant 11.75" x 11.75" x 2.375". Venetian Stone Large Rectangle 5.8125" x 11.75" x 2.375", Venetian Stone Square 5.8125" x 5.8125" x 2.375", and Medium Rectangle 5.8125" x 8.75" x 2.375", Winter Blend, random pattern, or approved equal.
- C. Type 2 Pavestone, "Venetian" Stone Giant 11.75" x 11.75" x 2.375". Venetian Stone Large Rectangle 5.8125" x 11.75" x 2.375", Venetian Stone Square 5.8125" x 5.8125" x 2.375", and Medium Rectangle 5.8125" x 8.75" x 2.375", Old Town Blend, random pattern, or approved equal.
- D. Type 3 Stonebilt Concepts Slate 12" x 24" x 1.75", color smoke, running bond pattern, or approved equal.
- E. Pavers shall meet the following requirements set forth in the ASTM C 936, Standard Specification for Interlocking Concrete Paving Units:
 - 1. Average compressive strength of 8,000 psi (55 Mpa) with no individual unit under 7,200 psi (50 Mpa).
 - 2. Average absorption of 5% with no unit greater than 7% when tested in accordance with ASTM C 140.
 - 3. Resistance to 50 freeze-thaw cycles when tested in accordance with ASTM C 67.
 - F. Pigment in concrete pavers shall conform to ASTM C 979.
 - G. Materials shall be manufactured in individual layers on production pallets.
 - H. Materials shall be manufactured to produce a solid homogeneous matrix in the produced unit.

2.02 <u>VISUAL INS</u>PECTION:

- A. All units shall be sound and free of defects that would interfere with the proper placing of unit or impair the strength or permanence of the construction.
- B. Minor cracks, incidental to the usual methods of manufacture or shipping, or resulting from customary methods of handling in shipment and delivery, shall not be deemed grounds for rejection.
- C. Any unit that is missing an inch or more from any corner or face shall be rejected.

2.03 SAMPLING AND TESTING

- A. Manufacturer shall provide access to lots ready for delivery to the Owner or his authorized representative for testing in accordance with ASTM C 936-82 for sampling of material prior to commencement of paver placement.
- B. Manufacturer shall provide a minimum of three (3) years of testing backup data showing manufactured products that meet and/or exceed ASTM C 936-82 when tested in compliance with ASTM C 140.
- C. Sampling shall be random with a minimum of nine (9) specimens per 20,000 sf per product shape and size with repeated samples taken every additional 20,000 sf or a fraction thereof.
- D. Test units in accordance with ASTM for compressive strength, absorption and dimensional tolerance. A minimum of three (3) specimens per test is required for an average value. Testing of full units is preferred.

2.04 <u>REJECTION:</u>

A. In the event the shipment fails to conform to the specified requirements, the Manufacturer may sort it, and new test units shall be selected at random by the Landscape Architect from the retained lot and tested at the expense of the Manufacturer. If the second set of test units fails to conform to the specified requirements, the entire lot shall be rejected.

2.05 EXPENSE OF TESTS:

A. The expense of inspections and testing shall be borne by the Owner.

2.06 BEDDING AND JOINT SAND:

- A. Bedding and joint sand shall consist of clean, non-plastic sand free of deleterious or foreign matter. The sand shall be natural or manufactured from crushed rock. Limestone screenings or stone dust shall not be used. When concrete pavers are subject to vehicular traffic, the sands shall be as hard as practically available.
- B. Grading of sand samples for the bedding course and joints shall be done according to ASTM C 136. The bedding sand shall conform to the grading requirements of ASTM C 33 as shown in Table 1.

Table 1
Grading Requirements for Bedding Sand
ASTM C 33

| SIEVE SIZE | PERCENT PASSING |
|------------|-----------------|
| 3/8 in. | 100 |
| No. 4 | 95 - 100 |
| No. 8 | 85 - 100 |
| No. 16 | 50 - 85 |
| No. 30 | 25 - 60 |
| No. 50 | 10 –30 |
| No. 100 | 2 - 10 |

C. Bedding sand may be used for joint sand. However, extra effort in sweeping and compacting `the pavers may be required in order to completely fill the joints. If joint sand other than bedding sand is used, the gradations shown in Table 2 are recommended. Joint sand should never be used for bedding sand.

Table 2 Grading for Joint Sand ASTM C 144

| Sieve Size | Natural Sand | |
|------------|-----------------|--|
| | Percent Passing | |
| No. 4 | 100 | |
| No. 8 | 95 - 100 | |
| No. 16 | 70 - 100 | |
| No. 30 | 40 - 75 | |
| No. 50 | 10 - 35 | |
| No. 100 | 2 - 15 | |
| No. 200 | 0 | |

2.07 EDGE RESTRAINT:

A. All edges of concrete paver installations shall be restrained with Pave Edge Flexible Paver Edge Restraint System or approved equal - Pave Tech, P.O. Box 576 Prior Lake, MN 55372, 612.226.6400 Install per manufacturers recommendations. No gaps between edges, buildings, or columns greater than 3/8 inch will be accepted. No pieces less than 1/3 of the brick size shall be used for installation.

PART THREE: EXECUTION

3.01 INSPECTION:

- A. Areas of the work to receive concrete pavers shall be examined, and unsatisfactory conditions reported to the Landscape Architect; commencement of work shall imply acceptance of conditions.
- B. The subgrade shall be well drained. Base course shall be examined for adequate compaction and uniform surface.
- C. Commencement of work shall imply acceptance of conditions.

3.02 SUBGRADE PREPARATION:

- A. Proof roll areas to receive paving prior to commencement of work. Excavate softened or loosened zones to suitable subgrade and replace with specified granular fill. Excavation of these areas shall be subject to the Landscape Architect's approval.
- B. The subgrade shall be adequately drained. All service and drainage trenches shall be properly compacted.
- C. The subgrade shall be shaped to a smooth uniform surface to within +0 to -1" of the grade and cross section required.
- D. Any unsuitable material shall be removed and replaced with approved fill or sub-base material.
- E. The subgrade shall be compacted to 95% Standard Proctor density per ASTM D 698 as determined by compaction control tests to be conducted by a qualified soil-testing firm. Higher density or compaction to ASTM D 1557 may be necessary for areas subject to continual vehicular traffic.

3.03 CONSTRUCTION OF THE SAND BED:

- A. The sand bed shall be spread evenly over the area not greater than required to receive concrete pavers in one working day. Sand shall be spread to a level of at least 2" and not exceeding the maximum thickness of 2 ½". Do not use the bedding sand to fill depressions in the base surface.
- B. Once screeded and leveled, this sand bed shall not be disturbed in any way and shall be protected against accidental pre-compaction and against rain and dew. Any sand which has been compacted shall be removed and brought back to profile in a loose condition.

3.04 LAYING OF CONCRETE PAVERS ON SAND BED:

- A. Ensure that the pavers are free of foreign materials before installation.
- B. The concrete pavers shall be laid in such a manner that the desired pattern is maintained as per detail drawing.
- C. Joints between the pavers on average shall be between 3/16 in. and 1/4 in. wide.
- D. Cut pavers to be placed along the edge with a double blade paver splitter or masonry saw.
- E. Use a low amplitude, high frequency plate vibrator to vibrate the pavers into the sand. The plate size shall be sufficient to cover at least twelve pavers. Use Table 3 below to select size of compaction equipment:

Table 3

| Paver Thickness | Minimum Centrifugal Compaction Force |
|-----------------|---|
| 60 mm | 3000 lbs (13 kN) |
| 80 mm | 5000 lbs (22 kN) |

- F. Vibrate the pavers, sweeping dry joint sand into the joints and vibrating until they are full. This will require at least two or three passes with the vibrator. Do not vibrate within 3 ft. of the unrestrained edges of the paving units.
- G. All work to within 3 ft. of the laying face must be left fully compacted with sand-filled joints at the completion of each day.
- H. The surface shall be true to elevation and shall not vary by more than 1/4" when tested with a 10' 0" straight edge at any location on the surface. Surfaces shall interface flush with adjacent materials as shown.

- I. The surface elevation of pavers shall be 1/8 in. to 1/4 in. above adjacent drainage inlets, concrete collars or channels.
- J. The re-sanding as necessary of paver joints shall be accomplished by the contractor for a period of 90 days after completion of work.
- K. Pattern for the Tumbled Concrete Paver is running bond or as indicated on drawings. Confirm alignment with Landscape Architect prior to laying the pavers. Contractor will be responsible to revised layout at his cost if prior approval has not been obtained.
- L. After removal of excess sand, check final elevations for conformance to the drawings.

3.05 <u>ADJUST AND CLEAN</u>:

- A. Surplus material shall be cleared away and removed from the work site.
- B. Excess sand or soil remaining on the paved surface shall be broomed away and removed from the work site.

3.06 LAYING OF CONCRETE PAVERS ON CONCRETE SLAB:

- A. The concrete pavers shall be laid in bedding sand on the top of the slab areas. Cut pavers to be next to the building or planter wall edges with full sized bricks along the more exposed paver band edges. Cut back pavers to ensure edge pavers are never less than 1/2 bricks.
- B. The surface shall be true to elevation and shall not vary by more than 1/8" when tested with a six-foot straight edge at any location on the surface. Surfaces shall interface flush with adjacent materials.

END OF SECTION 02515

PRODUCT DATA SHEET



ORNATE TELESCOPIC BOLLARDS

AMC ornate telescopic Bollards combine a traditional ornate appearance with the convenience to fully retract the unit to ground level to allow access for vehicles.

Manufactured from heavy duty polymer to ensure a long maintenance free lifespan with a steel inner mechanism. The standard lift assist mechanism features an integral gas strut to reduce the lifting weight of the bollard, making the lift and retract process simple and effortless.

Finished in black as standard, the bollards may be finished in ant RAL colour and we are able to apply logos and crests as required. Fixed bollards of the same design are also available to complement these bollards.











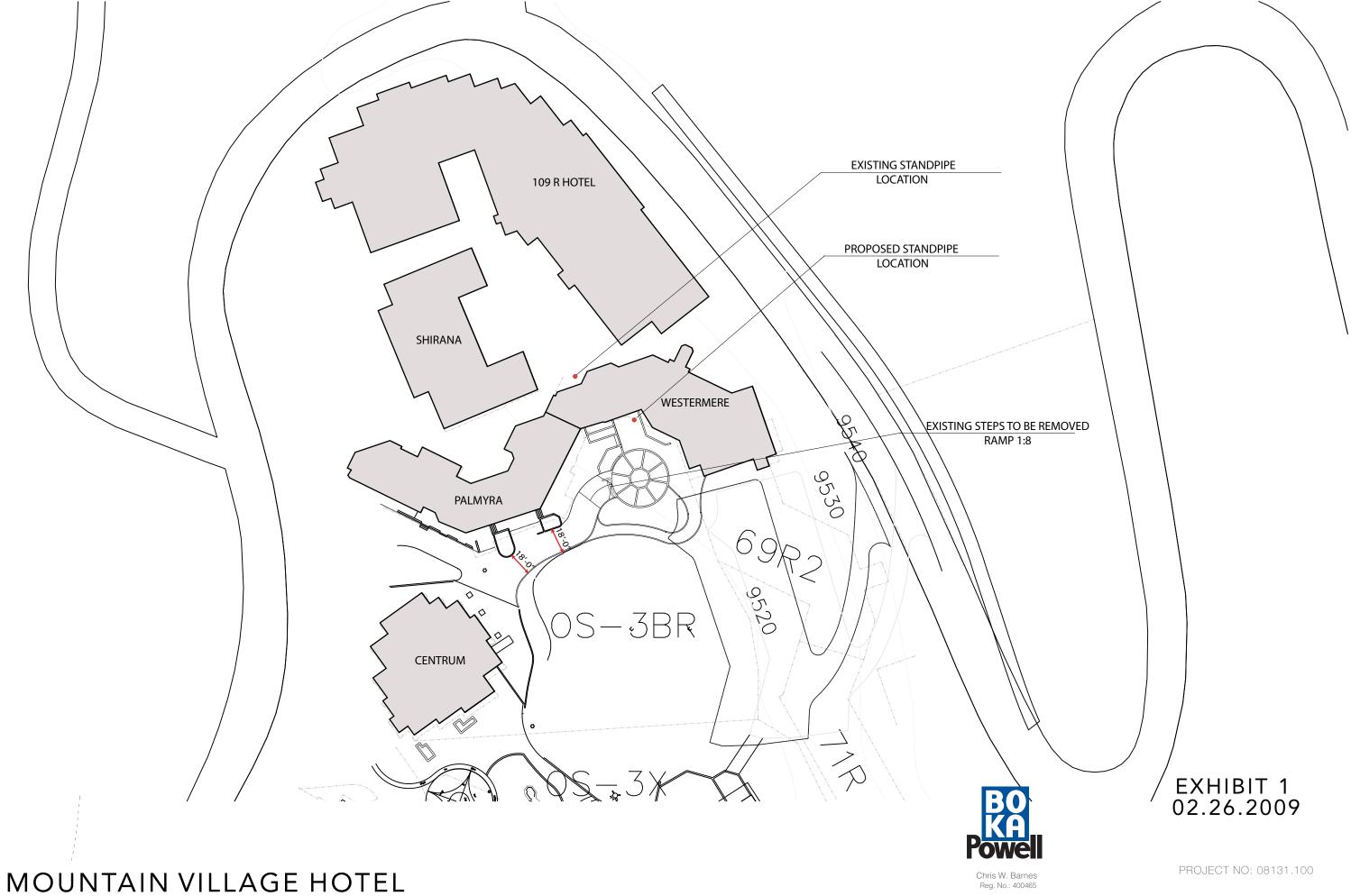
Color to match existing town of Mountain Village Bollards

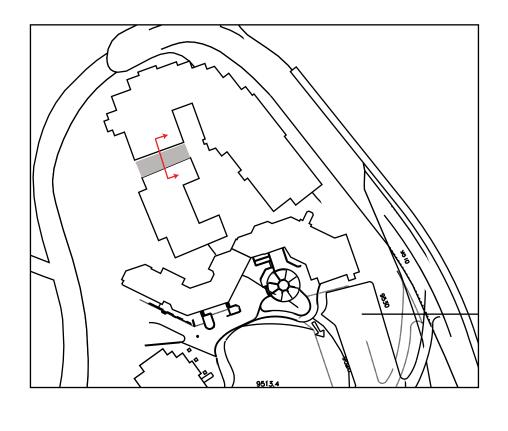
MOUNTAIN VILLAGE HOTEL



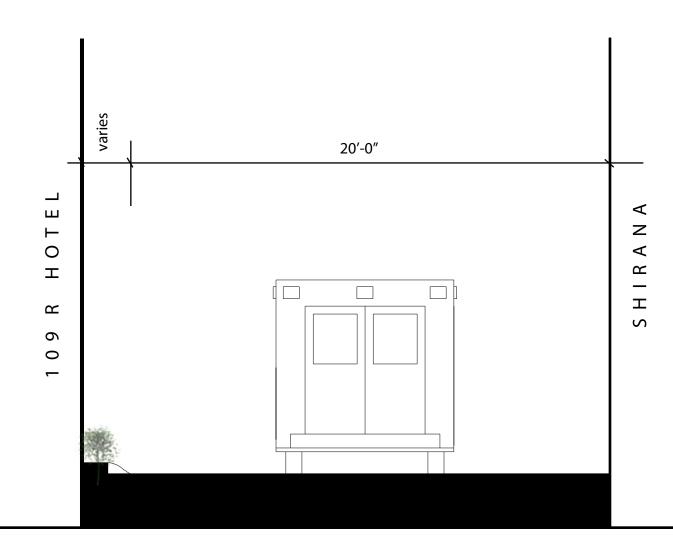
Chris W. Barnes Reg. No.: 400465 Bollard Detail 7.12.2010

Project No.: 08131.100





KEY PLAN



SECTION
SCALE 1/4" = 1'-0"



EXHIBIT 2 02.26.2009



MOUNTAIN VILLAGE HOTEL FINAL DRAINAGE REPORT 1ST SUBMITTAL

NOVEMBER 2010

For:

MV Colorado Development Partners, LLC 1601 Elm Street, Suite 400 Dallas, TX 75201



MOUNTAIN VILLAGE HOTEL FINAL DRAINAGE REPORT Page i of ii

ENGINEER'S STATEMENT

This report and plan for the final drainage design of Mountain Village Hotel was prepared by me (or under my direct supervision) in accordance with the provisions of Town of Mountain Village Technical Criteria for the owners thereof. I understand that the Town of Mountain Village does not and will not assume liability for drainage facilities designed by others.

Todd A Johnson, P.E.

Date

State of Colorado No. 37660

For and on behalf of Calibre Engineering, Inc.

This report is subject to change at such time when the site construction plans are completed.

MOUNTAIN VILLAGE HOTEL

FINAL DRAINAGE REPORT

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MOUNTAIN VILLAGE HOTEL FINAL DRAINAGE REPORT

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1.0 SCOPE & PURPOSE

The purpose of this Final Drainage Report is to provide final drainage calculations to support the Final PUD documents for the Mountain Village Hotel in Mountain Village, Colorado. This study relies and is based on information provided by the Town of Mountain Village regarding the existing storm sewer system and offsite basin runoff. This study includes runoff calculations for an approximate offsite basin area and the site. This report is subject to change at such time when final site construction plans are prepared.

2.0 GENERAL LOCATION AND DESCRIPTION

2.1 Location

• The project is located within the Town of Mountain Village, Colorado.

2.2 Description of Property

- The site is approximately 0.83 acres in size.
- It is surrounded by Mountain Village Boulevard on the north, west and east sides of the property.
- The existing condominium properties of Shirana, Palmyra and Westermere are to the south of the site.
- The site is currently partially developed with an asphalt driveway and parking lot and concrete sidewalks.
- Numerous utilities cross the property.
- The site has soils of Hydrologic Soils Group B.
- An aerial map is provided in Appendix A.

2.3 Proposed Development

• The project development consists of a hotel with underground parking surrounded by a plaza and landscaped areas.

3.0 HISTORIC DRAINAGE

3.1 Basin Description

- In general, the site drains from east to west.
- Mountain Village Boulevard drains from the southeast point of the property along the street north of the property to the southwest point of the property.
- Grate area inlets currently pick up flows generated in the onsite basin (A).
 Runoff is then carried in the existing system to an outfall location southwest of the site.
- The existing storm system also routs runoff from offsite basins (OS-1& OS-2) through the site.
- The offsite basins were delineated using an overall contour map provided by the Town of Mountain Village.



MOUNTAIN VILLAGE HOTEL

FINAL DRAINAGE REPORT

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- The existing imperviousness of the offsite basins were estimated using the aerial and existing zoning maps of the area.
- It is assumed that the Westermere property to the south of the site drains to the Village Pond to the south and does not affect the Mountain Village Hotel site.
- The offsite basins have soils of Hydrologic Soils Group B.
- See Appendix A for the Existing Drainage Basins Exhibit, DR1.

4.0 DESIGN CRITERIA

4.1 References

- The Town of Mountain Village does not have clear criteria regarding stormwater facility design, therefore the following references were used as a basis of design:
- The Colorado Water Conservation Board's *Colorado Floodplain and Stormwater Criteria Manual* (Reference 1).
- The Urban Drainage and Flood Control District's (UDFCD) *Urban Storm Drainage Criteria Manual* (Reference2).

4.2 Hydrologic Criteria

- The 5-year recurrence is the Minor Storm Event.
- The 100-year recurrence is the Major Storm Event.
- Rainfall was determined using the NOAA Atlas 2 for Colorado Region 2:
 - The 6-hr and 24-hr point precipitation values for the 2-year and 100-year storm events were estimated from Figures CH9-F401 to CH9-F412.
 - o Empirical equations for region 2 were then used to find the 1-hr point precipitation values for the 2-year and 100-year storm events.
 - o These values were then plotted on Figure CH9-F413 to determine return periods between the 2-year and 100-year events.
 - o 5-year: 0.84 inches per hour
 - o 100-year: 1.78 inches per hour
- The Rational Method has been used to calculate runoff.
- Percent Imperviousness and Runoff Coefficients were estimated from the UDFCD *Drainage Criteria Manual*.
- Hydrograph routing and the Empirical Formula (V=KA) were used to calculate detention volumes.
- The Mountain Village Hotel site will provide detention for its site only.
- The Mountain Village Hotel will not be responsible for providing detention or water quality for any offsite runoff.
- Copies of applicable figures and tables are included in Appendix E.
- See Appendix D for Hydraulic Calculations.

4.3 Hydraulic Criteria

• The hydraulic capacity of the existing system was analyzed in order to ensure adequate capacity to tie in the proposed system.



MOUNTAIN VILLAGE HOTEL

FINAL DRAINAGE REPORT

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- The proposed system has been designed such that the hydraulic grade line is within the pipe for the Minor Storm Event and within the ground for the Major Storm Event.
- Hydraulic Calculations will be included when construction plans are prepared

5.0 DRAINAGE PLAN

5.1 General Concept

• The proposed drainage patterns will follow the existing drainage patterns as closely as possible.

5.2 Specific Details

- Basin Descriptions
- Detention
- The detention basin has been sized such that the historic runoff in the 100year Major storm event is not exceeded. This resulted in a detention volume of approximately 600 cu-ft.
- The detention volume will be provided for onsite.

6.0 CONCLUSIONS

- Supporting calculations and maps are attached to this summary. With the development of the Mountain Village Hotel, the imperviousness increased from approximately 54% to 83%.
- The result of the increase in imperviousness shows approximately 1.1 cfs additional runoff for the minor storm and 1.8 cfs for the major storm with proposed conditions.
- The capacity of the existing system is adequate to convey runoff from The Mountain Village Hotel.

7.0 REFERENCES

- 1. Colorado Floodplain and Stormwater Criteria Manual, Colorado Water Conservation Board, January 2006.
- 2. *Urban Storm Drainage Criteria Manual*, Urban Drainage and Flood Control District, Revised August 2006.
- 3. Town of Mountain Village, Colorado, Design Regulations, Amended and Restated as of March 8, 2005.



APPENDIX A MAPS & EXHIBITS

37° 56' 18''

107° 50' 43"

37° 56' 22''

107° 50' 43"

Soil Properties and Qualities—Ouray Area, Colorado, Parts of Gunnison, Hinsdale, Ouray, San Juan, and San Miguel Counties (Mountain Village Hotel Site)

102. 20. 20.

37° 56' 21"

37° 56' 18"

107° 50' 50"





MAP INFORMATION

Map Scale: 1:1,460 if printed on A size (8.5" × 11") sheet.

The soil surveys that comprise your AOI were mapped at 1:24,000.

Area of Interest (AOI)

Area of Interest (AOI)

Soil Map Units

Soils

MAP LEGEND

Please rely on the bar scale on each map sheet for accurate map measurements.

Web Soil Survey URL: http://websoilsurvey.nrcs.usda.gov Coordinate System: UTM Zone 13N NAD83 Source of Map: Natural Resources Conservation Service

This product is generated from the USDA-NRCS certified data as of the version date(s) listed below.

Soil Survey Area: Ouray Area, Colorado, Parts of Gunnison, Hinsdale, Ouray, San Juan, and San Miguel Counties

Survey Area Data: Version 2, Feb 1, 2008

The orthophoto or other base map on which the soil lines were compiled and digitized probably differs from the background Date(s) aerial images were photographed: 7/19/2005

imagery displayed on these maps. As a result, some minor shifting

of map unit boundaries may be evident.

USDA

Hydrologic Soil Group

| Hydrologic Soil Gro | up— Summary by Map Unit — O and S | uray Area, Colorado, Pa San Miguel Counties | rts of Gunnison, Hir | nsdale, Ouray, San Juan, |
|-------------------------|---|--|----------------------|--------------------------|
| Map unit symbol | Map unit name | Rating | Acres in AOI | Percent of AOI |
| 169 | Washboard-Rock outcrop complex, 20 to 90 percent slopes, very stony | В | 4.9 | 100.0% |
| Totals for Area of Inte | erest | | 4.9 | 100.0% |

Description

Hydrologic soil groups are based on estimates of runoff potential. Soils are assigned to one of four groups according to the rate of water infiltration when the soils are not protected by vegetation, are thoroughly wet, and receive precipitation from long-duration storms.

The soils in the United States are assigned to four groups (A, B, C, and D) and three dual classes (A/D, B/D, and C/D). The groups are defined as follows:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

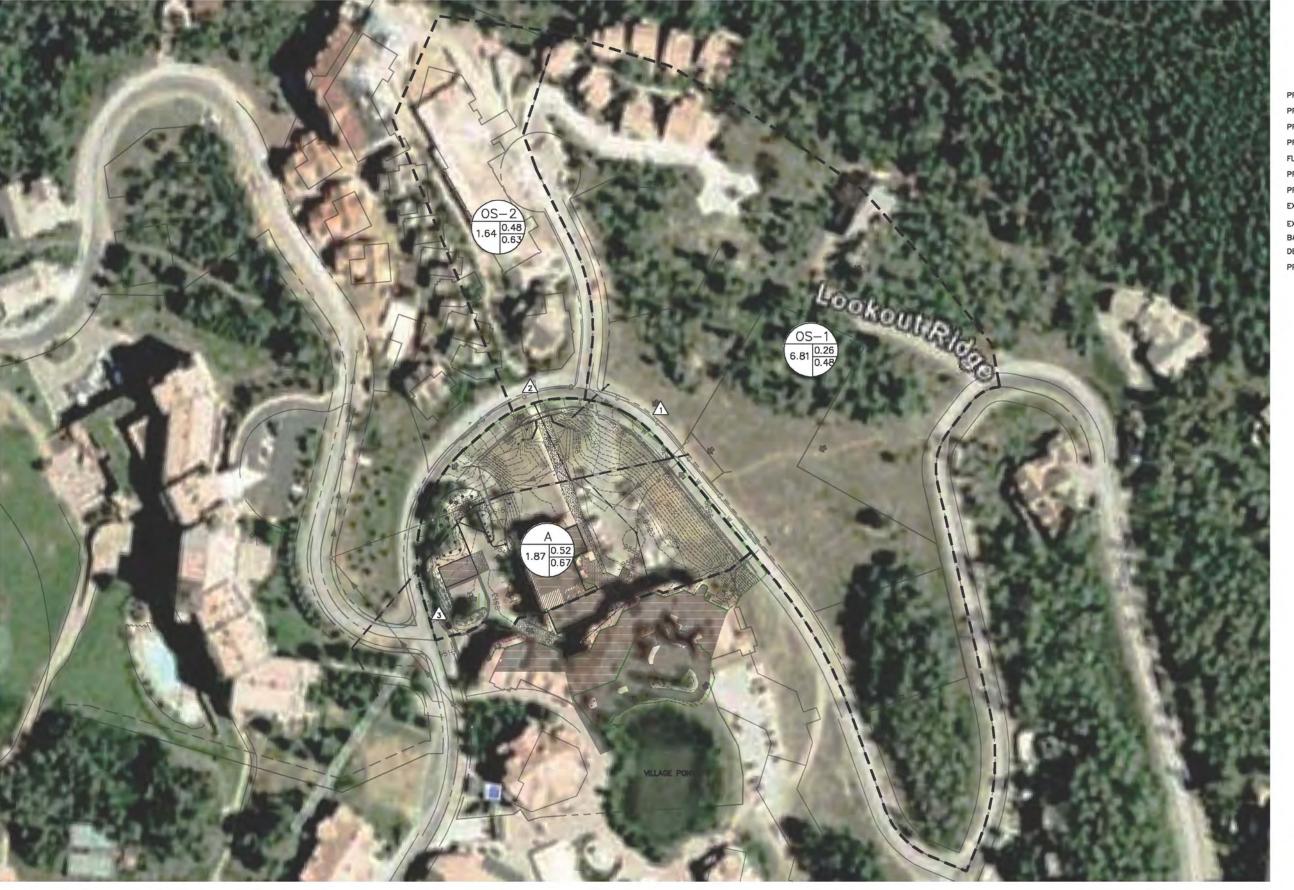
Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

If a soil is assigned to a dual hydrologic group (A/D, B/D, or C/D), the first letter is for drained areas and the second is for undrained areas. Only the soils that in their natural condition are in group D are assigned to dual classes.

Rating Options

Aggregation Method: Dominant Condition

Component Percent Cutoff: None Specified



LEGEND

PROPOSED STORM DRAIN PIPE
PROPOSED STORM DRAIN INLET
PROPOSED SWALE
PROPERTY LINE
FLOW ARROW
PROPOSED MAJOR CONTOUR (5')
PROPOSED MINOR CONTOUR (1')
EXISTING MINOR CONTOUR (1')
EXISTING MAJOR CONTOUR (5')
BASIN LINE
DESIGN POINT
PROPOSED BASIN LABEL

AREA (AC.) C-6
BASIN DESIGNATION

MINOR RUNOFF COEF.

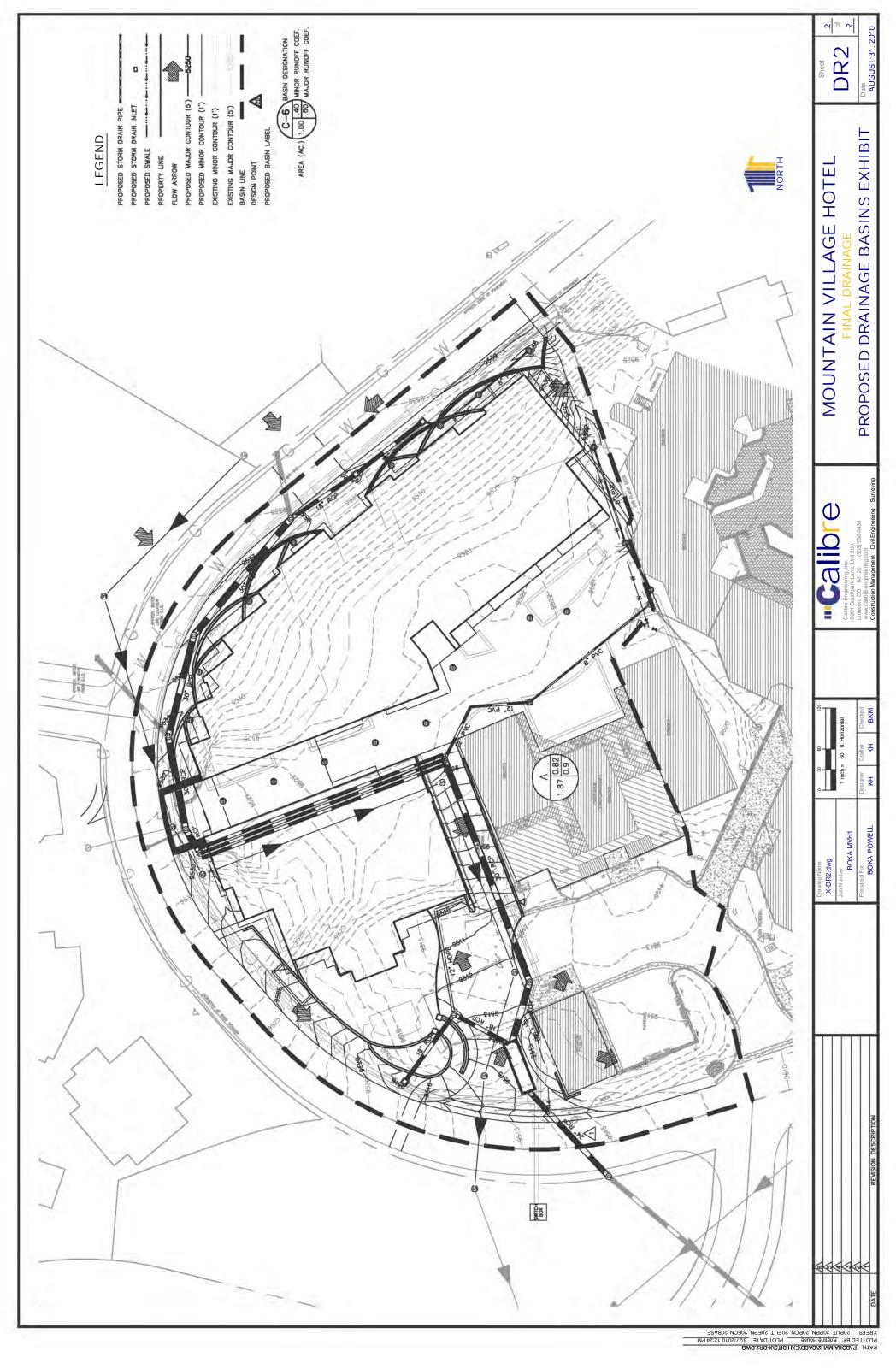
MAJOR RUNOFF COEF.

1 NORTH



MOUNTAIN VILLAGE HOTEL
FINAL DRAINAGE
EXISTING DRAINAGE BASINS EXHIBIT





APPENDIX B HYDROLOGIC CALCULATIONS



| | | | | | | | | | | CO | COMPOSITE 'C' FACTORS | <u>.</u> С | FACTC | SS | | | | | | | | | | | |
|----------------------------------|--------|-----------|---------|---------------|--------------------------------------|------|------|----|-------|---------|-----------------------------------|---------------|-----------|----|--------|--------|----|--------|--------|----|-------------|-------|-----------------|----------------|--------|
| LOCATION: Mountain Village Hotel | Mounta | ain Villa | ge Hote | - | | | | | CIJ | 7 OF: 1 | CITY OF: Town of Mountain Village | Mounta | าin Villa | ge | | | | | | | | D | DATE: 8/27/2010 | 8/27/2 | 010 |
| BASIN | | | AREAS (| AREAS (ACRES) | | | SOIL | | UNDEV | | 4 | PAVED | | | LAWNS | | - | ROOFS | | 2 | MU-DET | | COMP | COMP. C FACTOR | JR. |
| DESIGNATION | UNDEV | PAVED | LAWNS | ROOFS | UNDEV PAVED LAWNS ROOFS MU-DET TOTAL | | TYPE | 1% | 5 YR | 100 YR | 1% | 5 YR | 100 YR | 1% | 5 YR 1 | 100 YR | 1% | 5 YR 1 | 100 YR | 1% | 5 YR 100 YR | 00 YR | 1% | 5 YR 1 | 100 YR |
| EXISTING | | | | | | | | | | | | | | | | | | | | | | | | | |
| OS-1 | 4.84 | 1.03 | 0.00 | 0.14 | 0.80 | 6.81 | В | 2 | 0.09 | 0.36 | 100 | 0.90 | 96.0 | 0 | 0.08 | 0.35 | 06 | 0.73 | 0.81 | 09 | 0.41 | 0.56 | 25 | 0.26 | 0.48 |
| OS-2 | 0.00 | 0.28 | 0.29 | 0.23 | 0.84 | 1.64 | В | 2 | 0.09 | 0.36 | 100 | 0.90 | 96.0 | 0 | 0.08 | 0.35 | 90 | 0.73 | 0.81 | 09 | 0.41 | 0.56 | 09 | 0.48 | 0.63 |
| ٧ | 00:00 | 0.86 | 0.84 | 0.17 | 00:00 | 1.87 | В | 2 | 60.0 | 0.36 | 100 | 06.0 | 96.0 | 0 | 80.0 | 0.35 | 06 | 0.73 | 0.81 | 09 | 0.41 | 0.56 | 54 | 0.52 | 0.67 |
| | | | | | | | | | | | | | | | | | | | | | | | | | |
| PROPOSED | | | | | | | | | | | | | | | | | | | | | | | | | |
| А | 0.00 | 0.00 0.80 | 0.24 | 0.83 | 0.00 | 1.87 | В | 2 | 0.09 | 0.36 | 100 | 0.90 | 96.0 | 0 | 0.08 | 0.35 | 06 | 0.73 | 0.81 | 09 | 0.41 | 0.56 | 83 | 0.72 | 0.82 |

| "Calibre | pre | A N | | | | | | | | | | | | | |
|----------------------------------|------------|-------------|-------------|--------------------------|------------|-----------------|------------------|------------------|--------------|----------|-------------|------------|----------------------------|-----------|--|
| | | | | | TIME | | OF CONCENTRATION | ATION | | | | | | | REMARKS |
| LOCATION: Mountain Village Hotel | Mounta | าin Village | Hotel | | | Final | | | BY: KH | ΉΣ | | DATE: 8 | DATE: 8/27/2010 | | FORMULAS: |
| BASI | BASIN DATA | | /O/:TINI | INIT./OVERLAND TIME (Ti) | ME (Ti) | | TRA | TRAVEL TIME (Tt) | (Tt) | | TOTAL T | c Check (L | Tc Check (Urbanized Basins | FINAL TC | * Ti = 0.395 (1.1-C5)L^0.5/S^1/3 |
| DESIGNATION | CS | AREA (AC) | LENGTH (FT) | SLOPE % | Ti (Min.)* | GRASS/ PAVED | LENGTH (FT) | SLOPE % | VEL. (FPS)** | Tt(Min.) | Ti+Tt(Min.) | LGTH. (FT) | Tc = (L/180) + 10 | (minutes) | ** V=Cv(Sw^1/2) |
| EXISTING | | | | | | | | | | | | | | | where Cv=15 for grassed waterways and 20 for paved areas |
| OS-1 | 0.26 | 6.81 | 160 | 25.0 | 6.6 | GRASS | 1250 | 8.0 | 4.2 | 4.9 | 11.5 | 1410.00 | 17.8 | 12 | |
| 08-2 | 0.48 | 1.64 | 110 | 25.0 | 4.1 | PAVED | 455 | 8.0 | 5.7 | 1.3 | 5.4 | 565.00 | 13.1 | 5 | |
| A | 0.52 | 1.87 | 100 | 16.0 | 4.2 | GRASS | 250 | 6.0 | 3.7 | 1.1 | 5.4 | 350.00 | 11.9 | 5 | |
| | | | | | | | | | | | | | | | |
| PROPOSED | | | | | | | | | | | | | | | |
| 4 | 0.72 | 1.87 | 20 | 2.0 | 2.5 | PAVED | 099 | 5.0 | 4.5 | 2.5 | 4.9 | 680.00 | 13.8 | 2 | |



STORM DRAINAGE SYSTEM DESIGN (RATIONAL METHOD PROCEDURE)

DESIGN STORM: 5-YEAR

Calc. by: KH

REMARKS Town of Mountain Village <u>Date:</u> 8/27/2010 Chk'd by: BKM **TRAVEL TIME Tt** -LOW IN STREET 2.7 3.8 2.2 -COM IN DIDE (cfs) Q (sfs) TOTAL RUNOFF AD mu2 (.nd\.ni) Final (.nim) aT mu2 Sum AREA 3.8 3.8 2.2 2.7 (cfs) D 2.85 2.79 2.79 2.14 (.nd \.ni) 1.35 0.79 0.97 C*AMountain Village Hotel DIRECT RUNOFF (.niM) oT 12 2 2 2 0.72 0.26 0.52 COEFF. (C) 1.64 1.87 6.81 1.87 (DA) A3AA PROPOSED **EXISTING OS-2 OS-1** NISA8 ⋖ ⋖ LOCATION: DESIGN POINT က 2 STRUCTURE



STORM DRAINAGE SYSTEM DESIGN

(RATIONAL METHOD PROCEDURE)

DESIGN STORM: 100-YEAR

Calc. by: KH

REMARKS 8/27/2010 Chk'd by: BKM Town of Mountain Village <u>Date:</u> TRAVEL TIME Tŧ **FLOW IN STREET** 15.0 9.2 6.1 **EFOM IN DIDE** (cfs) Q (sfo) TOTAL RUNOFF AD mu2 (.nd\.ni) Final Sum Tc (min.) Sum AREA 15.0 9.2 6.1 (cfs) D 5.92 6.04 4.54 5.91 (.nd \.ni) 3.29 1.03 1.26 1.52 C_*V Mountain Village Hotel DIRECT RUNOFF 12 (.niM) oT 2 2 2 0.48 0.82 0.63 0.67 COEFF. (C) 6.81 1.64 1.87 1.87 (AC) A3AA PROPOSED EXISTING **OS-2** OS-1 NISA8 ⋖ LOCATION: **DESIGN POINT** က N STRUCTURE

APPENDIX C POND CALCULATIONS

DETENTION VOLUME BY THE MODIFIED FAA METHOD (See USDCM Volume 2 Storage Chapter for description of method)

Project: Basin ID:

(For catchments less than 160 acres only. For larger catchments, use hydrograph routing method) (NOTE: for catchments larger than 90 acres, CUHP hydrograph and routing are recommended)

| Design from the control of the con | | | | | | | | | | | | | | |
|--|---------------|--------------------|------------------|------------------|-----------------|--------------------|-----------------|----------------|-------------------|------------------|------------------|---------------|--------------------|----------------|
| Price E8.00 Percent Cardenier Distriction Price Pr | Design Ir | formation (In | out): | | | | | Design Inf | ormation (In | point): | | | | |
| The bold The bold | Catchment E | rainage Imperviou | sseust | a - | | percent | | Catchment Dr. | ainage Imperviou | Isness | _e = e | 83.00 | percent | |
| Type B A B C of D Protein-signment MFCS Sol Gloup Type B Protein-signment MFCS Sol Gloup Trans Type B Protein-signment MFCS Sol Gloup Trans Type To To B To To To To To | Catchment D | rainage Area | | A | | acres | | Catchment Dr. | ainage Area | | A = A | 1.870 | acres | |
| Te 5 | Predevelopn | nent NRCS Soil G | dno | Type = | | A, B, C, or D | | Predevelopme | ant NRCS Soil Gr | roup | Type = | В | A, B, C, or D | |
| Coefficient Company Company Coefficient Coeffici | Return Perio | d for Detention Cc | untrol | | | years (2, 5, 10, 2 | 25, 50, or 100) | Return Period | for Detention Co | untrol | =_ | | years (2, 5, 10, 2 | 5, 50, or 100) |
| The Cry Property Control of C | Time of Con | centration of Wate | srshed | Tc= | | minutes | | Time of Conc | entration of Wate | rshed | Tc= | 5 | minutes | |
| Table Cut Cu | Allowable Ur | nit Release Rate | | = 0 | | cfs/acre | | Allowable Unit | Release Rate | | = 0 | | cfs/acre | |
| Conficient Tree Conficient | One-hour Pri | ecipitation | | - <u>P</u> | | inches | | One-hour Pre | cipitation | | P = | 1.78 | inches | |
| Conficient Trees Conficient | Design Rain | Mall IDF Formula | i = C,* P,//C,+T | ري. | | 1 | | Design Rainf | all IDF Formula | i = C,* P,//C,+T | ې. | | _ | |
| This paper Company C | Coefficient C | One | | | | | | Coefficient Or | ЭС | | | | | |
| Comparison Cy E. Cy Cy Cy Cy Cy Cy Cy C | Coefficient 1 | _wo | | C ₂ = | | | | Coefficient Tv | O/ | | C ₂ = | | | |
| Think Rail | Coefficient 1 | Three | | ပိ | | | | Coefficient Th | тее | | C3 | | | |
| Thirty Apparent | Determin | ation of Avera | | from the Ba | sin (Calcula | ted): | | Determina | tion of Avera | age Oufflow | from the Ba | sin (Calculat | ed): | |
| High Rain Cupul | Rimoff Coef | ficient | | C | | | | Runoff Coeffie | pient | | C | | | |
| Circle 2.69 GTs Albovable Peak Outflow Rate Moof FAA Major Storage Volume = 15.00 Cubic feat cubic feat (activation) Albovable Peak Outflow Rate Albovable Rate (a.g. 6) of SMInnutes) Albovable Rate (a.g. 6) of SMInnute | Inflow Peak | Runoff | | O ui-aC | | cfs. | | Inflow Peak R | unoff | | = ui-aO | 81.8 | cfs | |
| Act FAA Minor Storage Volume = 300 Cubic feet Mood FAA Major Storage Volume = 300 Cubic feet Acre-It accounts (All Major Storage Volume = 10074) Storage Volume = 10074 Acre-It accounts (All Major Storage Volume = 10074) Acri-It accounts (All Major Storage Volume = 10074) Acri-It accounts (All Major | Allowable De | ak Oufflow Rate | | 1 to 0 | |)) | | Allowable Dea | A Cufflow Rate | | 1 400 | 7 39 | ٤ | |
| Mod. FAA Mirot Storage Volume Control of | | Mon | 1. FAA Minor Sto | rade Volume = | | cubic feet | | | Mod | 1. FAA Maior Sto | rade Volume = | 592 | cubic feet | |
| Cartier Rainfal Duration Increases Value Here (e.g. 5) or 5-Mindes Activates Activat | | Moc | 1. FAA Minor Sto | rage Volume = | | acre-ft | | | Moc | 1. FAA Major Sto | rage Volume = | 0.014 | acre-ft | |
| Rainfall Inflow Adjustment Average Outflow Storage Parinfall Inflow Adjustment Average Outflow Storage Duration Inflowed | - | <- Fnter Rainfall | Duration Increm | ental Increase V | alue Here (e.g. | for 5-Minutes) | | | | | | | | |
| Interestity Volume Factor Outfllow Volume Volume Interestity Volume Factor Outflow Volume Volume Interestity Volume Factor Outflow O | Rainfall | Rainfall | Inflow | Adjustment | | Outflow | Storage | Rainfall | Rainfall | Inflow | Adjustment | Average | Outflow | Storage |
| Cution C | Duration | Intensity | Volume | Factor | Outflow | Volume | Volume | Duration | Intensity | Volume | Factor | Outflow | Volume | Volume |
| Cutiput Cutiput | minutes | inches / hr | cubic feet | "u | cts | cubicfeet | cubic feet | minutes | inches / hr | cubic feet | <u>"</u> E | cfs | cubic feet | cubic feet |
| 1000 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 | (input) | (o utput) | (o utput) | (ontbnt) | (ontbnt) | (o utput) | (o ntbnt) | (input) | (ontbnt) | (o atbat) | (output) | (ontbnt) | (output) | (o utput) |
| 3.61 2.65 1.00 2.69 162 94 1 7.65 627 1.00 7.39 443 3.37 476 1.00 2.69 323 163 2 7.44 1,170 7.39 443 3.36 6.47 1.00 2.69 486 197 4 6.32 2.072 1.00 7.39 1.373 2.88 6.44 1.00 2.69 486 197 4 6.32 2.072 1.00 7.39 1.373 2.89 9.44 1.00 2.69 486 197 4 6.32 2.072 1.00 7.39 1.373 2.69 1.67 0.89 2.47 869 2.97 7 6.07 7.39 1.773 1.773 2.69 1.67 0.89 2.47 869 2.97 7 6.39 2.443 8 6.19 2.443 8 6.19 2.453 1.773 1.773 1.773 1 | 0 | 0.00 | 0 | 00:00 | 00:00 | 0 | 0 | 0 | 00:0 | 0 | 00:00 | 00:00 | 0 | 0 |
| 3.37 476 1.00 2.69 32.3 153 2 7.14 1.170 1.00 7.39 886 2.86 844 1.00 2.69 646 197 6 6.07 1.647 1.00 7.39 1.730 2.83 844 1.00 2.69 646 191 6 5.99 2.453 1.00 7.39 1.730 1.730 2.83 1.99 1.00 2.69 808 191 6 5.99 2.453 1.00 7.39 1.773 1.730 1.720 1.730 1.720< | - | 3.61 | 255 | 1.00 | 2.69 | 162 | 94 | - | 7.65 | 627 | 1.00 | 7.39 | 443 | 183 |
| 3.16 671 1.00 2.66 485 186 3 6.70 1647 100 7.39 1.330 1.300 2.69 646 197 4 6.32 2.072 1.00 7.39 1.773 1.773 1.00 2.69 808 191 5.69 2.69 2.47 889 2.50 6 5.69 2.797 1.00 7.39 2.216 1.773 1.384 0.92 2.47 889 2.50 6 5.69 2.797 0.92 6.77 2.438 2.269 2.45 1.384 0.92 2.47 889 2.50 6 5.69 2.797 0.92 6.77 2.438 2.269 2.45 1.384 0.88 2.31 0.98 2.47 3.99 0.81 0.88 2.31 0.88 2.29 2.45 0.81 0.88 0.78 0.81 0.88 0.78 0.88 0.78 0.88 0. | 2 | 3.37 | 476 | 1.00 | 2.69 | 323 | 153 | 2 | 7.14 | 1,170 | 1.00 | 7.39 | 988 | 283 |
| 2.98 B44 1,00 2.69 646 197 4 6.32 2,072 1,00 7.39 1,773 2.83 999 1,00 2.69 808 191 5 6.59 2,433 1,00 7.39 2,216 2.69 1,139 0.02 2.47 889 2.50 6 6.69 2,797 0.92 6.77 2,438 2.66 1,139 0.08 2.31 969 297 7 5.69 2,797 0.92 6.77 2,438 2.45 1,384 0.81 2.19 1,060 334 8 6.19 3,389 0.81 6.00 2,881 2,881 2,883 0.81 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 6.00 2,881 </td <td>3</td> <td>3.16</td> <td>671</td> <td>1.00</td> <td>2.69</td> <td>485</td> <td>186</td> <td>Э</td> <td>6.70</td> <td>1,647</td> <td>1.00</td> <td>7.39</td> <td>1,330</td> <td>318</td> | 3 | 3.16 | 671 | 1.00 | 2.69 | 485 | 186 | Э | 6.70 | 1,647 | 1.00 | 7.39 | 1,330 | 318 |
| 2.83 999 1,00 2.69 808 191 5 5.99 2.453 1,00 7.39 2.216 2.79 2.69 1,139 0.92 2.47 889 250 6 5.69 2,797 0.92 6.77 2.488 2.56 1,284 0.89 2.31 969 297 7 5.69 0.81 6.77 2.488 2.488 2.456 1,384 0.89 2.97 7 5.69 0.81 6.77 2.488 2.269 2.35 1,492 0.78 2.09 1,131 361 9 4.97 3.663 0.81 6.77 2.669 2.78 2.488 2.269 2.78 2.693 0.81 6.00 2.683 2.248 2.269 2.269 2.248 2.269 2.78 2.269 0.81 6.00 0.81 6.00 2.669 2.79 2.269 2.248 2.269 2.269 0.78 2.54 3.248 2.269 | 4 | 2.98 | 844 | 1.00 | 2.69 | 646 | 197 | 4 | 6.32 | 2,072 | 1.00 | 7.39 | 1,773 | 299 |
| 269 1,139 0,92 247 889 250 6 5,69 2797 0,92 6.77 2,438 2,789 2,438 2,438 2,439 2,789 6,77 2,438 2,439 2,789 2,438 2,438 2,431 0,86 6,77 2,438 2,683 2,683 2,683 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 2,883 0.78 3,509 0.78 5,75 3,102 2,883 3,224 <t< td=""><td>5</td><td>2.83</td><td>666</td><td>1.00</td><td>2.69</td><td>808</td><td>191</td><td>2</td><td>5.99</td><td>2,453</td><td>1.00</td><td>7.39</td><td>2,216</td><td>237</td></t<> | 5 | 2.83 | 666 | 1.00 | 2.69 | 808 | 191 | 2 | 5.99 | 2,453 | 1.00 | 7.39 | 2,216 | 237 |
| 2.56 1.267 0.86 2.31 969 297 7 5.43 3.111 0.86 6.33 2,669 2.45 1,584 0.81 1,050 3.34 8 5.19 3.388 0.81 6.00 2.881 8 2.25 1,584 0.75 2.09 1,131 361 9 4.77 3,609 0.78 5.75 3,102 3,846 2.25 1,582 0.73 1.96 1,283 10 4.77 3,609 0.75 5.54 3,342 3,546 3,342 3,546 3,346 | 9 | 2.69 | 1,139 | 0.92 | 2.47 | 889 | 250 | 9 | 5.69 | 2,797 | 0.92 | 6.77 | 2,438 | 329 |
| 2.45 1,384 0.81 2.19 1,050 334 8 5.19 3.388 0.81 6.00 2,881 9 4.97 3.388 0.81 6.00 2,881 9 4.97 3.683 0.78 5.75 3.102 3 | 7 | 2.56 | 1,267 | 98.0 | 2.31 | 696 | 297 | 7 | 5.43 | 3,111 | 0.86 | 6.33 | 2,659 | 452 |
| 2.35 1,482 0.78 2.09 1,131 361 9 497 3,663 0.78 5.75 3,102 3,102 2.25 1,582 0.75 2.02 1,112 380 10 4,77 3,909 0.75 5.54 3,324 3,244 2.17 1,685 0.73 1,212 380 11 4,59 0.75 5.54 3,324 3,324 2.09 1,686 0.73 1,89 0.73 4,318 0.77 5.23 3,767 3,546 3,546 3,324 3,546 3,546 3,546 3,546 3,546 3,546 3,546 3,548 3,546 3,548 3,548 3,546 3,548 3,54 | 8 | 2.45 | 1,384 | 0.81 | 2.19 | 1,050 | 334 | 80 | 5.19 | 3,398 | 0.81 | 00.9 | 2,881 | 518 |
| 2.25 1,582 0.75 2.02 1,212 380 10 477 3909 0.75 5.54 3,324 2.17 1,685 0.73 1,283 383 11 4,59 4,188 0.73 5.54 3,546 2.09 1,772 0.71 1,283 389 12 4,47 4,551 0.09 5,11 3,989 2.09 1,930 0.68 1,86 1,454 399 13 4,73 0.69 5,11 3,989 1.95 1,930 0.68 1,86 1,454 399 13 4,73 0.69 5,11 3,989 1.95 1,930 0.68 1,86 1,454 399 14 4,13 4,739 0.69 5,11 3,989 1 1.89 1,86 1,86 1,616 387 16 4,739 0.68 5,01 4,210 1 1.89 1,77 1,87 1,77 1,777 3,59 | 6 | 2.35 | 1,492 | 0.78 | 2.09 | 1,131 | 361 | 6 | 4.97 | 3,663 | 0.78 | 5.75 | 3,102 | 561 |
| 2.17 1,685 0,73 1,98 383 11 4,59 4,138 0,73 5,37 3,546 <td>10</td> <td>2.25</td> <td>1,592</td> <td>0.75</td> <td>2.02</td> <td>1,212</td> <td>380</td> <td>10</td> <td>4.77</td> <td>3,909</td> <td>0.75</td> <td>5.54</td> <td>3,324</td> <td>585</td> | 10 | 2.25 | 1,592 | 0.75 | 2.02 | 1,212 | 380 | 10 | 4.77 | 3,909 | 0.75 | 5.54 | 3,324 | 585 |
| 2.09 1,772 0.71 1.91 1,373 399 12 443 4,351 0.71 5.23 3,767 7 2.02 1,854 0.69 1,864 399 13 4,73 0.69 5.11 3,989 1 1.95 1,930 0.68 1,854 396 13 4,73 0.69 5.11 3,989 1 1.89 1,63 1,63 1,64 387 16 4,03 0.68 5.01 4,210 4,210 1.89 1,77 1,696 374 16 4,03 0.66 4,85 4,635 4,635 1.78 2,136 0.66 1,77 1,696 374 16 3,66 0.66 4,73 5,097 4,875 1,8 | 11 | 2.17 | 1,685 | 0.73 | 1.96 | 1,293 | 393 | 11 | 4.59 | 4,138 | 0.73 | 5.37 | 3,546 | 592 |
| 2.02 1,854 0.69 1.854 399 13 427 4,551 0.69 5.11 3,889 9 1.95 1,930 0.68 1.83 1,535 395 14 4,13 6,739 0.68 5.01 4,210 4,210 1.89 2,003 0.67 1,635 387 15 4,00 0.68 5.01 4,210 4,210 1.89 2,003 0.67 1,77 1,696 374 16 3,77 5,244 0.66 4,875 4,875 4,875 1,875 | 12 | 2.09 | 1,772 | 0.71 | 1.91 | 1,373 | 399 | 12 | 4.43 | 4,351 | 0.71 | 5.23 | 3,767 | 584 |
| 1.95 1,930 0.68 1,535 396 14 4,13 4,739 0.68 5.01 4,210 4,210 1.89 2,003 0.67 1,80 1,616 387 15 4,00 4,917 0.67 4,92 4,432 1,432 1.83 2,071 0.66 1,77 1,696 374 16 3.88 0.66 4,875 | 13 | 2.02 | 1,854 | 69:0 | 1.86 | 1,454 | 399 | 13 | 4.27 | 4,551 | 69:0 | 5.11 | 3,989 | 562 |
| 189 2,003 0.67 1.80 1.616 387 450 4.917 0.67 4.92 4.432 | 14 | 1.95 | 1,930 | 0.68 | 1.83 | 1,535 | 395 | 14 | 4.13 | 4,739 | 0.68 | 5.01 | 4,210 | 529 |
| 1.83 2.071 0.66 1.77 1,696 374 16 3.88 5,085 0.66 4.85 4,653 6.63 4,653 6.63 4,875 4,653 6.64 4.87 4,875 8 1.73 2,197 0.65 1,74 1,777 359 17 3,77 5,244 0.65 4,78 4,875 8 1.73 2,197 0.63 1,72 1,888 339 18 3.66 0.64 4,72 5,097 8 1.64 2,316 0.63 1,68 2,020 2,83 20 3,47 5,678 0.63 4,67 5,540 1.59 1.69 1,67 2,100 2,68 2,0 3,47 5,678 0.63 4,67 5,540 1.59 2,36 1,67 2,10 2,66 2,1 3,38 5,80 0.63 4,57 5,761 1 1.59 2,36 1,59 2,1 3,38 5,80 < | 15 | 1.89 | 2,003 | 0.67 | 1.80 | 1,616 | 387 | 15 | 4.00 | 4,917 | 0.67 | 4.92 | 4,432 | 485 |
| 1.78 2.136 0.65 1.74 1.777 359 17 3.77 5.244 0.65 4.78 4.875 | 16 | 1.83 | 2,071 | 99:0 | 1.77 | 1,696 | 374 | 16 | 3.88 | 5,085 | 99.0 | 4.85 | 4,653 | 431 |
| 1.73 2.197 0.64 1.72 1.858 339 18 3.66 5.396 0.64 4.72 5.097 | 17 | 1.78 | 2,136 | 0.65 | 1.74 | 1,777 | 329 | 17 | 3.77 | 5,244 | 0.65 | 4.78 | 4,875 | 369 |
| 1.68 2.256 0.63 1.70 1.939 317 19 3.56 5.540 0.63 4.67 5.318 1.81 1.59 2.020 2.93 2.03 | 18 | 1.73 | 2,197 | 0.64 | 1.72 | 1,858 | 339 | 18 | 3.66 | 5,396 | 0.64 | 4.72 | 5,097 | 299 |
| 1.64 2,312 0.63 1.68 2,020 293 20 3.47 5,678 0.63 4,62 5,540 5,640 1.59 2,366 0.62 1.67 2,100 2.65 21 3.38 5,809 0.62 4.57 5,761 Mod. FAA Minor Storage Volume (cubic ft.) = 399 Mod. FAA Maior Storage Volume (cubic ft.) = 399 | 19 | 1.68 | 2,256 | 0.63 | 1.70 | 1,939 | 317 | 19 | 3.56 | 5,540 | 0.63 | 4.67 | 5,318 | 222 |
| 1.59 2,366 0.62 1.67 2,100 265 21 3.38 5,809 0.62 4.57 5,761 5,761 Mod. FAA Minor Storage Volume (cubic ft.) = 399 Mod. FAA Major Storage Volume (cubic ft.) = | 20 | 1.64 | 2,312 | 0.63 | 1.68 | 2,020 | 293 | 20 | 3.47 | 5,678 | 0.63 | 4.62 | 5,540 | 138 |
| 399 Mod. FAA Maior Storage Volume (cubic ft.) = | 21 | 1.59 | 2,366 | 0.62 | 1.67 | 2,100 | 265 | 21 | 3.38 | 5,809 | 0.62 | 4.57 | 5,761 | 48 |
| _ (iii alama) alima a alama a a | | | | | | | | | | | | | | |

Mod. FAA Minor Storage Volume (cubic ft.) = 399

Mod. FAA Minor Storage Volume (cubic ft.) = 0.0092

Mod. FAA Major Storage Volume (acre-ft.) = 0.0092

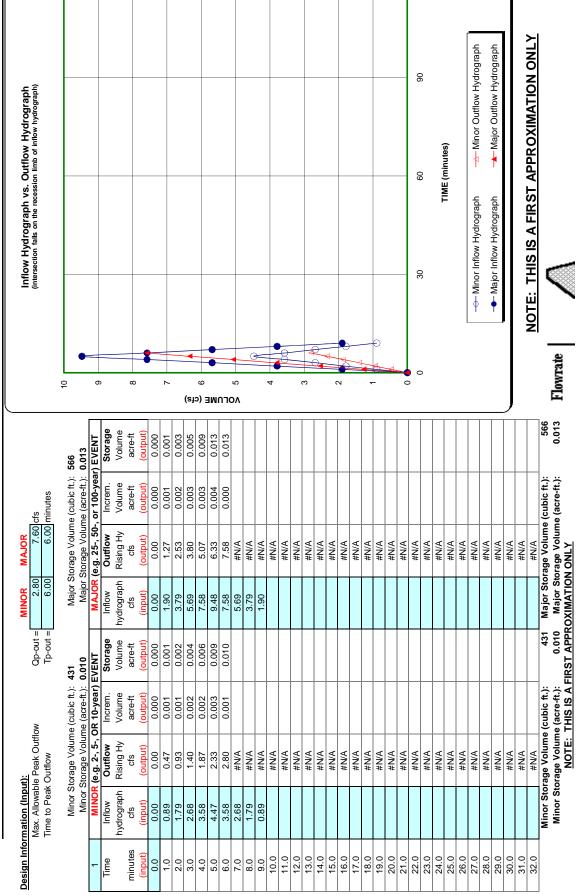
UDFCD DETENTION VOLUME ESTIMATING WORKBOOK Version 2.2, Released January 2010

120

DETENTION VOLUME APPROXIMATION USING THE HYDROGRAPH METHOD

Project: Mountain Village Hotel

Basin ID:



APPENDIX D HYDRAULIC CALCULATIONS

APPENDIX E RELATED FIGURES, TABLES & GRAPHS

Precipitation Frequency Data Output

NOAA Atlas 2 Colorado 37.939 N 107.846 W Site-specific Estimates

| Map | Precipitation (inches) | Precipitation Intensity (in/hr) |
|-------------------------|------------------------|------------------------------------|
| 2-year 6-hour | 1.04 | 0.17 |
| 2-year 24-hour | 1.59 | 0.07 |
| 100- year 6- hour | 2.43 | 0.41 |
| 100- year 24-hour | 3.46 | 0.14 |

Hydrometeorological Design Studies Center - NOAA/National Weather Service 1325 East-West Highway - Silver Spring, MD 20910 - (301) 713-1669 Wed Jan 28 12:36:03 2009

Table RO-3—Recommended Percentage Imperviousness Values

| Land Use or Surface Characteristics | Percentage Imperviousness |
|--|------------------------------|
| Business: | 1 |
| Commercial areas | 95 |
| Neighborhood areas | 85 |
| Residential: | |
| Single-family | * |
| Multi-unit (detached) | 60 |
| Multi-unit (attached) | 75 |
| Half-acre lot or larger | * |
| Apartments | 80 |
| Industrial: | |
| Light areas | 80 |
| Heavy areas | 90 |
| Parks, cemeteries | 5 |
| Playgrounds | 10 |
| Schools | 50 |
| Railroad yard areas | 15 |
| Undeveloped Areas: | |
| Historic flow analysis | 2 |
| Greenbelts, agricultural | 2 |
| Off-site flow analysis | 45 |
| (when land use not defined) | |
| Streets: | |
| Paved | 100 |
| Gravel (packed) | 40 |
| Drive and walks | 90 |
| Roofs | 90 |
| Lawns, sandy soil | 0 |
| Lawns, clayey soil | 0 |
| <u> </u> | <u> </u> |

^{*} See Figures RO-3 through RO-5 for percentage imperviousness.

$$C_A = K_A + (1.31i^3 - 1.44i^2 + 1.135i - 0.12)$$
 for $C_A \ge 0$, otherwise $C_A = 0$ (RO-6)

$$C_{CD} = K_{CD} + (0.858i^3 - 0.786i^2 + 0.774i + 0.04)$$
 (RO-7)

$$C_B = (C_A + C_{CD})/2$$

Table RO-5— Runoff Coefficients, C

| Percentage Imperviousness | | Type C and | D NRCS I | Hydrologic S | Soil Groups | |
|------------------------------|------|------------|----------|--------------|-------------|--------|
| | 2-yr | 5-yr | 10-yr | 25-yr | 50-yr | 100-yı |
| 0% | 0.04 | 0.15 | 0.25 | 0.37 | 0.44 | 0.50 |
| 5% | 0.08 | 0.18 | 0.28 | 0.39 | 0.46 | 0.52 |
| 10% | 0.11 | 0.21 | 0.30 | 0.41 | 0.47 | 0.53 |
| 15% | 0.14 | 0.24 | 0.32 | 0.43 | 0.49 | 0.54 |
| 20% | 0.17 | 0.26 | 0.34 | 0.44 | 0.50 | 0.55 |
| 25% | 0.20 | 0.28 | 0.36 | 0.46 | 0.51 | 0.56 |
| 30% | 0.22 | 0.30 | 0.38 | 0.47 | 0.52 | 0.57 |
| 35% | 0.25 | 0.33 | 0.40 | 0.48 | 0.53 | 0.57 |
| 40% | 0.28 | 0.35 | 0.42 | 0.50 | 0.54 | 0.58 |
| 45% | 0.31 | 0.37 | 0.44 | 0.51 | 0.55 | 0.59 |
| 50% | 0.34 | 0.40 | 0.46 | 0.53 | 0.57 | 0.60 |
| 55% | 0.37 | 0.43 | 0.48 | 0.55 | 0.58 | 0.62 |
| 60% | 0.41 | 0.46 | 0.51 | 0.57 | 0.60 | 0.63 |
| 65% | 0.45 | 0.49 | 0.54 | 0.59 | 0.62 | 0.65 |
| 70% | 0.49 | 0.53 | 0.57 | 0.62 | 0.65 | 0.68 |
| 75% | 0.54 | 0.58 | 0.62 | 0.66 | 0.68 | 0.71 |
| 80% | 0.60 | 0.63 | 0.66 | 0.70 | 0.72 | 0.74 |
| 85% | 0.66 | 0.68 | 0.71 | 0.75 | 0.77 | 0.79 |
| 90% | 0.73 | 0.75 | 0.77 | 0.80 | 0.82 | 0.83 |
| 95% | 0.80 | 0.82 | 0.84 | 0.87 | 0.88 | 0.89 |
| 100% | 0.89 | 0.90 | 0.92 | 0.94 | 0.95 | 0.96 |
| | | TYPE B N | RCS HYDR | OLOGIC SOIL | S GROUP | |
| 0% | 0.02 | 0.08 | 0.15 | 0.25 | 0.30 | 0.35 |
| 5% | 0.04 | 0.10 | 0.19 | 0.28 | 0.33 | 0.38 |
| 10% | 0.06 | 0.14 | 0.22 | 0.31 | 0.36 | 0.40 |
| 15% | 0.08 | 0.17 | 0.25 | 0.33 | 0.38 | 0.42 |
| 20% | 0.12 | 0.20 | 0.27 | 0.35 | 0.40 | 0.44 |
| 25% | 0.15 | 0.22 | 0.30 | 0.37 | 0.41 | 0.46 |
| 30% | 0.18 | 0.25 | 0.32 | 0.39 | 0.43 | 0.47 |
| 35% | 0.20 | 0.27 | 0.34 | 0.41 | 0.44 | 0.48 |
| 40% | 0.23 | 0.30 | 0.36 | 0.42 | 0.46 | 0.50 |
| 45% | 0.26 | 0.32 | 0.38 | 0.44 | 0.48 | 0.51 |
| 50% | 0.29 | 0.35 | 0.40 | 0.46 | 0.49 | 0.52 |
| 55% | 0.33 | 0.38 | 0.43 | 0.48 | 0.51 | 0.54 |
| 60% | 0.37 | 0.41 | 0.46 | 0.51 | 0.54 | 0.56 |
| 65% | 0.41 | 0.45 | 0.49 | 0.54 | 0.57 | 0.59 |
| 70% | 0.45 | 0.49 | 0.53 | 0.58 | 0.60 | 0.62 |
| 75% | 0.51 | 0.54 | 0.58 | 0.62 | 0.64 | 0.66 |
| 80% | 0.57 | 0.59 | 0.63 | 0.66 | 0.68 | 0.70 |
| 85% | 0.63 | 0.66 | 0.69 | 0.72 | 0.73 | 0.75 |
| 90% | 0.71 | 0.73 | 0.75 | 0.78 | 0.80 | 0.81 |
| 95% | 0.79 | 0.81 | 0.83 | 0.85 | 0.87 | 0.88 |
| 100% | 0.89 | 0.90 | 0.92 | 0.94 | 0.95 | 0.96 |



COLORADO

FLOODPLAIN AND STORMWATER CRITERIA MANUAL

| Region of applicability* | Equation | Corr. coeff. | No. of stations | Mean of computed stn. values (inches) | Standard error of estimate (inches) |
|---|--|-----------------|-----------------|--|--|
| South Platte, Republican, Arkansas, and Cimarron River | $Y_2 = 0.218 + 0.709[(X_1)(X_1/X_2)]$ $Y_{100} = 1.897 + 0.439[(X_2)(X_2/X_4)]$ | 0.94 | 75 | 1.01 | 0.074 |
| Basins (1) | — 0.008Z | .84 | 75 | 2.68 | .317 |
| San Juan, Upper Rio Grande, Upper Colorado, and Gunnison River Basins and Green River Basin below confluence with the Yampa River (2) | $Y_2 = -0.011 + 0.942[(X_1)(X_1/X_2)]$ $Y_{100} = 0.494 + 0.755[(X_8)(X_9/X_4)]$ | .95 .90 | 86 85 | 0.72 1.96 | .085 .290 |
| Yampa and Green River Basins above confluence of Green and Yampa Rivers (3) | $Y_2 = 0.019 + 0.711[(X_1)(X_1/X_2)] + 0.001Z$ $Y_{100} = 0.338 + 0.670[(X_3)(X_3/X_4)]$ | .82 | 98 | 0.40 | .031 |
| | + 0.001Z | .80 | 79 | 1.04 | .141 |
| North Platte (4) | $Y_2 = 0.028 + 0.890[(X_1)(X_1/X_2)]$ $Y_{100} = 0.671 + 0.757[(X_3)(X_3/X_4)]$ | .93 | 90 | 0.60 | .062 |
| | — 0.003Z | .91 | 88 | 1.71 | .236 |

^{*} Numbers in parentheses refer to geographic regions shown in figure 19. See text for more complete description.

List of variables

 $Y_2 = 2$ -yr 1-hr estimated value $Y_{100} = 100$ -yr 1-hr estimated value $X_1 = 2$ -yr 6-hr value from precipitation-frequency maps

X₂ = 2-yr 24-hr value from precipitation-frequency maps

X₃ = 100-yr 6-hr value from precipitation-frequency maps
 X₄ = 100-yr 24-hr value from precipitation-frequency maps
 Z = point elevation in hundreds of feet

VERSION: JANUARY 2006 REFERENCE: NOAA ATLAS 2, VOL III, 1973

TABLE CH9-T402 **EQUATIONS FOR ESTIMATING 1-HOUR RAINFALL VALUES**



COLORADO

FLOODPLAIN AND STORMWATER CRITERIA MANUAL

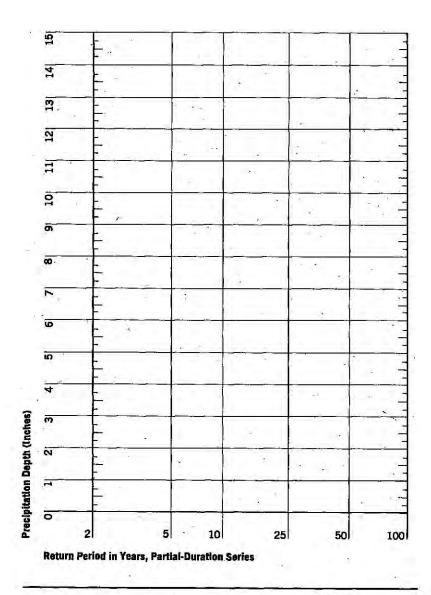


Figure 6. Precipitation depth versus return period for partial-duration series.

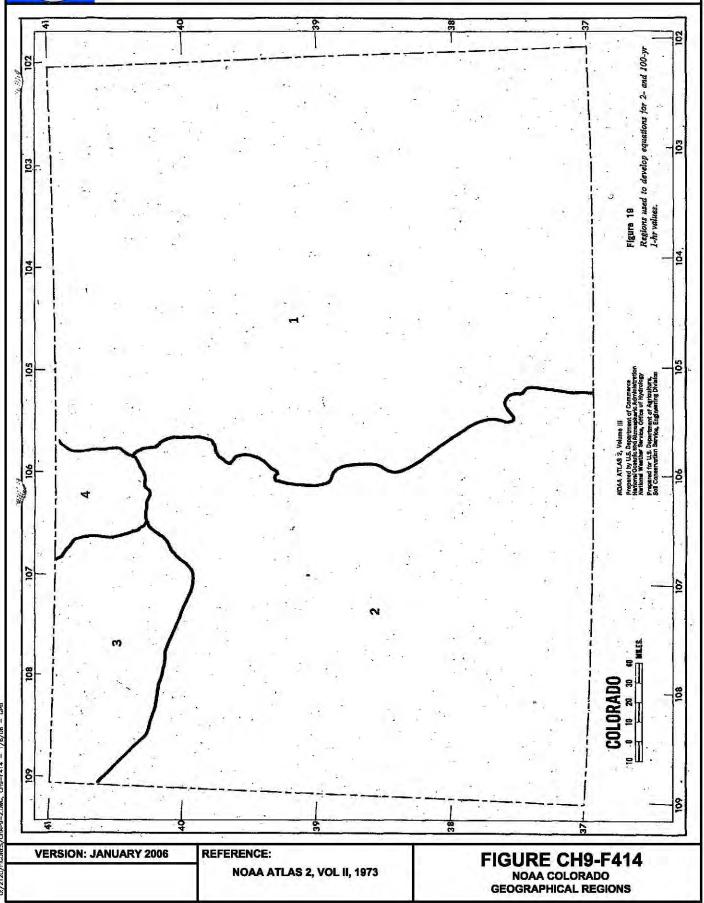
VERSION: JANUARY 2006 REFERENCE:
NOAA ATLAS 2, VOL II, 1973

FIGURE CH9-F413

PRECIPITATION RETURN PERIOD DIAGRAM



COLORADO FLOODPLAIN AND STORMWATER CRITERIA MANUAL





222 South Park Ave. • Montrose, CO 81401 Ph.: (970) 249-6828 • FAX: (970) 249-0945

GEOTECHNICAL REPORT THE ALPENGLOW LOTS 73 & 76 MOUNTAIN VILLAGE SAN MIGUEL COUNTY, COLORADO

Executive Summary

On May 22, 2007, Buckhorn Geotech, Inc., carried out an investigation of site and subsurface conditions at the proposed *Alpenglow* project. This report presents the factual data obtained during the investigation and our interpretation of these data.

Lots 73 and 76 are situated at the north end of the Mountain Village core, at the inside bend of Mountain Village Boulevard. The lots encompass a total of approximately 4,700 square feet in what is primarily a grassed area between the Shirana Condominiums to the south and Mountain Village Boulevard to the north. The topography across the property is a somewhat hummocky drop to the southwest at an approximate grade of 15%.

We understand the proposed structure will enclose approximately 29,000 square feet in four stories above a daylighting garage basement.

The subsurface conditions consist of up to 2 feet of fill, underlain by reddish-brown to dark brown silty clay with gravel to depths between 10 and 38 feet below the ground surface. Underlying this material is a jumbled shale, extending to depths of 32 to 44 feet. Both the silty clay and jumbled shale exhibited low to moderate potential for heave, and the silty clay exhibited significant settlement under increasing loading. Below this depth, hard shale/siltstone was encountered. Groundwater was encountered between depths of 9.3 and 37.3 feet below the ground surface during drilling and between 10.7 and 42.2 feet several weeks after drilling.

Below is a summary of our recommendations for the proposed development of this site.

 A deep foundation consisting of driven piles is the preferred foundation system at this site.

- Alternatively a spread footing foundation system may be considered but the existing subsurface conditions may contribute to settlement of the strucure.

 Recommendations are provided to reduce this potential.
- A slab on-grade floor may be used in the garage parking basement but should be placed on a minimum one foot of structural fill to reduce the potential for post-construction heave.
- Foundation and under-slab drainage should be provided to aggressively evacuate groundwater from the building excavation.
- Excavation stabilization will be needed for this site as spatial constraints preclude the ability to lay slopes back. Potential shoring systems include soil nails, sheet piles, and tieback anchor systems.
- All of the recommendations presented in the *Conclusions and Recommendations*Section of this report should be incorporated into design and construction at this site.

Introduction

Buckhorn Geotech, Inc., conducted an investigation of subsurface and site conditions on May 22, 2007, at the proposed building site on Lots 73 and 76, Mountain Village, Colorado. This work was performed at the request of Mr. Randall Huggins of RDH & Associates, Inc. The purpose of the investigation was to evaluate the property for construction of a multi-story condominium unit. The investigation consisted of a site inspection, drilling of four boreholes, logging and testing of materials encountered, and analysis of available data. This report presents the findings of our investigation and our geotechnical engineering recommendations for site preparation and foundation design.

Construction Plans

Based on the latest set of plans electronically provided to us by Mr. Huggins entitled *Lot 73 & 76 Telluride Mountain Village Design Review Board Sketch Approval* and digitally dated 9 August 2006, we understand the previously-named *Rockmist* structure will enclose approximately 29,000 square feet in four stories above a daylighting garage basement. The exterior will comprise stone, stucco, wood, and copper. The garage basement will have a concrete slab floor. No structural details are known at this point, although buildings of similar size in Mountain Village are supported with concrete and steel framing. It is anticipated that the structure will transfer approximately 2,000 to 15,000 pounds per linear foot to the foundation level with concentrated loads of 50,000 to 250,000 pounds.

Because the structural details are unknown at this point, Buckhorn Geotech should be provided the opportunity to review the conclusions and recommendations provided below once a full set of the plans is available for this project. The applicability of the conclusions and recommendations provided below may be affected by the final proposed project plans.

Site Conditions

The Town of Mountain Village is situated on the north flank of Silver Mountain immediately above and to the south of the San Miguel River Valley and Town of Telluride, as shown on the attached Vicinity Map. Lots 73 and 76 are situated at the north end of the Mountain Village core, at the inside bend of Mountain Village Boulevard. Fantastic views of the Wilson Range (to the southwest) and the Sneffels Range (to the north) are afforded from this property. The lots encompass a total of approximately 4,700 square feet in what is primarily a grassed area between the Shirana Condominiums to the south and Mountain Village Boulevard to the north. As shown on the attached Site Map, parking lots lie to the east and west of the lots, separated by a concrete sidewalk on the east boundary of the lots. Vegetation comprises mostly irrigated lawn with a few pine trees (planted) on the north boundary and aspen trees (also planted) near the southwest corner. The topography across the property is a somewhat hummocky drop to the southwest at an approximate grade of 15%, as seen in the following photograph. The approximate elevation of the property is 9,525 feet above mean sea level (msl).



Looking southwest across the lots from the Mountain Village Boulevard overpass, the Shirana Condominiums (far upper left) and the western parking lot can be seen. The Wyndham Peaks can be seen in the far upper right.

Geology

The San Juan Mountains of southwestern Colorado are a region of uplifted Paleozoic and Mesozoic sedimentary formations intruded by Tertiary volcanics. In the Telluride region,

uplifting that accompanied the volcanic eruptions caused warping and folding of older sedimentary bedrock. As magma rose towards the ground surface, some was injected into deeper fractures in sedimentary strata forming a network of dikes and sills. The magma was rich in mineralized fluids, forming the gold and silver veins that made the area a rich mining district. In the millions of years since the intrusives were formed, much of the overlying sedimentary rock has been weathered and stripped away by erosion, landslides, and glaciation to create the dramatic present landscape.

Based upon the general geologic study conducted by Lincoln DeVore of the Telluride Mountain Village (Geology and Soils of Proposed Expansion of the Telluride Ski Area, San Miguel County, Colorado: 1979), Lots 73 and 76 is geologically complex, mapped as Quaternary Slope Failure Complex (Qsfc) with Quaternary Glacial Drift and Moraine (Qm) on the northern portion of Lot 76 and close-by mapping of Dakota Sandstone (Kd) to the east (uphill) and seasonally wet areas (Sw) to the south. The Slope Failure Complex (Qsfc) is an undifferentiated, extensive, ancient landslide believed to be associated with Silver Mountain Landslide, a large-scale event covering about 15 square miles, including the Mountain Village, Telluride Ski Ranches and Elk Run developments. This massive slide complex is composed of silty to clayey sand with volcanic gravels, cobbles and boulders, but can sometimes contain entrained blocks and fragments of shale and sandstone. These soils generally have low to moderate plasticity and are moderately dense to dense. The depth and composition of Slope Failure materials are highly variable, but are generally considered stable landforms for development if drainage and potential slope instability are properly accounted for during design and construction.

Quaternary Glacial Drift and Moraine (Qm) occurs as a more or less continuous band bordering the northern edge and northwest corner of the Mountain Village. Glacial drift and moraine deposits were formed in the Pleistocene as the result of glacial and glaciofluvial processes. These glacial moraine deposits are highly variable in composition but generally consist of medium to coarse gravel with frequent, subrounded to angular, cobbles and boulders within a matrix of silty sand with some clay.

The Dakota Formation (Kd) is typically a tan, yellow, light-red to light brown sandstone with conglomeratic lenses and localized beds of coal and carboniferous shale. This unit is well-cemented and resists weathering, and is therefore a ridge and cliff-forming rock. Although originally lain down as horizontal beds of sand and muds in a beach and tidal lagoon environment, the strata were uplifted and tilted to the west during the Tertiary Period. The dip of the rock strata typically matches the grade of the ground surface on the slope of Coonskin Mountain, to the east of the property. Overlying the Dakota Formation in this region is usually the younger Mancos Shale (Kmc), but due to the steep slope, this shale layer has generally been removed.

Mancos Shale (*Kmc*) is a gray to black marine shale (claystone to siltstone) that is generally thinly bedded and highly fractured in mountainous areas such as this. This rock unit is highly variable in composition and can range from low to high plasticity and from soft and highly weathered to dense and unweathered. Generally, shale becomes less weathered with depth.

The surficial materials found on these lots during our field investigation are probably best represented by a thin veneer of fine-grained slope wash from the Silver Mountain landslide (*Qsfc*) underlain by shallow Mancos Shale (*Kmc*). The subsurface encountered at this site will be further discussed in the *Subsurface Conditions* Section of this report. See the 1979 Lincoln DeVore geologic study of Mountain Village for more information about the described subsurface types.

Geologic Hazards

This section of the report is included so that the potential building owner is aware that the beauty of the Telluride region comes with certain risks. Modern development in the river valley and surrounding mountains can be considered to be only about 40 years old, with most occurring in the past 25 years. Because of this relatively short period of time, useful empirical data are limited. Some buildings and roadways throughout the local mountains and valleys have experienced negative impacts due to slope movement, poor subgrade, and groundwater problems. Logical structural engineering techniques for design and construction of buildings and roadways can be used to reduce the potential for problems related to troublesome climate and soil conditions. However, because of the overall dynamic characteristics of the area, almost every structure is subject to at least some degree of potential risk. These risks are explained below.

Runoff & Erosion

Surface runoff from rainfall and snowmelt is allowed to sheet flow across the property toward the southwest corner of the property, where it is caught by a storm sewer drop inlet. The grass vegetation is mature and no signs of erosion (as may be indicated by gullying or piping) were observed on the property. No natural or man-made drainages (swales, ditches, gullies, etc.) cross the building site.

Shallow Groundwater

Groundwater was encountered at depths between 9.3 and 37.3 feet below adjacent ground surface during our investigation and between 10.7 and 42.2 feet below adjacent ground several weeks after drilling. The shallowest groundwater was found at the south end of the site, quickly deepening to the north and west.

Shallow groundwater can be problematic as it weakens foundation subgrade materials, creates hydraulic pressure, and can seep into the interior of the buildings if foundation components are not properly waterproofed. Consequently, aggressive management of surface and subsurface water at this site is very important for the long-term performance of the foundation components and slope stability. A comprehensive site drainage plan, in tandem with grading and landscape plans, should be designed to intercept surface and subsurface water and remove it from the foundation area. This may include an intercept or curtain drain, a foundation drain, and/or other active surface or subsurface drainage features. Ideally, construction would occur during a dry time of the year to minimize the amount of water and saturated soils in the foundation area.

Specific recommendations for grading and foundation preparation are given below in the *Conclusions and Recommendations* Section of this report.

It should be noted that groundwater levels may not remain static due to permeability of soils, seasonal variations, and drilling/excavating effects. Changes in the groundwater level are a concern at this site, and we should be contacted to prescribe appropriate recommendations if groundwater is encountered during design or development of the property.

Expansive and Compressive Soil and Rock

Soil and rock materials containing some types of clay, especially bentonite (montmorillonite), can expand in volume with water absorption and then shrink upon drying. In some areas of Colorado these expansive geomaterials are very hazardous and can cause serious damage to foundations, roadways, pavements, and embankments. The geology of swelling soils, the effects of moisture on these soils, and construction and landscaping on swelling soils are discussed in the Colorado Geological Survey publication, *A Guide to Swelling Soils for Colorado Homebuyers and Homeowners* (Special Publication #43: Noe et al., 1997). In the Telluride area, these clays are derived from such parent material as Mancos Shale, Dakota Sandstone, Morrison Formation, and volcanic material.

Compressive soils are generally soils that have been laid down rapidly, have a weak matrix containing voids, and/or are not naturally in a dense or compacted state. Compressive soils typically have a large proportion of fine-grained materials, especially silt, but they can also contain a mixture of material if deposited in a chaotic manner. For example, debris fan and landslide deposits are often laid down rapidly and comprise materials that are not sorted or reworked, leaving behind voids and a loose matrix of rocks, soil, and possibly organic debris. Clayey soils can also be compressive if they are saturated when loads are applied.

Mancos Shale and its residual soil can be very sensitive to variations in moisture, being quite strong when dry but either expansive or losing strength rapidly when wetted. Additionally, wetting and drying cycles can weaken the shale so that it becomes highly erodible. When in a dry and dense state, the shale and its residual soil can exert expansive pressures when moisture is absorbed. Conversely, when in a loose, highly fractured state, the material can consolidate when wetted under moderate loads. The expansive and compressive characteristics of the shale and soil are discussed in detail in the *Subsurface Conditions* Section of this report.

The potential hazard from expansive and compressive soil and rock is the differential movement of the subgrade under loads applied through the foundation. This hazard can be partly mitigated by control of on-site drainage so that no water is allowed to accumulate, stand, or penetrate into the soil in the vicinity of foundations and slab/pavement areas. Further mitigation can be attained through design of foundation systems that extend to firmer material or which have sufficient strength to resist differential movements. The removal of problematic soil and replacement with structural fill is another option. These methods are discussed in further detail below in the *Conclusions and Recommendations* Section. Special Publication #43 gives general explanations and illustrations of design and drainage options on swelling soils.

Seismicity

According to the Geologic Map of the Telluride Quadrangle (USGS Map GQ-504), the Vanadium Fault is located 4 miles west of Mountain Village. This northwest trending fault, which extends from the intersection of the Skunk Creek drainage and Highway 145 to the south side of Gray Head Mountain, is not identified in either of the Colorado Geological Survey (CGS) reports identifying geologically recent (Quaternary-aged) and potentially active faults [Earthquake Potential in Colorado—A Preliminary Evaluation (Bulletin #43: Kirkham and Rogers, 1981) and Preliminary Quaternary Fault and Fold Map and Database of Colorado (Open-file Report 98-8: Widmann et al., 1998)]. The closest mapped potentially active faults to the subject property are the San Miguel Canyon Faults (located roughly 14 miles northwest of Telluride). These faults, interpreted as active during the Quaternary, are thought to be related to salt tectonism (movement of deep-seated salt deposits). A maximum credible earthquake for this fault zone is a magnitude (M) 5.0 event. The next closest potentially active faults are the Ridgway, Busted Boiler, and Log Hill Mesa Graben Faults north of Ridgway (roughly 16 miles north of Telluride). These faults are located at the southern end of the Uncompangre Plateau and are also interpreted to be Quaternary-aged. The maximum credible earthquake inferred for these faults is M6.0 to 6.75.

Mountain Village is located in the Western Mountain Seismotectonic Province in Colorado, where maximum credible earthquakes are estimated to be on the order of magnitude 6 to 6.5, equivalent to Modified Mercalli (MM) VI to VIII (CGS Bulletin #43). The largest recorded earthquake in the region was the 1994 M4.4 (MM VI) Norwood event [according to the CGS Bulletin #52 entitled *Colorado Earthquake Information, 1867-1996* (Kirkham and Rogers, 2000) and the CGS website database of earthquake events: http://geosurvey.state.co.us]. There were several other similar magnitude earthquakes in the Telluride region: Telluride in 1894 (MM IV), Ridgway in 1897 (MM V), Lake City in 1913 (MM VI) and 1955 (MM VI), and Cimarron Ridge/Montrose in 1960 (MM VI) and 1962 (MM V). Many other earthquake events less than MM V have been identified for the region.

The Colorado Geological Survey indicates that, based on limited historical records, Colorado is considered to be a region of minor earthquake activity, where moderate to large events are relatively infrequent. There is a growing body of evidence, however, suggesting that Colorado is at greater risk than previously thought. According to the Uniform Building Code, western Colorado is in Seismic Risk Zone 1 where distant earthquakes would be expected to cause only minor damage to structures with fundamental periods of vibration greater than one second. Except for transmission towers, we are unaware of such tall, slender structures in western Colorado. However, the CGS recommends in Bulletin #43 that a Seismic Risk Zone 2 designation may be more appropriate for all of Colorado except the extreme northeast corner. It also suggests that a minimum 0.1g horizontal acceleration be used in design and safety analyses even for areas that are distant from known active faults.

Radon Gas

Radon gas is produced by decay of radioactive minerals contained in subsurface rock and soil. The U.S. Environmental Protection Agency (EPA) has determined that radon is the second leading cause of lung cancer and that radon can accumulate in homes if the gas is not properly removed through passive or active methods. The EPA map of Radon Zones indicates that virtually all of western Colorado, including San Miguel County, is in Zone 1 (www.epa.gov/iaq/ radon/zonemap/colorado.htm). Although there is no known safe level of radon, Zone 1 is the zone of highest risk for exposure to radon gas [i.e., greater than 4 picoCuries per Liter (pCi/L)]. The Colorado Geological Survey (CGS) participated in an EPA study in 1987 and 1988 to record indoor radon levels throughout Colorado homes and compiled its results in a report that relates geologic setting and house construction with radon levels (CGS 1991 Open-File Report 91-4). Generally, homes with basements had higher levels of radon than homes built on grade on the same geologic material. In our region of Colorado, Precambrian igneous rocks had the highest readings, followed by older Mesozoic sedimentary rocks, and Tertiary volcanic and volcaniclastic rocks. Radon values in alluvial and glacial valley fill was highly variable. The CGS is careful to state that radon potential can vary considerably within the same geologic unit due to the non-uniform distribution of uranium, secondary leaching, and the accumulation of uranium and other radioactive elements into other strata.

The EPA recommends testing radon levels in existing homes, but has not developed a sampling test that will determine levels of radon gas in the native soils prior to construction. This is due to the many factors that affect the movement of radon through soils, such as soil moisture, soil types, weather patterns, and wind. These factors cannot be completely accounted for or controlled during testing. Based on levels of radon recorded in existing homes in the region and the presence of rock types that are known to produce radon, it is reasonable to assume that radon is present in the Telluride area. The EPA, the Colorado Department of Public Health and Environment (CDPHE) Radiation Management Division, and the National Association of Home Builders (NAHB) recommend that all new homes constructed in Zone 1 should include radonresistant features. These organizations also recommend that after the house is constructed, radon should be measured in the home and if the results are greater than 4 pCi/L, the system should be upgraded from passive to active (usually by installing a fan). In the EPA publication entitled, Building Radon Out: A Step-by-Step Guide on How to Build Radon-Resistant Homes (USEPA Office of Air and Radiation EPA/402-K-01-002, April 2001), three practical and inexpensive alternatives for passive, sub-slab depressurization systems are presented: gravel with vents, perforated pipes, or soil gas collection mats. As stated in that EPA publication, radon-reduction techniques not only reduce radon in the home but also are "consistent with state-of-the-art energy-efficient construction...which will result in energy savings and lower utility bills for the homeowner" and they have the added benefits of "decreasing moisture and other soil gases in the home, reducing molds, mildews, methane, pesticide gases, volatile organic compounds, and other indoor air quality problems." It is estimated that retrofitting a house after construction with radon resistant features is 2 to 10 times more expensive than if it had been included in the original construction.

The *Building Radon Out* EPA publication can be obtained from the CDPHE in Denver by calling (303) 692-3420. Other recommendations for passive and active design and construction

techniques for reducing radon gas can be found on the EPA radon website www.epa.gov/radon/or the CDPHE radon website www.cdphe.state.co.us/hm/rad/radon.

No other geologic hazards are known to be present in the vicinity of the Alpenglow project.

Subsurface Conditions

Four borings (BH07-1 to BH07-4) were advanced to depths between 33 and 48.5 feet using a Bombardier track-mounted CME 45 drill rig at the locations noted on the attached Site Map. The locations of the borings were selected prior to the investigation based on the building location provided to us in the undated sketch plans and adjusted in the field as conditions dictated (actual utility locations, access, etc.). The boreholes were drilled with a 4-inch solid stem continuous-flight auger. Samples of the subsurface materials were obtained at discrete depths by withdrawing the 4-inch drill string and inserting either a standard 1.375-inch inside diameter (I.D.) split-spoon sampler without liners or a 2-inch I.D. split-spoon "California" sampler to perform in-situ Standard Penetration Tests (SPTs) in general accordance with ASTM Standard D-1586. The number of blows required to drive the sampler 12 inches in 6-inch increments were recorded (SPT "N" penetration resistance values) and, when properly evaluated, indicate the relative density or consistency of the soils.

The soil, bedrock, and groundwater conditions were logged, and representative samples of subsurface materials encountered were brought back to our laboratory for detailed examination and testing. The subsurface conditions encountered in the borings and laboratory results are shown on the attached Logs of Exploratory Drilling.

Generally, the subsurface conditions encountered at the site consist of a thin veneer (up to 2 feet thick) of gravelly **fill/topsoil** overlying dark brown to reddish-brown **sandy to silty clay** with varying amounts of gravel. This material is generally moist, very stiff to hard and contains some shale chips. At depths between 10 and 38 feet below adjacent ground surface, brown to black **jumbled shale** was encountered. This material exhibited significant variations in strength, density, and degree of weathering. At depths between 32 and 44 feet, hard shale/siltstone was encountered. No samples of this material (save what could be taken from the auger bit) were recovered. Refusal was encountered within one to 6.5 feet of the top of this surface. **Groundwater** was encountered at depths between 9.3 (BH07-1) and 37.3 (BH07-3) feet below the ground surface during or immediately after drilling. On 12 June 2007 the recorded groundwater levels were between 10.7 and 11.9 feet (BH07-2 and -3, respectively), 26.6 feet (BH07-1), and 42.2 feet (BH07-4).

Index, behavioral, and strength tests were conducted on samples of the subsurface materials encountered at this site. Samples of the brown to reddish-brown sandy to silty clay and the jumbled shale were tested for chemical properties, which yielded water soluble sulfate concentrations between 0.01 and 0.02%, chloride contents of 15 parts per million (ppm), electroconductivies between 15 and 27 microsiemens per centimeter (µS/cm), and pH between 6.8 and 7.3. A sample of the jumbled shale was subjected to a swell/consolidation test to determine its behavior under varying conditions of loading moisture. The sample was initially

loaded to 1,900 pounds per square foot (psf), approximately equal to its in-situ stress state, then the pressure was dropped to 200 psf and inundated with water. The sample swelled 0.03%, considered negligible, then was subjected to progressively increasing pressures to a total of 8,000 psf. Under these pressures the sample consolidated a total of 4.7%, considered moderate. The initial dry density of the sample was 114 pounds per cubic foot (pcf) and the calculated swell pressure is 220 psf, considered low. A sample of the brown to reddish-brown sandy to silty clay was likewise subjected to a swell/consolidation test, revealing a 5.1% compression under an initial seating pressure of 2,500 psf, 0.8% swell under a confining pressure of 600 psf, and a consolidation of 11.6% under a final pressure of 10,000 psf. The initial dry density of this sample was 118 pcf and the theoretical calculated estimated pre-construction swell pressure is 840 psf, considered moderate.

A sample of the reddish-brown sandy clay with gravel was subjected to a direct shear test to determine strength parameters. Cohesion of 90 psf and a phi angle (ϕ) of 32° was used for our analyses. A sample of the jumbled shale was also tested for shear strength parameters, revealing a cohesion of 1000 psf with a phi angle (ϕ) of 18°.

In summary, the site is overlain by up to 2 feet of fill, underlain by reddish-brown to dark brown silty clay with gravel to depths between 10 and 38 feet below the ground surface. Underlying this material is a jumbled shale, extending to depths of 32 to 44 feet. Both the silty clay and jumbled shale exhibited low to moderate potential for heave, and the silty clay exhibited significant settlement under increasing loading. Below this depth, hard shale/siltstone was encountered. Groundwater was encountered between depths of 9.3 and 37.3 feet below the ground surface during drilling, and 10.7 to 42.2 feet several weeks after drilling.

CONCLUSIONS AND RECOMMENDATIONS

Based upon our site inspection and results of the subsurface exploration, the following recommendations are offered as measures to enhance the stability of the site and the long-term performance of the foundation soils. It should be noted that the mitigation measures offered address only the construction at the building site. They cannot and will not arrest or prevent large-scale geologic processes that may be on-going elsewhere on the property and within the Mountain Village area. Also, as mentioned earlier in this report, some degree of risk is inherent in all construction in mountainous areas of Colorado. The recommended measures are intended to be reasonable and prudent but cannot be considered as absolute protection against the vagaries of nature.

This report does not contain project specifications. The recommendations given are provided to guide the design process. We anticipate these recommendations, together with site-specific geotechnical information, will be used by the design team to formulate specifications for construction of buildings, infrastructure, and grading.

General Design Criteria

- 1. Based on the elevation of 9,525 feet, the Structural Engineers Association of Colorado recommends that the Basic Roof Snow Design Load be a minimum of 93 psf. It is recommended that the local building official be contacted to verify the required snow design load for this property.
- 2. Shallow components of the foundation system should be extended into the soil a minimum depth below finished grade as prescribed by the local building official to reduce the negative effects of frost heave.

Seismic Design Criteria

In accordance with Section 1615 of the 2003 International Building Code (IBC) and our knowledge of the site, this site may be designated as Site Class D. The mapped spectral response acceleration at short periods (0.2 second, S_s) is 0.468g and at one second (S_1) is 0.106g. These values are taken from the USGS website, and are referenced to the National Earthquake Hazard Reduction Program (NEHRP) 1997 and 2000 maps, reproduced in the IBC. As provided in the 2003 IBC, these values are for Site Class B, and should be adjusted accordingly for the proper site class given above.

Foundation

Because the foundation elevation will place the load transfer points on varying subgrade materials across the site and the differing materials show significantly different behavior under loading, it is recommended that the owner consider placing the structure upon a deep foundation. Given the softer subsurface conditions in the near-surface, driven piles are our preferred foundation system at this site.

Driven Piles

Designed properly, driven piles will isolate the structure from subsurface and surficial soil movements. Support of the structure is provided through concrete grade beams constructed upon and spanning the piles. The primary advantage of using driven piles is that the bearing capacity of each pile is verified during the driving process. The secondary advantage of driven piles is the simplicity and speed of installation as compared with other deep foundation types. The primary disadvantage of this system is the disturbance caused by the noise and shock waves produced during installation. Design parameters and recommendations for driven pile foundation systems are outlined below.

- 1. Piles may be Grade 36 H-piles or closed-ended pipe piles, with an allowable capacity of 9,000 psi based on the cross-sectional area of the pile.
- 2. Piles should be driven to "virtual refusal" in the shale bedrock, defined as 3 or more consistent sets of a defined blow count per unit penetration. Buckhorn Geotech will

define refusal criteria once the pile driving contractor has been selected and his hammer energy and stroke criteria are available for evaluation. The hammer and cushion should match the pile type to obtain the proper load capacity during driving. Appropriate recommendations on tip reinforcement will also be provided.

- 3. Piles spaced closer than 3 times the pile diagonal or diameter shall be considered to be influenced by group action and an appropriate reduction in individual pile capacity should be made. Piles driven into bedrock should be placed no closer than 1.75 times the pile's cross-sectional diagonal or 24 inches on centers (whichever is greater).
- 4. Piles broken or damaged prior to, during, or after installation should not be used.
- 5. A piling contractor with demonstrated successful experience driving similar piles with qualified personnel in similar conditions should be chosen to perform the pile installations.

A representative of Buckhorn Geotech should be present to observe and document all pile installation operations.

Other Deep Foundation Types

Other deep foundation systems (such as micropiles or screwpiles) may be considered for use at this site. If these systems are being considered, we would be happy to provide additional design parameters.

Shallow Foundation

A shallow foundation system may also be considered, but limiting design parameters may preclude this alternative. The bearing capacity of the subsurface materials found at the proposed foundation grade (the reddish-brown silty clay with gravel and jumbled shale) are limited by the poor strengths of each and the tendency of the reddish-brown soil to settle under moderate loading.

Design parameters for this alternative are presented below. These recommendations are intended to limit post-construction movement to less than one inch. These recommendations may be reexamined once structural data are known for the project.

- 1. The footings, bearing pads, and retaining walls to be placed on the prepared native soil should be designed using a maximum allowable bearing capacity ($\mathbf{q_a}$) of 2,000 psf. This allowable bearing capacity may be increased to 2,500 psf for short-term dynamic loads (seismic and wind events).
- 2. Additionally, the application of bearing pressures to the subgrade should be limited to the pre-construction overburden pressure at the proposed foundation grade plus 1,000 psf. This will generally result in the above-stated bearing capacity but may reduce the bearing capacity where the depth of excavation to the planned foundation grade is small.

- 3. A minimum dead load of 400 psf should be placed on the foundation to reduce the heave potential of the subgrade. Continuous footings should have a minimum width of 2 feet and isolated pad footings should have a minimum dimension of 3 feet.
- 4. After excavation to foundation depth, the exposed soil surface should be proof-compacted using vibratory or roller compaction equipment to provide a uniformly dense surface prior to placement of footing forms. If the presence of large rocks makes disturbing the native soils below footing elevation unavoidable, then the rocks should be removed and replaced with compacted structural fill. If soft or yielding soils are encountered in the trench bottoms, they should be removed until firm material is encountered and replaced with compacted structural fill. If the soft or yielding areas appear to be more extensive, we should be contacted to assess the soil conditions and prescribe remedial action.
- 5. Once the excavation is exposed, but prior to placement of any fill or footing forms, a representative of Buckhorn Geotech must be called out to verify the nature and density of the foundation excavations, to ensure that uniform subsurface conditions are present and to confirm that our recommendations are consistent with actual conditions. If we do not verify the subgrade conditions, Buckhorn Geotech cannot be held responsible for recommendations that may be inconsistent with actual conditions.
- 6. Observation and testing during construction is essential to ensure that the geotechnical recommendations are consistent with conditions and that the project is constructed in compliance with project design and specifications. Any geotechnical observations or testing will be provided at additional charge and we should be contacted at least 2 days in advance for scheduling site visits.
- 7. Foundation walls should be designed with sufficient strength to resist lateral earth pressures and to bridge an unsupported span of at least 10 feet. The components of the foundations should be sufficiently interconnected to ensure that they act as a unit. This will provide resistance to the forces associated with soil movement and will provide unity to the foundation systems.

Floor Systems

Slabs on-grade may be used at the site for the basement parking garage, but special precautions, as outlined below, will be needed to minimize potential damage from the potential subgrade heave. It should be noted that potential swell pressures within the subgrade exceeds the dead weight of a typical floor slab. The following recommendations will minimize, but cannot eliminate, potential slab movement.

1. To provide an adequate bearing surface, topsoil and organic material should be stripped. The subgrade material should be proof-compacted and soft spots removed and replaced with washed rock or structural fill. A minimum one foot of over-excavation and replacement with structural fill should be used under all interior floor slabs to reduce the potential heave to one inch. If it is desired to reduce the heave potential to approximately

- one inch, two feet of overexcavation and replacement with structural fill should be undertaken. If any additional fill is needed to elevate the slab area to the desired foundation grade, this can be accomplished using structural fill.
- 2. To provide a capillary break, slabs on-grade should be placed on 4 inches of ¾-inch to 1½-inch washed rock on the prepared subgrade. Where moisture-sensitive interior floor finishes are applied to the slab, an unpunctured vapor barrier between the gravel and the floor slab is also recommended.
- 3. The garage floor should be structurally separate from the foundation, bearing walls, and interior partitions so that the slab can "float" freely in response to soil volume changes.
- 4. Under-slab plumbing should be avoided to minimize the potential for leakage under the slab. Where necessary, under-slab plumbing should be provided with flexible couplings and should be leak-tested prior to being placed in service.
- 5. Slab sections constructed upon the native subgrade should be designed using a vertical subgrade modulus of 140 pci. A 25 pci increase in the subgrade modulus may be granted for each 6 inches of structural fill placed under the slab.
- 6. Groundwater was encountered during our field investigation at approximately the proposed slab elevation. We recommend an under-slab drain system to minimize the possibility of water ingression during a high groundwater event. This would comprise a system of perforated collection pipes surrounded by free-draining granular fill with an hydraulic connection to either daylight or to a sump pump. Additionally, the slab must be waterproofed and designed to resist potential hydraulic forces in the event of a malfunction of the underdrain system.

Exterior Concrete Flatwork

- 1. Flatwork may be placed on undisturbed native soil with the topsoil and organic material removed. If fill is needed, it should consist of washed rock or structural fill, placed and compacted in accordance with project specifications.
- 2. Flatwork adjacent to buildings should not be placed over loosely compacted fill. To minimize future settlement and damage to the flatwork and/or adjacent foundations, the fill should consist of approved material placed and compacted per project specifications (see the *Retaining Structures* Section, following).
- 3. Flatwork adjacent to exterior doorways should be dowelled into the foundation to prevent long-term differential movement between the flatwork and structure.
- 4. Exterior concrete flatwork should be designed and constructed so that it drains freely away from the structure. Concrete flatwork adjacent to the foundation should slope away at a grade of at least ¼-inch per foot.

Retaining Structures

1. Walls acting to restrain soil should be designed using the lateral earth pressures given in Table 1 below. These values assume a level backslope with no hydraulic pressures behind the wall, the use of native soil, and no surcharge loads applied within the backslope zone (as defined on the attached Retaining Wall Schematic). We should be contacted to recommend lateral earth pressure values for increased backslope angles or loading within the backslope zone.

Table 1. Lateral Earth Pressures

| irlamentu akanyak (dalipinga | | Nativ | ve Soil |
|------------------------------|--|-------|---------|
| Activ | e Earth Pressure | 38 | pcf* |
| Pass | ive Earth Pressure | 400 | pcf* |
| At-Re | est Earth Pressure | 63 | pcf* |
| Unit v | veight of soil | 125 | pcf** |
| Coef | ficient of Friction | 0.32 | *** |
| * | pounds per cubic foot (fluid equivalent) | | |
| ** | pounds per cubic foot | | |
| *** | concrete on dry soil conditions | | |

- 2. The retaining walls should have provisions for drainage so that hydrostatic pressures are not allowed to build up. This is usually accomplished by providing free-draining granular backfill between the wall and retained soil, with a collection drain provided at the bottom of this granular zone (shown in the attached Retaining Wall Schematic), and/or the use of weep holes through the face of the wall. The drain system should be continuous and have a positive outfall which releases the collected water well away from the wall in a manner that minimizes the erosive energy of concentrated flow. The design engineer should ensure that drainage design is compatible with design assumptions.
- 3. Excavations for retaining and foundation walls should be laid back a minimum of 35° from the vertical prior to backfilling against retaining structures (see attached Foundation Excavation Detail). For safety, excavations should also be in accordance with OSHA Regulations 29 CFR 1926. Consequently, gentler excavation faces may be required.
- 4. Fill material placed behind the walls may consist of on-site material compacted as per the design engineer's specifications. Compaction of 95% of Standard Proctor maximum dry density at 0 to 4% above optimum moisture content is typically used to minimize post-construction settlement of the backfill. Over-compaction of the backfill should be avoided so that excessive pressures are not placed against the stemwalls. Unless expressly approved by the design engineer, only hand-operated light-duty compaction equipment should be used within three feet of the wall. The upper one foot of backfill should consist of clayey soil to create a barrier against infiltration of surface runoff.

Concrete

A water-soluble sulfate test conducted on a sample of the subsurface materials encountered in our excavations showed sulfate concentrations less than 0.02%, considered by the American Concrete Institute (ACI) to be a "negligible" sulfate exposure. We recommend Type I/II cement be used in all concrete at this site.

Foundation Drainage and Ventilation

It is important to prevent moisture from penetrating into the soil beneath or adjacent to the structure. Moisture can accumulate as a result of poor surface drainage, over-irrigation of landscaped areas, waterline leaks, melting snow, subsurface seepage, or condensation from vapor transport.

- 1. Provisions should be made to evacuate subsurface moisture accumulation from around foundations and under slabs. This may be accomplished using conventional footing and under-slab drains in tandem with a positively-vented moisture and radon control system. Alternatively, consideration may be given to using concrete forms that facilitate both dewatering and the removal of radon gases and vapors.
- 2. An aggressive system of both construction and permanent dewatering is needed due to the deep excavations planned for this lot. If the foundation drains cannot discharge by gravity, a pump with emergency backup provisions will be needed.
- 3. Roof drainage should be captured by eave gutters. Downspouts should discharge a minimum of 10 feet away from the house or piped into a closed underground drain system and evacuated off-site. In no case should the downspouts be directed into the foundation or under-slab drain system. These points of discharge should be identified in the site drainage plan so that water is readily removed from the site. Snow gates and/or heat traces should be employed to minimize snow and ice accumulation on the components of the roof drainage system.
- 4. All foundation drains should be integrated into the site drainage plan as discussed below for final disposal from the building site. In no case should surface or roof drainage be introduced into the foundation or under-slab drain system.
- 5. Floor systems and confined areas above concrete floor slabs should be properly ventilated to allow for the release of radon gas. See the *Radon Gas* Section of this report for more radon information.

Site Preparation and Grading

1. The site drainage plan, in tandem with the landscape and grading plans, should ensure that the construction does not impede natural drainage patterns. Surface water should be removed and not allowed to accumulate or stand anywhere near the building foundation

either during or after completion of construction. This includes water from landscaped areas, patios, decks, and roofs. Drainage plans should ensure that precipitation, snowmelt, and runoff are conveyed around and away from the building as well as the driveway. This runoff should be dispersed (not concentrated) in a manner consistent with the natural, pre-construction drainage pattern.

- 2. Final grading around the perimeter of the foundation should slope downward with at least one foot of drop within the first 10 feet of horizontal distance. Concrete flatwork adjacent to the foundation should slope away at a grade of at least ¼-inch per foot.
- 3. Development should utilize "best practices" for design and construction so that on-site erosion is minimized. This may include selective thinning of vegetation, construction of temporary diversion ditches, silt fencing, and/or dust suppression. On-site erosion control should be planned and executed in conformance with Colorado Department of Public Health and Environment (Water Quality Control Division) stormwater discharge regulations. The local building official will be able to provide specific details regarding these requirements.
- 4. Grading of all permanent cut and fill slopes should not exceed 2H:1V. All slopes greater than 2H:1V and over 3 feet in vertical height should be restrained by an engineered retaining structure/system.
- 5. Backfill placed in utility trenches leading to the building should be compacted in accordance with project specifications. This will inhibit surface water infiltration and migration towards the foundations, as well as minimize post-construction settlement of the trench backfill. Particular attention should be given to preventing transmission of subsurface water through permeable fill used for bedding in the trenches, through the use of substitute materials or check dams.
- 6. Disturbed areas should be revegetated as soon as practical to reduce soil erosion.
- 7. Imported fill used at this site should meet the gradational and compaction requirements listed in Tables 2 and 3 below. Fill should be placed and compacted in **maximum 6-inch lifts**, unless otherwise directed by the design engineer. Structural fill should not be placed on frozen or wet native soil. It is recommended that the foundation excavation be open a minimum period of time to avoid degradation of the foundation soils.

Table 2. Gradation Requirements for Imported Fill Material

| Type | Sieve | %Passing, by weight |
|---|-------------------------|---------------------|
| Structural Fill (CDOT Class 6 roadbase) | 3/4" (19.0 mm) | 100 |
| | #4 (4.75 mm) | 30-65 |
| | #8 (2.36 mm) | 25-55 |
| | #200 (0.075 mm) | 3-12 |
| Structural Fill (CDOT Class 1) | 2.5" (63.5 mm) | 100 |
| , | 2" (50 mm) [′] | 95-100 |
| | #4 (4.75 mm) | 30-65 |
| | #200 (0.075 mm) | 3-15 |
| Fill under exterior concrete flatwork | 3" (75 mm) | 100 |
| | #200 (0.075 mm) | 0-5 |
| Free-draining fill | 3" (75 mm) | 100 |
| J | ¾" (19 mm) | 20-90 |
| | #4 (4.75 mm) | 0-20 |
| | #200 (0.075 mm) | 0-3 |

Note: The Plasticity Index for all fill soils should be less than 6.

Table 3. Compaction Requirements for Fill Material

| Application | Compaction Requirement | Proctor | Moisture |
|--------------------------|-----------------------------|----------|----------------|
| Under footings and slabs | 95% max. dry density | Modified | ±2% of optimum |
| Under exterior flatwork | 90% max. dry density | Modified | ±2% of optimum |
| Behind retaining walls | Per project specifications* | | |
| Utility Trenches | Per project specifications* | | |
| General landscaping | Per project specifications* | | |

^{*}As specified by the design engineer on project documents or in accordance with local municipal requirements,

- 8. Any soils containing organics, debris, topsoil, frozen soil, snow, ice, and other deleterious materials shall not be used for anything other than landscaping unless authorized by the foundation engineer.
- 9. A representative of Buckhorn Geotech should be called out to the site to observe placement of structural fill and verify the compacted density. The owner should contact Buckhorn Geotech in advance of the excavations to discuss the specific testing requirements, budget, and scheduling needed for these services.

Excavation and Shoring

We understand the garage floor level will be approximately 19 feet below existing grade at the highest point, adjacent to Mountain Village Boulevard. The resulting excavation will require temporary support during construction as spatial site constraints may not allow laying back the excavation to the extent needed to create stable slopes. Failure to provide excavation support could endanger construction personnel and could undermine Mountain Village Boulevard including any utilities buried under the road. There are numerous methods of providing support for the excavation walls. Below some systems are described in brief for planning purposes. Buckhorn Geotech can provide design services for the excavation support.

Soil nails may be used efficiently for steep or vertical cuts, provided that they can effectively resist the traffic and structural loads. Soil nails are small-diameter steel bars that are horizontally drilled and grouted into the wall face as excavation progresses downward. As each lift (typically five feet) is excavated in a hillside, the bars are installed, heavy gauge wire mesh and/or reinforcing steel is set at the face, and the face is shotcreted. Thus, it is a top-down construction method that allows work to begin immediately at the floor of the excavation once completed. The walls can be designed for temporary or permanent use. In other words, the soil nails can be used to permanently retain the soils around the structure so that the lateral forces against the walls are reduced.

Another option is the use of soldier beams and/or sheet piling that can be driven or drilled into the soil prior to excavation and used to provide cantilevered support for the retained soil in smaller cuts. Larger cuts may be made using this system together with post-tensioned soil anchors installed in a similar manner to the soil nails. The height of the retained cut will depend on the size of piles and the depth to which they are installed. We can assist with this decision once we are provided details regarding building design.

- 1. Temporary excavations should be in accordance with Occupational Safety and Health Administration (OSHA) regulations and with worker safety in mind.
- 2. Construction equipment, materials, and soil stockpiles should be located a minimum horizontal distance equal to the height of the excavation from the crest of the excavation unless otherwise approved by the design engineer.
- 3. Based upon our investigation, the silty clay and jumbled shale above the groundwater would be most nearly represented by an OSHA Type B soil. Our assessment is based upon the soil and groundwater conditions encountered in limited investigation and sampling. The contractor's "competent person" (defined by OSHA as "an individual capable of identifying existing and predictable hazards...and who has the authorization to take prompt corrective measures to eliminate or control these hazards and conditions) should evaluate the soil materials exposed during excavation based on composition, structure, and environmental conditions per 29 CFR 1926 and recommend appropriate slope laybacks or shoring, as required. Refer to OSHA's Technical Manual Section V: Chapter 2 on Excavations: Hazard Recognition in Trenching and Shoring (available on-

line at: www.osha.gov) for further excavation guidelines. We are happy to provide these services, as requested.

- 4. If the excavations will be made or remain open during wet weather, it is recommended that polyethylene sheeting be secured over the excavation face to minimize sediment runoff and deterioration of the foundation soils. Surface runoff above the cuts should be directed away from the excavation using berms or diversion ditches. Large rocks exposed in the excavation face should be scaled for worker safety.
- 5. Excavation dewatering may be required if the excavation is made during peak groundwater season (springtime). Dewatering may be accomplished with conventional sump pumping located below the floor of the excavation. The excavation should be designed to accommodate clarification and discharge of this intercepted water. The Town of Mountain Village Building Department should be contacted for specific regulations regarding the discharge of groundwater off-site.
- 6. Excavations should be performed during the low groundwater season (late summer through early spring) to minimize the amount of water that needs to be removed during shoring and construction operations. This will also minimize pumping of the soil so that maximum densities can be achieved.
- 7. We anticipate that the excavation of the site soils can be accomplished by conventional excavating equipment.

Closing Considerations

Standard of Care and Interpretation of Subsurface Data

This report has been prepared in a manner consistent with local standards of professional geotechnical engineering practice. The classification of soils and interpretation of subsurface conditions is based on our training and years of experience, but is necessarily based on limited subsurface observation and testing. As such, inferred ground conditions cannot be guaranteed to be exact. No other warranty, express or implied, is made.

Inspection of the excavation(s) subgrade by Buckhorn Geotech prior to erection of the foundation system is an integral part of these recommendations. If subsurface conditions differing from those described herein are discovered during excavation, construction should be stopped until the situation has been assessed by a representative of Buckhorn Geotech. Construction should be resumed only when remedies or design adjustments, as necessary, have been prescribed.

Investigation for environmental contaminants was not part of our scope of services performed at this site.

Use of This Report

This report is intended for use by the client specifically to address the site and subsurface conditions as they relate to the proposed structure(s) described in the *Construction Plans* Section. Changes to the site or proposed development plans may alter or invalidate the conclusions and recommendations contained herein.

Buckhorn Geotech retains an ownership and property interest in this report. Consistent with the industry, copies of this document that may be relied upon by the client are limited to printed copies (also known as hard copies) that are signed and sealed by the Geotechnical Engineer (Standard Form of Agreement Between Owner and Geotechnical Engineer for Professional Services, Engineer's Joint Contract Documents Committee, 1996). Any conclusions or information obtained or derived from electronic files, data, or graphics will be at the user's sole risk. This report together with ancillary data, analyses, test results, and other components and/or supporting parts are not intended or represented to be suitable for reuse by the client or others on extensions to this project or on any other project. Any such reuse or modification invalidates all aspects of the report and excuses the Geotechnical Engineer for all responsibility and liability or legal exposure.

This report is considered valid for a period of two years from the date of issue provided the site conditions and development plans have not changed from what is referenced in this report. Changes to the site may occur due to development or natural processes. Additionally, technological advances made in construction and changes in legislation may alter the recommendations made herein. Depending upon the site and proposed development changes, Buckhorn Geotech may require additional investigation (at additional cost) to update the recommendations contained herein.

Retention of Samples

Samples of soil and rock collected during the course of our geotechnical investigation(s) are routinely held in our laboratory for a period of three months from the date of the investigation, then discarded. A written request by the client is required for samples to be stored for a longer period.

Additional Services

To provide continuity and consistency from project start to finish, we should be retained to make observations and carry out material testing as a service to the owner. As noted above, we recommend the owner contact us to discuss required services and scheduling in advance of the construction phase.

Buckhorn Geotech is a full-service engineering firm providing foundation, on-site wastewater system, site drainage, structural, and retaining structure design services, as well as surveying, construction materials testing, and inspections. Please visit www.BuckhornGeo.com for a full description of our services.

Thank you for the opportunity to perform this soil investigation for you. If you require any of these services or have any questions regarding this report, please do not hesitate to contact us.

Respectfully Submitted:

July 5, 2007

Reviewed by: July 5, 2007

TRANSMITTED ELECTRONICALLY

Shane M. Duckworth, P.E. Geotechnical Engineer

Thomas E. Griepentrog, P.E., P.G.

Principal

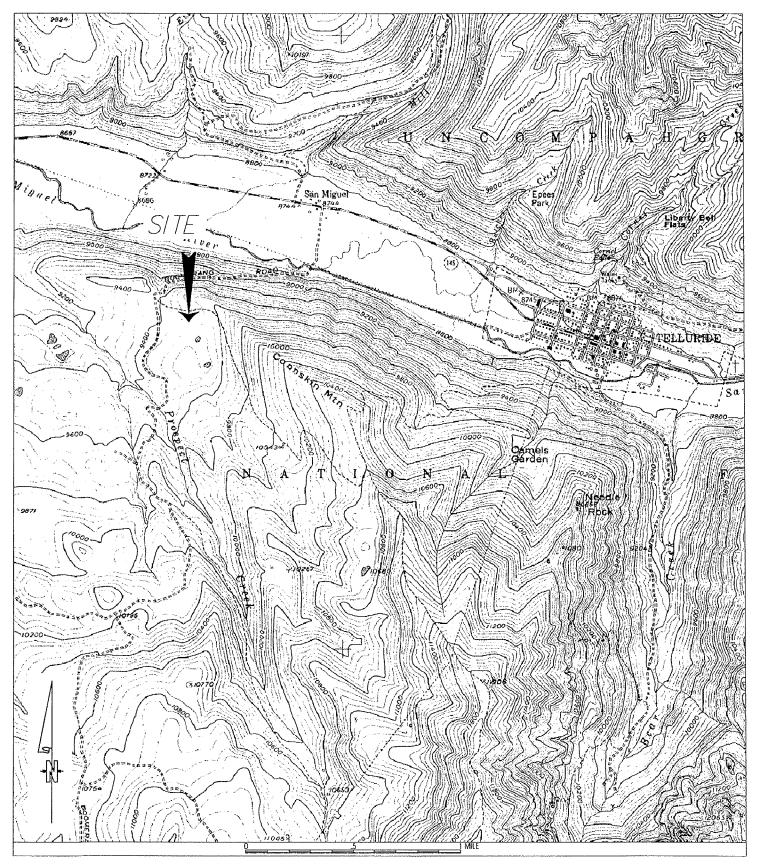
Enclosures:

Vicinity Map, Site Map, Drill Log Key, Swell/Consolidation graph,

Corrosivity Test results, Retaining Wall Schematic, Glossary of Soils and

Engineering Terms

VICINITY MAP



| MAP | INVESTIGATION | SD |
|--------|---------------|------------|
| NUMBER | DRAFTING | SD |
| 1 | DATE | 5/22/07 |
| 0F 2 | IOP NO | 07-161-GF0 |

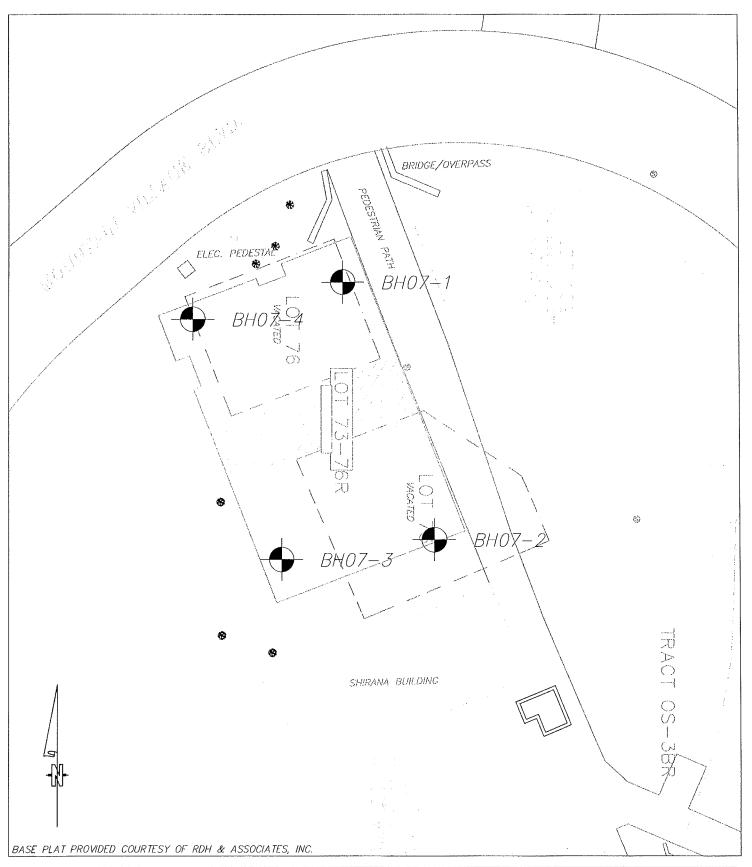
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ALPENGLOW

MOUNTAIN VILLAGE, COLORADO

BUCKHORNGEOTECH

SITE PLAN



| MAP NUMBER | INVESTIGATION | SD |
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| OF 1 | JOB NO. | 07-161-GEO |

RDH & ASSOCIATES, INC.

ALPENGLOW

MOUNTAIN VILLAGE, COLORADO

BUCKHORNGEOTECH

DRILL LOG KEY DRILLER: BOREHOLE LOCATION: DRILL RIG: SURFACE ELEVATION: DRILL STEM: NOTES: SAMPLER: COUNTS SAMPLE NUMBER VALUE Ē FIELD & LABORATORY 77E TENET SOIL DESCRIPTION BLOW £ TEST RESULTS RECOVERY SAMPLE WATER (stratigraphic transitions are approximate and SPT (bpt) SPT are inferred from cuttings and drillers comments) indicates drive sample Notes in this column indicate tests performed indicates bulk sample and test results DD: dry density in cubic indicates core sample pounds per foot (pcf) 10 MC: % moisture content Sample identifier: DS=Drive sample GS=Bulk sample from auger flights LL: Liquid Limit CS=Core sample 15 PL: Plastic Limit Blows required to drive sampler 6 inches each. The first six inches is considered to be the "seating" drive. 10 Plasticity Index 20 indicates seven blows required to drive the sampler 21/12 twelve inches with a 140-lb hammer falling 30 inches Gravel fraction (%) Sand fraction (%) SF: 12 - length of intact soil plug recovered from the sampler 25 F200: Silt/Clay (%) - indicates free water surface at time of drilling Shear resistance - Constituents-30 trace = 0 - 5%P: Penetration resistance little = 0 -12%clay some = 12 - 20% CBR: California Bearing Ratio -ey = 20-30%35 and = >30%SP: swelling pressure silt total movement under consolidation pressure TM: Unified Classification System (ASTM D-2487) 40 sand CL = lean clay to sandy/gravelly lean clay psf: pounds per sq. ft. ML = silt to sandy/gravelly silt pcf: pounds per cu. ft. $\it CH = high\ plasticity\ clay\ to\ sandy/gravelly\ high\ plasticity\ clay$ gravel MH = high elasticity silt to sandy/gravelly high elasticity silt SW = well-graded sand or well-graded sand with gravel shale SP = poorly graded sand or poorly graded sand with gravel 50 SM = silty sand to silty sand with gravel -hard bedrock SC = clayey sand to clayey sand with gravel GW = well-graded gravel or well-graded gravel with sand GP = poorly graded gravel or poorly graded gravel with sand GM = silty gravel or silty gravel with sand 60 GC = clayey gravel or clayey gravel with sand * SPT N-values not corrected for energy or depth SHEET INVESTIGATION **BUCKHORN**GEOTECH NUMBER

DRILL LOG KEY

Civil, Structural, and Geotechnical Engineers, Inc. 222 South Park Avenue

Montrose, Colorado 81401 Phone (970) 249-6828 Fax (970) 249-0945

DRAFTING

JOB NO.

DATE

OF 1

DRILL LOG - BOREHOLE 1 (BH07-1)

BOREHOLE LOCATION: Northeast corner of property

SURFACE ELEVATION: Approx. 9521'

NOTES:

DRILLER: Ager Drilling

DRILL RIG: Bombardier CME45

DRILL STEM: 4" Solid-stem continuous flight auger

SAMPLER: 2" I.D. California split spoon

| | GRAPHIC | WATER LEVEL | SAMPLE TYPE | SAMPLE NUMBER | SPT BLOW COUNTS | *SPT 'N' VALUE (bpf) | RECOVERY (in.) | SUBSURFACE DESCRIPTION | FIELD & LABORATORY TEST RESULTS |
|---------------------|---------|-------------|-------------|---------------|------------------|----------------------|----------------|--|---|
| 10 — | | ▽ | | -DS1 -DS2 | 8 9 8 9 | 18 | 16 | reddish—brown, sandy angular gravel [FILL] with trace clay (0-2') dark brown to black, organic clayey GRAVEL (2-3.5') drive sample DS1 @4.5-5.5' reddish—brown, mottled grey and iron stained, stiff to very stiff, moist, CLAY and SILT with little sand and organics groundwater @9.3 after drilling drive sample DS2 @9.5-10.5' same in upper 12" or so, then grey to brown, very stiff, damp, clayey SAND (shale chips, jumbled) | DS1 MC=22.6% Sulfates=0.010% Chlorides=15 ppm EC=27 µS/cm pH=7.30 DS2 MC=18.8% |
| 20 — | | | | -DS3 | 67 | 14 | 16 | drive sample DS3 @14.5—15.5' jumbled SHALE, mostly chips, coarse sand size rock @19—20', easily drilled through drive sample DS4 @24.5—25.5' same but large chunk of hard thinly bedded shale within sampler | DS3 MC=18.6% Sulfates=0.020% Chlorides=15 ppm EC=15 µs/cm pH=6.82 0.03% swell @200psf +H₂0 SP=220psf @200psf +H₂0 TM=4.7% @8000psf +H₂0 DD=114 pcf |
| 30 — | | | CA | - DS5 | <u>50</u> 0** | >100 | 0 | grinding @33', hard shale? drive sample DS5 @34.5—35.5' occasional siltstone seams (inferrred from drill behavior) end of borehole @39.5' hard slow drilling, lots of water | |
| - - - 50 - | * SPT N | '-val | ues n | ot coi | rrecte | ed for | · energ | groundwater @26.6' 6/12/07 groundwater @26.6' 6/12/07 groundwater @26.6' 6/12/07 | m cuttings & drillers comments |

| SHEET | INVESTIGATION | SD |
|--------|---------------|------------|
| NUMBER | DRAFTING | CD |
| 1 | DATE | 5/22/07 |
| OF 1 | JOB NO. | 07-161-GEO |

ALPENGLOW LOTS 73/76

RDH & ASSOCIATES

MOUNTAIN VILLAGE, COLORADO

BUCKHORNGEOTECH

DRILL LOG - BOREHOLE 2 (BH07-2)

BOREHOLE LOCATION: Southeast corner of property

SURFACE ELEVATION: Approx. 9521'

NOTES:

DRILLER: Ager Drilling

DRILL RIG: Bombardier CME45

DRILL STEM: 4" Solid-stem continuous flight auger

SAMPLER: 2" I.D. California split spoon

| | | | | , | , | | | | |
|-------------|--------------|-------------|-------------|---------------|-----------------|----------------------|----------------|---|---|
| DEPTH (R.) | GRAPHIC | WATER LEVEL | SAMPLE TYPE | SAMPLE NUMBER | SPT BLOW COUNTS | *SPT 'N' VALUE (bpf) | RECOVERY (in.) | SUBSURFACE DESCRIPTION | FIELD & LABORATORY TEST RESULTS |
| | 0080 | | | | | | | TOPSOIL (0—1') | |
| | | | | | | | | reddish-brown clayey GRAVEL (1-1.5) then CLAY | |
| | | | CA | -DS6 | 12 12 | 24 | 12 | drive sample DS6 @4.5–5.5' dark brown to grey to black, moist, very stiff to hard, silty CLAY with some sand (shale chips), mottled | |
| 10 — | | | CA | -DS7 | 10 9 | 18 | 16 | drive sample DS7 @9.510.5' light reddish-brown, very stiff, sandy CLAY with little to some fine rounded sandstone gravel | DSZ LL=30 PL=16 Pl=14 GF=26.9% SF=22.4% |
| | | | CA | -DS8 | 8 10 | 20 | 16 | drive sample DS8 @14.5—15.5' dark grey to brown, moist, very stiff, CLAY and SAND, jumbled shale | P200=50.7% MC=17.8% DS8 LL=29 PL=20 Pl=9 |
| 20 | | | CA | - <i>DS9</i> | 9 14 | 28 | 16 | drive sample DS9 @19.520.5' dark grey to brown, moist to damp, very stiff to hard, sandy (shale) CLAY, jumbled shale | GF=15.3% SF=44.3% F200=40.4% MC=17.9% |
| 30 | | ₹ | CA | - DS10 | 14 19 | 36 | | drive sample DS10 @29.5—30.5' same groundwater @31.8' after drilling grinding @32'— tough drilling, pulled auger and replaced teeth, new teeth wore off in next foot | |
| _ | | | | | | | | end of borehole @33' due to refusal groundwater @10.7' on 6/12/07 | |
| | | | | | | | | groundwater Gro.7 on 6/12/07 | |
| 40 | | | | | | | | | |
| 40 | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| | | | | | | | | | |
| 50 — | | | | | | | | | |
| | · • SPT N | Ival | ues n | ot coi | recte | d for | ene | rgy or depth; stratigraphic transitions are approximate and are inferred from | m cuttings & drillers comments |

| SHEET | INVESTIGATION | SD |
|--------|---------------|------------|
| NUMBER | DRAFTING | CD |
| 1 | DATE | 5/22/07 |
| OF 1 | JOB NO. | 07-161-GEO |
| | | |

ALPENGLOW LOTS 73/76

RDH & ASSOCIATES

MOUNTAIN VILLAGE, COLORADO

BUCKHORNGEOTECH

DRILL LOG - BOREHOLE 3 (BH07-3)

BOREHOLE LOCATION: Southwest corner of property

SURFACE ELEVATION: Approx. 9516'

NOTES:

DRILLER: Ager Drilling

DRILL RIG: Bombardier CME45

DRILL STEM: 4" Solid-stem continuous flight auger

SAMPLER: 1.375" I.D. Standard and 2" I.D. California split spoon

| | | | | | · | | · | · | | |
|--|--------------------|-----------|-------------|-------------|------------------|----------------------------------|----------------------|----------------|---|------------------------------------|
| | <i>ре</i> ртн (п.) | GRAPHIC | WATER LEVEL | SAMPLE TYPE | SAMPLE NUMBER | SPT BLOW COUNTS | "SPT 'N" VALUE (bpf) | RECOVERY (in.) | SUBSURFACE DESCRIPTION | FIELD & LABORATORY TEST RESULTS |
| | | 0000 | | | | | | | TOPSOIL (0-1') | |
| | | 900 | | | | | | | gravelly CLAY (1-3') | |
| | _ | | | | | | | | black to grey, damp to moist, stiff, CLAY with shale chips (3—8') | |
| | _ | | | | | | | | rocks @8' hard shale/sandstone, no sample possible | |
| 10 | | | | | - DS11 - DS11 | 14 9 14 <u>50</u> 2" | 23 >100 | 12 6 | drive sample DS11 @1112.5' reddish-brown, moist to damp, stiff to very stiff, SILT and CLAY with some sandstone gravel at bottom of sample (sampler refusal) | |
| | | | | | | | | | out of gravel into clay @ 17' | |
| 20 | | | | CA | DS12 | 16 20 | 40 | 16 | drive sample DS12 @19.5—20.5' dark grey to black, damp, hard, silty CLAY with trace shale flecks, mottled, iron stained | |
| 30 | | | | CA | - DS13 | 15 20 | 40 | 15 | drive sample DS13 @29.5—30.5° same, jumbled shale, shale pieces are highly weathered, extremely weak, bedding in sample observed between 70—90° to horizontal | |
| All the second of the second o | | | 록 | | | | | | groundwater @37.3' after drilling | |
| 40 | | V- 1 - | L | L | | 1 | | | hard shale @39' end of borehole @40'; teeth ground to nothing groundwater @11.9' on 6/12/07 | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | | | | | | | | | | |
| | - | | | | | | | | | |
| 50 | | # CDT | l | lues - | not oc | roote | ad for | rena | rgy or depth; stratigraphic transitions are approximate and are inferred fro | m cuttings & drillers comments |
| 1 | , | 3P 1 1 | · ··· VUI | ues / | 01 001 | 10010 | u 101 | GIIG | rgy or depart, stratigraphic transitions are approximate and are interred ind | in outtings & unifors commonts |

| SHEET | INVESTIGATION | SD |
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| NUMBER | DRAFTING | CD |
| 7 | DATE | 5/22/07 |
| OF 1 | JOB NO. | 07–161–GEO |

ALPENGLOW LOTS 73/76

RDH & ASSOCIATES

MOUNTAIN VILLAGE, COLORADO

BUCKHORNGEOTECH

DRILL LOG -- BOREHOLE 4 (BH07-4)

BOREHOLE LOCATION: Northwest corner of property

SURFACE ELEVATION: Approx. 9521'

NOTES:

DRILLER: Ager Drilling

DRILL RIG: Bombardier CME45

DRILL STEM: 4" Solid-stem continuous flight auger

SAMPLER: 2" I.D. California split spoon

| | | | · · · · · · · · · · · · · · · · · · · | | | | SAMPLER: 2" I.D. California split spoon | |
|------------|------------------------|-------------|---------------------------------------|-----------------|----------------------|----------------|--|---|
| ОЕРТН (п.) | GRAPHIC WATER LEVEL | SAMPLE TYPE | SAMPLE NUMBER | SPT BLOW COUNTS | *SPT 'N' VALUE (bpf) | RECOVERY (in.) | SUBSURFACE DESCRIPTION | FIELD & LABORATORY TEST RESULTS |
| | | | | | | | black, gravelly, organic TOPSOIL (0–2') dark brown, moist, stiff, clayey SILT (2–7') reddish—brown, sandy CLAY ©7' | |
| 10 — 9 | | CA | -DS14 | 26 19 | 38 | 16 | drive sample DS14 @9.5—10.5' reddish—brown, damp to moist, very stiff to hard, subrounded to subangular gravelly CLAY with iron staining | <u>DS14</u> MC=13.3% |
| | 9 | CA | - DS15 | <u>50</u> 3" | >100 | 3 | drive sample DS15 @14.5—15.5' rock @2" (SPT); only small sample, similar to above, some sandstone gravel | |
| 20 — | | CA | -DS16 | 7 8 | 16 | 16 | drive sample DS16 @19.5—20.5' same, quite mottled, softer, more moist | <u>DS16</u> 0.8% swell @600psf +H₂0 SP=840 psf @600psf +H₂0 TM=11.64% @10000psf +H₂0 DD=103.4 pcf MC=14.3% |
| 30 — 6 | | CA | - DS17 | 12 29 | 58 | 16 | drive sample DS17 @29.5—30.5' brown, hard, clayey SILT with some fine sand and trace gravel | |
| | | -01 | 2013 | 11 | 64 | 16 | transistion to black cuttings at approximately 37' | |
| 40 | | CA | - DS18 | 11 32 | 64 | 16 | drive sample DS18 @39.5—40.5' black, damp to moist, very stiff, CLAY with shale chips, scratching, hard @44' drill to 48.5', then refusal | |
| 50 -* 54 | 訓 27 N-v | alues r | not cor | recte | ed for | energ | end of borehole @48.5' due to refusal carbide teeth shot, rock at end is sandstone groundwater @42.2' on 6/12/07 ny or depth; stratigraphic transitions are approximate and are inferred from | m cuttings & drillers comments |

| SHEET | INVESTIGATION | SD |
|--------|---------------|------------|
| NUMBER | DRAFTING | CD |
| 1 | DATE | 5/22/07 |
| OF 1 | JOB NO. | 07-161-GEO |

ALPENGLOW LOTS 73/76

RDH & ASSOCIATES

MOUNTAIN VILLAGE, COLORADO

BUCKHORNGEOTECH



222 South Park Ave. • Montrose, CO 81401 Ph.: (970) 249-6828 • FAX: (970) 249-0945

Corrosivity Series

Based on HACH methods

| Project Name | Alpenglow | Date Tested | 5/23/2007 |
|------------------|---|-------------|------------|
| Project Location | Lots 73 & 76, Mountain Village, Colorado | Project # _ | 07-161-GEO |
| Client | RDH & Associates, Inc | Sample by | SD |
| Test Location | BH07-1 @4.5-5.5' | Tested by | DNJ |
| Sample # | DS1 | - | |
| Soil Description | brown mottled lean CLAY with pieces of coal and little gravel | | |

In-situ Moisture Content

22.6 %

Water-soluble sulfates, dry soil basis

0.010 %

Chlorides

15 ppm

Electro-conductivity

27 μS/cm

pН

7.30



Civil, Structural & Geotechnical Engineers

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In-Situ Moisture Content

ASTM D 2216

 Project Name
 Alpenglow
 Date
 5/23/2007

 Project Location
 Lots 73 & 76, Mountain Village, Colorado
 Project # 07-161-GEO

 Client
 RDH & Associates, Inc
 Sample by
 SD

 Sample #
 DS2, DS14
 Tested by
 DNJ

| Sample # | Bowl ID | Tare (g) | Tare + wet (g) | Tare + dry (g) | % Moisture |
|----------|---------|----------|----------------|----------------|------------|
| DS2 | 1A | 99.7 | 256.7 | 231.9 | 18.8% |
| DS14 | DD | 97.4 | 241.0 | 224.1 | 13.3% |



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Corrosivity Series

Based on HACH methods

| Project Name | Alpenglow | Date Tested_ | 5/23/2007 |
|------------------|--|--------------|------------|
| Project Location | Lots 73 & 76, Mountain Village, Colorado | Project # | 07-161-GEO |
| Client | RDH & Associates, Inc | Sample by | SD |
| Test Location | BH07-1 @14.5-15.5' | Tested by | DNJ |
| Sample # | DS3 | _ | |
| Soil Description | very dark brown lean CLAY (shale) | | |

In-situ Moisture Content

18.6 %

Water-soluble sulfates, dry soil basis

0.020 %

Chlorides

15 ppm

Electro-conductivity

15 µS/cm

рH

6.82



Civil, Structural & Geotechnical Engineers

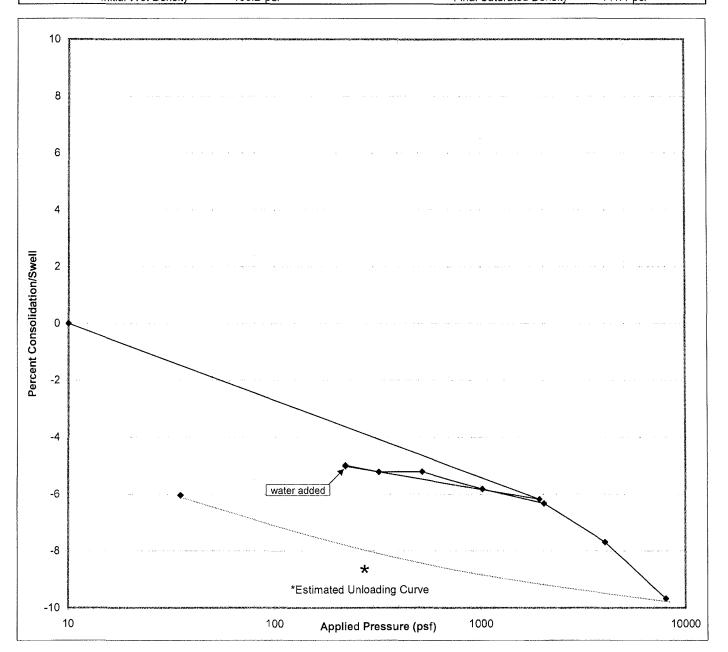
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Swell/Consolidation Test

ASTM D4546

Project Name Alpenglow Date 5/23/2007 Project Location Lots 73 & 76, Mountain Village, Colorado Project # 07-161-GEO Client RDH & Associates, Inc. Sampled by SD Sample Location BH07-1 @14.5-15.5' Tested by DNJ Sample # DS3 Soil Description very dark brown lean CLAY (shale)

Initial compression due to 1900 psf pressure = 6.18% Swell potential due to water and 200 psf pressure = 0.03% Total consolidation due to water and 8000 psf pressure = 4.69% Estimated swell pressure = 220 psf Initial Moisture Content 18.6 % Final Moisture Content 15.4 % Initial Dry Density 114.0 pcf Final Dry Density 122.5 pcf Initial Wet Density 135.2 pcf Final Saturated Density 141.4 pcf





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Sieve Analysis and Atterberg Limits

Project Name

Alpenglow

Date

7/3/2007

Project Location Lots 73 & 76, Mountain Village, Colorado

Project #

07-161-GEO

Client

RDH & Associates, Inc.

Sample by

SD

Test Location

BH07-2 @9.5-10.5'

Tested by

VΒ

Sample #

DS7

Sieve Analysis

ASTM C136 / C117

| Sieve | Opening (mm) | % Passing | |
|-------|-----------------|-----------|--|
| 3" | 76.2 | 100.0 | |
| 3/4" | 19.0 | 89.1 | |
| 3/8" | 9.5 | 79.1 | |
| #4 | 4.75 | 73.1 | |
| #10 | 2.0 | 69.3 | |
| #40 | 0.425 | 66.5 | |
| #200 | 0.075 | 50.7 | |

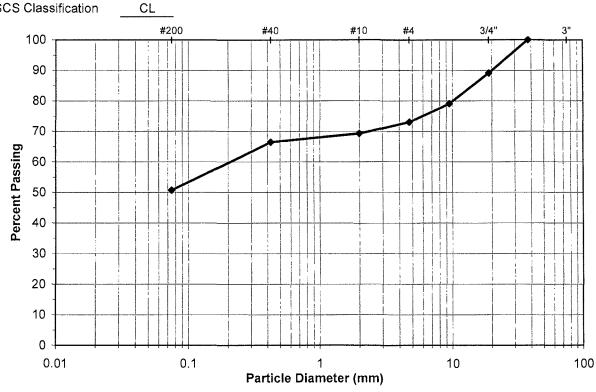
Atterberg Limits

ASTM D4318

| Liquid Limit (LL) | 30 |
|-----------------------|----|
| Plastic Limit (PL) | 16 |
| Plasticity Index (PI) | 14 |
| | |

Natural Moisture Content (%) = 17.8%

Soil Description **USCS** Classification reddish brown gravelly lean CLAY with sand



| Clay/Silt | Fine | Medium | Coarse | Fine | Coarse |
|-----------|------|--------|--------|------|--------|
| FINES | | SAND | | GRA | VEL |

% Fines = 50.7

% Sand = 22.4

% Gravel = 26.9



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Sieve Analysis and Atterberg Limits

Project Name

Alpenglow

Date

7/3/2007

Project Location Lots 73 & 76, Mountain Village, Colorado

Project #

07-161-GEO

Client

RDH & Associates, Inc.

Sample by

SD

Test Location

BH07-2 @14.5-15.5'

Tested by

VΒ

Sample #

DS8

Sieve Analysis

ASTM C136 / C117

| Sieve | Opening (mm) | % Passing |
|-------|--------------|-----------|
| 3" | 76.2 | 100.0 |
| 3/4" | 19.0 | 100.0 |
| 3/8" | 9.5 | 94.7 |
| #4 | 4.75 | 84.7 |
| #10 | 2.0 | 73.2 |
| #40 | 0.425 | 55.4 |
| #200 | 0.075 | 40.4 |

Atterberg Limits

ASTM D4318

| Liquid Limit (LL) | 29 |
|-----------------------|----|
| Plastic Limit (PL) | 20 |
| Plasticity Index (PI) | 9 |
| | |

Natural Moisture Content (%) = 17.9%

Soil Description **USCS** Classification olive-brown clayey SAND with gravel

SC #200 #40 #10 100 90 80 70 Percent Passing 60 50 40 30 20 10 0 0.01 0.1 10 100 Particle Diameter (mm)

| Clay/Silt | Fine | Medium | Coarse | Fine | Coarse |
|-----------|------|--------|--------|------|--------|
| FINES | | SAND | | GRA | VEL |

% Fines = 40.4

% Sand = 44.3

% Gravel = 15.3



Civil, Structural & Geotechnical Engineers

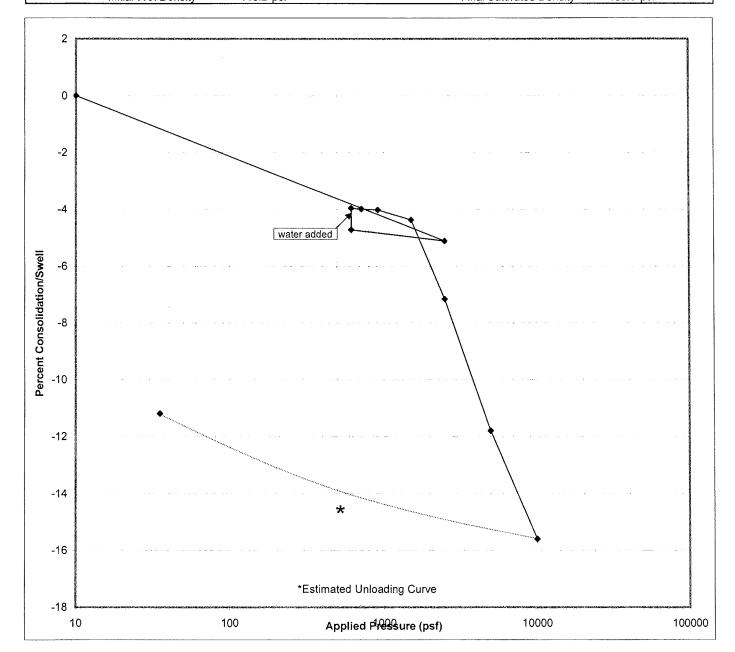
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Swell/Consolidation Test

ASTM D4546

Project Name Alpenglow Date 5/23/2007 Project Location Lots 73 & 76, Mountain Village, Colorado Project # 07-161-GEO Client RDH & Associates, Inc. Sampled by SD Sample Location BH07-4 @19.5-20.5' Tested by EJ Sample # DS16 dark olive brown CLAY with sand Soil Description

Initial compression due to 2500 psf pressure = 5.11% Swell potential due to water and 600 psf pressure = 0.76% Total consolidation due to water and 10000 psf pressure = 11.64% Estimated swell pressure = 840 psf Initial Moisture Content 14.3 % Final Moisture Content 18.7 % Initial Dry Density 103.4 pcf Final Dry Density 113.7 pcf Initial Wet Density 118.2 pcf Final Saturated Density 135.0 pcf





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GEOTECHNICAL REPORT JUNO STONEGATE DEVELOPMENT LOTS 109, 110 & 89A MOUNTAIN VILLAGE, COLORADO SAN MIGUEL COUNTY, COLORADO

Executive Summary

On October 18, 2007, Buckhorn Geotech, Inc., carried out a feasibility level geotechnical investigation of site and subsurface conditions at the proposed building site on Lots 89A, 109, and 110 in Mountain Village, Colorado. Lots 109, 110, and 89A are situated at the north end of the Mountain Village core, at the inside bend of Mountain Village Boulevard. The proposed project area encompasses these three lots as well as the paved parking lot (*open space*) adjacent to them. The topography across the property is primarily flat with steep slopes up to Mountain Village Boulevard around the north and east edges of the property.

Design is at a preliminary sketch phase so no specific construction plans are available at the writing of this report.

The subsurface conditions we encountered consist of reddish-brown to dark brown silty to sandy clay with gravel overlying formational shale bedrock. Hard shale bedrock was encountered at depths of 23 and 19 feet in our boreholes. Groundwater was encountered at depths of 3 and 12 feet during drilling and at depths of 10.8 and 15.1 feet four weeks after drilling. Both the silty clay and jumbled shale and siltstone exhibited low to moderate swell potential.

Below is a summary of our preliminary recommendations for the proposed development of this site.

- A deep foundation consisting of driven piles is the preferred foundation system at this site.
- Alternatively, a spread footing foundation system may be considered. Potentially
 excessive settlement may preclude the use of spread footing foundations.

- Slab on-grade floors may be used but should be placed on a minimum one foot of structural fill to reduce the potential for post-construction heave.
- Foundation and under-slab drainage should be provided to aggressively evacuate groundwater from the building excavation.
- Depending on excavation depths and location, excavation stabilization may be needed for this site as spatial constraints preclude the ability to lay slopes back. Potential shoring systems include soil nails, pile and lagging walls, and tieback anchor systems.
- Additional geotechnical investigation is recommended to better characterize the subsurface conditions across the building site.
- All of the recommendations presented in the Conclusions and Recommendations
 Section of this report should be incorporated into design and construction at this
 site.

Introduction

Buckhorn Geotech, Inc. has conducted a feasibility level geotechnical evaluation of Lots 109, 110, and 89A in Mountain Village, Colorado for construction of a multi-story hotel. An investigation of subsurface and site conditions was conducted on October 18, 2007, at the proposed building site. This work was performed at the request of the architect, Boka Powell, on behalf of Juno Stonegate Development, LLC. The investigation consisted of a site inspection, drilling of two boreholes, logging and testing of materials encountered, and analysis of available data. This report presents the findings of our investigation and our preliminary geotechnical engineering recommendations for site preparation and foundation design. Additional geotechnical investigation should be conducted by Buckhorn Geotech to better characterize the site once conceptual design has progressed sufficiently.

Construction Plans

We understand that a multi-story hotel is proposed for this property. At the writing of this report, design was at a preliminary sketch phase and no construction plans were available. Nonetheless, we anticipate some below grade construction at this site (e.g., underground parking). A site plan showing the approximate building footprint was provided to us by Boka Powell.

Site Conditions

The Town of Mountain Village is situated on the north flank of Silver Mountain immediately above and to the south of the San Miguel River Valley and Town of Telluride, as shown on the attached Vicinity Map. Lots 109, 110, and 89A are situated at the north end of the Mountain Village core, on the south side of a sharp bend in Mountain Village Boulevard. The proposed project area encompasses these three lots as well as the paved parking lot adjacent to them. The parking lot is currently designated as open space and comprises approximately half of the

proposed project area. The parking area is bounded on the south by the Westermere Condominiums and to the west by the Shirana Condominiums.

Vegetation comprises mostly irrigated lawn and native grasses. The topography across the property is primarily flat with steep slopes up to Mountain Village Boulevard around the north and east edges of the property. The approximate elevation of the property is 9,525 feet. The photograph below was taken of the building site at the time of our field investigation.



Looking southeast across the site, shows the vegetative cover, the local topography, locations of our boreholes, and the conditions at the time of our site investigation. The Westermere building can be seen at the far end of the parking lot and the Shirana building is on the far right side.

We advanced two boreholes (BH#1 and BH#2) and installed two standpipe piezometers (SP#1 and SP#2) at locations requested by the architect, as indicated on the attached Site Plan and on the photograph above. The results of our field and laboratory testing are discussed in the *Subsurface Conditions* Section of this report.

Geology

The San Juan Mountains of southwestern Colorado are a region of uplifted Paleozoic and Mesozoic sedimentary formations intruded by Tertiary volcanics. In the Telluride region, uplifting that accompanied the volcanic eruptions caused warping and folding of older sedimentary bedrock. As magma rose towards the ground surface, some was injected into deeper fractures in sedimentary strata forming a network of dikes and sills. The magma was rich in mineralized fluids, forming the gold and silver veins that made the area a rich mining district. In the millions of years since the intrusives were formed, much of the overlying

sedimentary rock has been weathered and stripped away by erosion, landslides, and glaciation to create the dramatic present landscape.

Based upon the general geologic study conducted by Lincoln DeVore of the Telluride Mountain Village (*Geology and Soils of Proposed Expansion of the Telluride Ski Area, San Miguel County, Colorado*: 1979), Lots 109, 110, and 89A are mapped as Quaternary Slope Failure Complex (*Qsfc*) with Quaternary Glacial Drift and Moraine (*Q*m) on the northern portion of Lot 109. Dakota Sandstone (*Kd*) is mapped just east (uphill) and seasonally wet areas (*Sw*) are mapped to the south of the properties. The Slope Failure Complex (*Qsfc*) is an undifferentiated, extensive, ancient landslide believed to be associated with Silver Mountain Landslide, a large-scale event covering about 15 square miles, including the Mountain Village, Telluride Ski Ranches and Elk Run developments. This massive slide complex is composed of silty to clayey sand with volcanic gravels, cobbles and boulders, but can sometimes contain entrained blocks and fragments of shale and sandstone. These soils generally have low to moderate plasticity and are moderately dense to dense. The depth and composition of Slope Failure materials are highly variable, but are generally considered stable landforms for development if drainage and potential slope instability are properly accounted for during design and construction.

Quaternary Glacial Drift and Moraine (\mathcal{Q} m) occurs as a more or less continuous band bordering the northern edge and northwest corner of the Mountain Village. Glacial drift and moraine deposits were formed in the Pleistocene as the result of glacial and glaciofluvial processes. These glacial moraine deposits are highly variable in composition but generally consist of medium to coarse gravel with frequent, subrounded to angular, cobbles and boulders within a matrix of silty sand with some clay.

The Dakota Formation (*Kd*) is typically a tan, yellow, light-red to light brown sandstone with conglomeratic lenses and localized beds of coal and carboniferous shale. This unit is well-cemented and resists weathering, and is therefore a ridge and cliff-forming rock. Although originally deposited as horizontal beds of sand and mud in a beach and tidal lagoon environment, the strata were uplifted and tilted to the west during the Tertiary Period. The dip of the rock strata typically matches the grade of the ground surface on the slope of Coonskin Mountain, to the east of the property. Overlying the Dakota Formation in this region is usually the younger Mancos Shale (*Kmc*), but due to the steep slope, this shale layer has generally been removed.

Mancos Shale (*Kmc*) is a gray to black marine shale (claystone to siltstone) that is generally thinly bedded and highly fractured in mountainous areas such as this. This rock unit is highly variable in composition and can range from low to high plasticity and from soft and highly weathered to hard and unweathered. Generally, shale becomes less weathered with depth.

The surficial materials found on these lots during our field investigation are probably best represented by a thin veneer of fine-grained slope wash from the Silver Mountain landslide (*Qsfc*) overlying shallow Mancos Shale (*Kmc*). The subsurface materials encountered at this site will be further discussed in the *Subsurface Conditions* Section of this report. See the 1979 Lincoln DeVore geologic study of Mountain Village for more information about the described subsurface types.

Geologic Hazards

A variety of geological hazards can exist in western Colorado as a result of elevation, extreme, topography, soil/geologic conditions, surface and groundwater, and climatic effects. The hazards that potentially affect the proposed hotel are discussed below. Some buildings and roadways throughout the region have experienced negative impacts due to slope movement, expansive and compressible soils, and groundwater problems. Appropriate engineering techniques for design and construction relating to troublesome climate and soil conditions should be used to reduce the potential for such problems. However, because of the overall dynamic characteristics of the area, almost every site is subject to at least some degree of potential risk. These risks are explained below.

Shallow Groundwater

Groundwater was encountered at depths of 3 and 12 feet below the ground surface during drilling and between 10.8 and 15.1 feet below adjacent ground approximately four weeks after our site investigation. The shallowest groundwater was found at the southeast end of the site, slightly deepening to the northwest.

Shallow groundwater can be problematic as it weakens foundation subgrade materials, creates hydraulic pressure, and can seep into the interior of the buildings if foundation components are not properly waterproofed. Consequently, aggressive management of surface and subsurface water at this site is very important for the long-term performance of the foundation components and slope stability. A comprehensive site drainage plan, in tandem with grading and landscape plans, should be designed to intercept surface and subsurface water and remove it from the foundation area. This may include an intercept or curtain drain, a foundation drain, and/or other active surface or subsurface drainage features. Ideally, construction would occur during a dry time of the year to minimize the amount of water and saturated soils in the foundation area. Specific recommendations for grading and foundation preparation are given below in the *Conclusions and Recommendations* Section of this report.

It should be noted seasonal variations of the groundwater level are anticipated. Changes in the groundwater level are a concern at this site, and we should be contacted to prescribe appropriate recommendations if groundwater is encountered during design or development of the property.

Floodina

According to the Federal Emergency Management Agency (FEMA) Flood Insurance Rate Map Panel 287D, the lots are located in *Zone X*, areas of 500-year flood and/or areas of 100-year flood with average depths of less than 1 foot or with drainage areas less than 1 square mile. These are areas with minimal flooding potential but where elevating the ground floor 1-foot above the surrounding ground surface would be prudent to mitigate potential flood hazards.

Expansive and Compressive Soil and Rock

Soil and rock materials containing some types of clay, especially bentonite (montmorillonite), can expand in volume with water absorption and then shrink upon drying. In some areas of

Colorado these expansive soils/rock are hazardous and can cause serious damage to foundations, roadways, pavements, and embankments. The geology of swelling soils, the effects of moisture on these soils, and construction and landscaping on swelling soils are discussed in the Colorado Geological Survey publication, *A Guide to Swelling Soils for Colorado Homebuyers and Homeowners* (Special Publication #43: Noe et al., 1997). In the Telluride area, these clays are derived from such parent material as Mancos Shale, Dakota Sandstone, Morrison Formation, and volcanic material.

Compressive soils are generally soils that have been deposited rapidly, have a weak matrix containing voids, and/or are not naturally in a dense or compacted state. Compressive soils typically have a large proportion of fine-grained materials, especially silt, but they can also contain a mixture of material if deposited in a chaotic manner. For example, debris fan and landslide deposits are often laid down rapidly and comprise materials that are not sorted or reworked, leaving behind voids and a loose matrix of rocks, soil, and possibly organic debris. Clayey soils can also be compressive if they are saturated when loads are applied.

Mancos Shale and its residual soil can be very sensitive to variations in moisture, being quite strong when dry but either expansive or losing strength rapidly when wetted. Additionally, wetting and drying cycles can weaken the shale so that it becomes highly erodible. When in a dry and dense state, the shale and its residual soil can exert expansive pressures when moisture is absorbed. Conversely, when in a loose, highly fractured state, the material can consolidate when wetted under moderate loads. The expansive and compressive characteristics of the shale and soil are discussed in detail in the *Subsurface Conditions* Section of this report.

The potential hazard from expansive and compressive soil and rock is the differential movement of the subgrade under loads applied through the foundation. This hazard can be partly mitigated by control of on-site drainage so that no water is allowed to accumulate, stand, or penetrate into the soil in the vicinity of foundations and slab/pavement areas. Further mitigation can be attained through design of foundation systems that extend to firmer material or which have sufficient strength to resist differential movements. The removal of problematic soil and replacement with structural fill is another option. These methods are discussed in further detail below in the *Conclusions and Recommendations* Section. Special Publication #43 gives general explanations and illustrations of design and drainage options on swelling soils.

Seismicity

According to the *Geologic Map of the Telluride Quadrangle* (USGS Map GQ-504), the Vanadium Fault is located 4 miles west of Mountain Village. This northwest trending fault, which extends from the intersection of the Skunk Creek drainage and Highway 145 to the south side of Gray Head Mountain, is not identified in either of the Colorado Geological Survey (CGS) reports identifying geologically recent (Quaternary-aged) and potentially active faults [*Earthquake Potential in Colorado—A Preliminary Evaluation* (Bulletin #43: Kirkham and Rogers, 1981) and *Preliminary Quaternary Fault and Fold Map and Database of Colorado* (Open-file Report 98-8: Widmann et al., 1998)]. The closest mapped potentially active faults to the subject property are the San Miguel Canyon Faults (located roughly 14 miles northwest of Telluride). These faults, interpreted as active during the Quaternary, are thought to be related to salt tectonism (movement of deep-seated salt deposits). A maximum credible earthquake for this fault zone is a magnitude (M) 5.0 event. The next closest potentially active faults are the Ridgway, Busted

Boiler, and Log Hill Mesa Graben Faults north of Ridgway (roughly 16 miles north of Telluride). These faults are located at the southern end of the Uncompanded Plateau and are also interpreted to be Quaternary-aged. The maximum credible earthquake inferred for these faults is M6.0 to 6.75.

Mountain Village is located in the Western Mountain Seismotectonic Province in Colorado, where maximum credible earthquakes are estimated to be on the order of magnitude 6 to 6.5, equivalent to Modified Mercalli (MM) VI to VIII (CGS Bulletin #43). The largest recorded earthquake in the region was the 1994 M4.4 (MM VI) Norwood event [according to the CGS Bulletin #52 entitled *Colorado Earthquake Information, 1867-1996* (Kirkham and Rogers, 2000) and the CGS website database of earthquake events: http://geosurvey.state.co.us]. There were several other similar magnitude earthquakes in the Telluride region: Telluride in 1894 (MM IV), Ridgway in 1897 (MM V), Lake City in 1913 (MM VI) and 1955 (MM VI), and Cimarron Ridge/Montrose in 1960 (MM VI) and 1962 (MM V). Many other earthquake events less than MM V have been identified for the region.

The Colorado Geological Survey indicates that, based on limited historical records, Colorado is considered to be a region of minor earthquake activity, where moderate to large events are relatively infrequent. There is a growing body of evidence, however, suggesting that Colorado is at greater risk than previously thought. According to the Uniform Building Code, western Colorado is in Seismic Risk Zone 1 where distant earthquakes would be expected to cause only minor damage to structures with fundamental periods of vibration greater than one second. Except for transmission towers, we are unaware of such tall, slender structures in western Colorado. However, the CGS recommends in Bulletin #43 that a Seismic Risk Zone 2 designation may be more appropriate for all of Colorado except the extreme northeast corner. It also suggests that a minimum 0.1g horizontal acceleration be used in design and safety analyses even for areas that are distant from known active faults.

Radon Gas

Radon gas is produced by decay of radioactive minerals contained in subsurface rock and soil. The U.S. Environmental Protection Agency (EPA) has determined that radon is the second leading cause of lung cancer and that radon can accumulate in buildings if the gas is not properly removed through passive or active methods. The EPA map of Radon Zones indicates that virtually all of western Colorado, including San Miguel County, is in Zone 1 (www.epa.gov/iaq/ radon/zonemap/colorado.htm). Although there is no known safe level of radon, Zone 1 is the zone of highest risk for exposure to radon gas [i.e., greater than 4 picoCuries per Liter (pCi/L)]. The Colorado Geological Survey (CGS) participated in an EPA study in 1987 and 1988 to record indoor radon levels throughout Colorado buildings and compiled its results in a report that relates geologic setting and building construction with radon levels (CGS 1991 Open-File Report 91-4). Generally, buildings with basements had higher levels of radon than buildings built on grade on the same geologic material. In our region of Colorado, Precambrian igneous rocks had the highest readings, followed by older Mesozoic sedimentary rocks, and Tertiary volcanic and volcaniclastic rocks. Radon values in alluvial and glacial valley fill was highly variable. The CGS is careful to state that radon potential can vary considerably within the same geologic unit due to the non-uniform distribution of uranium, secondary leaching, and the accumulation of uranium and other radioactive elements into other strata.

The EPA recommends testing radon levels in existing buildings, but has not developed a sampling test that will determine levels of radon gas in the native soils prior to construction. This is due to the many factors that affect the movement of radon through soils, such as soil moisture, soil types, weather patterns, and wind. These factors cannot be completely accounted for or controlled during testing. Based on levels of radon recorded in existing buildings in the region and the presence of rock types that are known to produce radon, it is reasonable to assume that radon is present in the Telluride area. The EPA, the Colorado Department of Public Health and Environment (CDPHE) Radiation Management Division, and the National Association of Home Builders (NAHB) recommend that all new buildings constructed in Zone 1 should include radon-resistant features. These organizations also recommend that after the building is constructed, radon should be measured in the building and if the results are greater than 4 pCi/L, the system should be upgraded from passive to active (usually by installing a fan). In the EPA publication entitled, Building Radon Out: A Step-by-Step Guide on How to Build Radon-Resistant Homes (USEPA Office of Air and Radiation EPA/402-K-01-002, April 2001), three practical and inexpensive alternatives for passive, subslab depressurization systems are presented: gravel with vents, perforated pipes, or soil gas collection mats. As stated in that EPA publication, radon-reduction techniques not only reduce radon in the building but also are "consistent with state-of-the-art energy-efficient construction...which will result in energy savings and lower utility bills" and they have the added benefits of "decreasing moisture and other soil gases in the building, reducing molds, mildews, methane, pesticide gases, volatile organic compounds, and other indoor air quality problems." It is estimated that retrofitting a building after construction with radon resistant features is 2 to 10 times more expensive than if it had been included in the original construction.

The *Building Radon Out* EPA publication can be obtained from the CDPHE in Denver by calling (303) 692-3420. Other recommendations for passive and active design and construction techniques for reducing radon gas can be found on the EPA radon website *www.epa.gov/radon/* or the CDPHE radon website *www.cdphe.state.co.us/hm/rad/radon*.

No other geologic hazards are known to be present in the vicinity of the Mountain Village Hotel project.

Subsurface Conditions

Two borings (BH#1 and BH#2) were advanced to depths of 34 and 40 feet using a truck-mounted Simco 2800 HS drill rig at the locations noted on the attached Site Plan. The locations of the borings were selected by the architect prior to the investigation and indicated to us to us in the Boka Powell preliminary site plan dated June 21, 2007. The locations were adjusted in the field as conditions dictated (utility locations, access, etc.). The boreholes were drilled with a 4-inch solid stem continuous-flight auger. California split spoon samples of subsurface materials were obtained at discrete depths. Standard penetration tests (SPTs) were conducted in general accordance with ASTM Standard D-1586.

The soil, bedrock, and groundwater conditions were logged, and representative samples of subsurface materials encountered were brought back to our laboratory for detailed examination

and testing. The subsurface conditions encountered in the borings and laboratory results are shown on the attached Borehole Logs.

Generally, the subsurface conditions encountered at the site consist of dark brown to reddish-brown silty to sandy clay with varying amounts of gravel. This material is generally moist to wet, stiff, and contains some shale chips and sandstone gravel. SPT N-values ranged from 4 to 13 blows/foot in this material. At depths of approximately 23 and 29 feet, hard shale/siltstone was encountered. No samples of this material (save what could be taken from the auger bit) were recovered. Groundwater was encountered at depths of 3 (BH#1) and 12 (BH#2) feet below the ground surface during or immediately after drilling. On November 14, 2007 the recorded groundwater levels were at 10.8 and 15.1 feet (BH#1 and #2, respectively). Standpipe piezometers were installed in the two boreholes to allow for future groundwater monitoring.

Laboratory tests were conducted on soil samples obtained from the boreholes to characterize the index, behavioral, and geochemical properties. Atterberg limits, gradations, swell-consolidation, sulfates, chlorides, electro-conductivity, and pH tests were conducted. Laboratory test results are discussed herein and attached to this report.

Atterberg limits and Gradation Analyses were conducted on several samples the sandy to silty clay overburden soils. The liquid limit (LL) of the samples tested ranged from 26 to 35, with an average LL of 30, while the plasticity index (PI) ranged from 9 to 18, with an average PI of 13. A soil with a PI of less than 15 is generally considered to have a low potential for swelling when wetted and shrinking when dried, while a soil with a PI of between 15 and 30 is considered to have moderate potential for swelling or shrinking. The samples were found to be composed of approximately 8 to 20% gravel, 32 to 46% sand, and 36 to 55% fines. Based on these results, these samples classify as clayey sand with gravel (SC) and sandy lean clay (CL) according to the Unified Soil Classification System (USCS). Natural moisture contents of the soils ranged from approximately 12 to 15%.

Two samples of the sandy to silty clay overburden soils were tested for chemical properties, which yielded water soluble sulfate concentrations 0.03%, chloride contents of 10 and 35 ppm, electro-conductivities of 15 and 80 μ S/cm, and pH of 7.1.

Swell/consolidation tests were conducted on five samples of the clayey overburden soils to characterize their behavior under varying conditions of loading and moisture. The samples were initially loaded to 1,000 pounds per square foot (psf) and inundated with water. Two samples swelled slightly (0.03 and 0.06%), two samples collapsed slightly (0.08 and 0.14%), and the volume change was negligible for the fifth sample. The samples were subsequently subjected to progressively increasing pressures to a total of 3,000 psf. The initial dry densities of the samples ranged from 104 to 124 pcf. Swell pressures of 1,130 psf were estimated for samples DS7 (BH#2 at 10.5 to 11.5 feet) and DS8 (BH#2 at 15.5 to 16.5 feet). These swell pressures are considered to be low to moderate.

In summary, subsurface conditions we encountered in our boreholes consist of 23 and 29 feet of clayey sand and sandy to silty clay with jumbled shale overlying hard shale bedrock. The overburden silty clay and jumbled shale exhibited low to moderate potential for swell and a slight potential for collapse upon wetting. Groundwater was encountered between depths of 3

and 12 feet below the ground surface during drilling, and 10.8 to 15.1 feet on November 14, 2007 (approximately four weeks after drilling).

CONCLUSIONS AND RECOMMENDATIONS

Based upon our site inspection and results of the shallow soil exploration, the following feasibility-level recommendations are offered as measures to enhance the stability of the site and the long-term performance of the foundation soils. It should be noted that the mitigation measures offered address only the construction at the building site. They cannot and will not arrest or prevent large-scale geologic processes that may be on-going elsewhere on the property and within the Mountain Village area. Also, as mentioned earlier in this report, some degree of risk is inherent in all construction in mountainous areas of Colorado. The recommended measures are intended to be reasonable and prudent but cannot be considered as absolute protection against the vagaries of nature.

This report does not contain project specifications. These recommendations are provided to guide the conceptual design process. Additional geotechnical investigation is recommended for development of final design and construction specifications.

General Design Criteria/Considerations

- 1. The geotechnical investigation conducted is considered feasibility level. Additional geotechnical investigation is recommended to better characterize the subsurface conditions across the building site.
- 2. Based on the elevation of 9,525 feet, the Structural Engineers Association of Colorado recommends that the Basic Roof Snow Design Load be a minimum of 107 psf. It is recommended that the local building official be contacted to verify the required snow design load for this property.
- 3. Shallow components of the foundation system should be extended into the soil a minimum depth below finished grade as prescribed by the local building official to reduce the negative effects of frost heave.
- 4. The conceptual design should consider relatively shallow groundwater (10 to 15 feet). Below grade construction may require dewatering and waterproofing. Suitable shoring systems may be limited due to groundwater. Further discussion of groundwater management is discussed in later sections.

Seismic Design Criteria

In accordance with Section 1615 of the *2003 International Building Code* (IBC) and our knowledge of the site, this site may be designated as Site Class D. This classification is based on limited shallow exploratory data and assumes that subsurface conditions similar to those encountered during our site investigation extend to a depth of 100 feet. It is recommended

that the local building official be contacted to verify the Site Class for this property. The mapped spectral response acceleration at short periods (0.2 second, \mathbf{S}_s) is 0.468g and at one second (\mathbf{S}_1) is 0.106g. These values are taken from the USGS website, and are referenced to the National Earthquake Hazard Reduction Program (NEHRP) 1997 and 2000 maps, reproduced in the IBC. As provided in the 2003 IBC, these values are for Site Class B, and should be adjusted accordingly for the proper site class given above.

Foundation

Due to potential for excessive settlement and post-construction differential movement of the clayey overburden soils, we recommend that a deep foundation be anticipated for the conceptual design. Considering the relatively shallow groundwater, driven piles are our preferred foundation system for this site.

Driven Piles

Designed properly, driven piles will isolate the structure from subsurface and surficial soil movements. Support of the structure is provided through concrete grade beams constructed upon and spanning the piles. The primary advantage of using driven piles is that the bearing capacity of each pile is verified during the driving process. The secondary advantage of driven piles is the simplicity and speed of installation as compared with other deep foundation types. The primary disadvantage of this system is the disturbance caused by the noise and shock waves produced during installation. Preliminary design parameters and recommendations for driven pile foundation systems are outlined below.

- 1. Piles may be Grade 36 H-piles or closed-ended pipe piles, with an allowable capacity of 9,000 psi based on the cross-sectional area of the pile.
- 2. Piles should be driven to "virtual refusal" in the shale bedrock, defined as 3 or more consistent sets of a defined blow count per unit penetration. Buckhorn Geotech will define refusal criteria once the pile driving contractor has been selected and his hammer energy and stroke criteria are available for evaluation. The hammer and cushion should match the pile type to obtain the proper load capacity during driving. Appropriate recommendations on tip reinforcement will also be provided.
- 3. Piles spaced closer than 3 times the pile diagonal or diameter shall be considered to be influenced by group action and an appropriate reduction in individual pile capacity should be made. Piles driven into bedrock should be placed no closer than 1.75 times the pile's cross-sectional diagonal or 24 inches on centers (whichever is greater).
- 4. Piles broken or damaged prior to, during, or after installation should not be used.
- 5. A piling contractor with demonstrated successful experience driving similar piles with qualified personnel in similar conditions should be chosen to perform the pile installations.
- 6. A representative of Buckhorn Geotech should be present to observe and document all pile installation operations.

Other Deep Foundation Systems

Other deep foundation systems, such as micropiles or caissons, may be considered for this site. Buckhorn Geotech would be happy to provide additional design parameters for other deep foundation systems upon request.

Shallow Foundations

A shallow foundation system may also be considered, however limiting design parameters may preclude this alternative. The bearing capacity of the subsurface materials found at the proposed foundation grade is limited by excessive settlement under moderate loading. Design parameters for spread footings are presented below. These recommendations should be reevaluated once conceptual plans and anticipated loads are determined for this project.

- 1. Conceptual design of footings, bearing pads, and retaining walls to be placed on the prepared native soil should use an allowable bearing capacity (\mathbf{q}_a) of 2,000 psf.
- 2. A minimum dead load of 400 psf should be placed on the foundation to reduce the heave potential of the subgrade. Continuous footings should have a minimum width of 2 feet and isolated pad footings should have a minimum dimension of 3 feet.
- 3. After excavation to foundation depth, the exposed soil surface should be proof-compacted using vibratory or roller compaction equipment to provide a uniformly dense surface prior to placement of footing forms. If the presence of large rocks makes disturbing the native soils below footing elevation unavoidable, then the rocks should be removed and replaced with compacted structural fill. If soft or yielding soils are encountered in the trench bottoms, they should be removed until firm material is encountered and replaced with compacted structural fill. If the soft or yielding areas appear to be more extensive, we should be contacted to assess the soil conditions and prescribe remedial action.
- 4. Once the excavation is exposed, but prior to placement of any fill or footing forms, a representative of Buckhorn Geotech must be called out to verify the nature and density of the foundation excavations, to ensure that uniform soil conditions are present and to confirm that our recommendations are consistent with actual conditions. If we do not verify the soil conditions, Buckhorn Geotech cannot be held responsible for recommendations that may be inconsistent with actual conditions.
- 5. Observation and testing during construction is essential to ensure that the geotechnical recommendations are consistent with conditions and that the project is constructed in compliance with project design and specifications. Any geotechnical observations or testing will be provided at additional charge and we should be contacted at least 2 days in advance for scheduling site visits. In addition to excavation observations, we can provide observation and testing of soil density, concrete and grout, foundation forms and rebar, pile installation, steel, welds, grading features, and drain systems.
- 6. Foundation walls should be designed with sufficient strength to resist lateral earth pressures and to bridge an unsupported span of at least 10 feet. The components of the foundations should be sufficiently interconnected to ensure that they act as a unit.

This will provide resistance to the forces associated with soil movement and will provide unity to the foundation systems.

Floor Systems

Depending on the purpose, performance criteria, and floor elevations, slabs on-grade may be considered for use at this site. Special precautions, as outlined below will be needed to minimize potential damage from soil expansion. It should be noted that potential swelling pressures within the subgrade soils exceeds the dead weight of a typical floor slab. The following recommendations will minimize, but cannot eliminate, potential slab movement.

- To provide an adequate bearing surface, topsoil and organic material should be stripped. The subgrade material should be proof-compacted and soft spots removed and replaced with washed rock or structural fill. A minimum of 1-foot of over-excavation and replacement with structural fill should be used under all interior floor slabs. Additional structural fill may be placed as needed to elevate the slab area to the desired foundation grade.
- 2. To provide a capillary break, slabs on-grade should be placed on 4 inches of ¾-inch to 1½-inch washed rock on the prepared subgrade. Where moisture-sensitive interior floor finishes are applied to the slab, an unpunctured vapor barrier between the gravel and the floor slab is also recommended.
- 3. Slabs on-grade should be structurally separate from the foundation, bearing walls, and interior partitions so that the slab can "float" freely in response to soil volume changes.
- 4. Under-slab plumbing should be avoided to minimize the potential for leakage under the slab. Where necessary, under-slab plumbing should be provided with flexible couplings and should be leak-tested prior to being placed in service.
- 5. Slab sections constructed upon the native subgrade should be designed using a vertical subgrade modulus of 100 pci. A 25 pci increase in the subgrade modulus may be granted for each 6 inches of structural fill placed under the slab.
- 6. Groundwater was measured at depths of 11 and 15 feet in our boreholes in November 2007. We recommend the foundation engineer employ an under-slab drain system to minimize the possibility of water ingression during a high groundwater event. This would comprise a system of perforated collection pipes surrounded by free-draining granular fill with a hydraulic connection to either daylight or to a sump pit for pumping. Depending on slab elevation, the slab should be waterproofed and designed to resist potential hydraulic forces in the event of a malfunction of the underdrain system.

Exterior Concrete Flatwork

- 1. Flatwork may be placed on undisturbed native soil with the topsoil and organic material removed. If fill is needed, it should consist of washed rock or structural fill, placed and compacted in accordance with project specifications.
- 2. Flatwork adjacent to buildings should not be placed over loosely compacted fill. To minimize future settlement and damage to the flatwork and/or adjacent foundations, the fill should consist of approved material placed and compacted per project specifications.
- 3. Flatwork adjacent to exterior doorways should be dowelled into the foundation to prevent long-term differential movement between the flatwork and structure.
- 4. Exterior concrete flatwork should be designed and constructed so that it drains freely away from the structure. Concrete flatwork adjacent to the foundation should slope away at a grade of at least ¼-inch per foot.
- 5. All concrete used at this site in contact with native soil should comply with the recommendations in the *Concrete* Section of these recommendations.

Retaining Structures

1. Walls acting to restrain soil should be designed using the lateral earth pressures given in Table 1. These values assume a level backslope with no hydraulic pressures behind the wall, the use of native soil or structural fill, and no surcharge loads applied within the backslope zone (as defined on the attached Foundation Excavation Detail). We should be contacted to recommend lateral earth pressure values for increased backslope angles or loading within the backslope zone.

Table 1. Lateral Earth Pressures

| | | Nati | ve Soil | Structural Fill | | |
|------------|--|------|---------|-----------------|-------|--|
| Active Ea | rth Pressure | 40 | pcf* | 34 | pcf* | |
| Passive E | arth Pressure | 350 | pcf | 375 | pcf* | |
| At-Rest E | arth Pressure | 60 | pcf | 64 | pcf* | |
| Unit weig | ht of soil | 120 | pcf** | 125 | pcf** | |
| Coefficier | nt of Friction | 0.32 | *** | 0.32 | *** | |
| * po | unds per cubic foot (fluid equivalent) | | | | | |
| ** po | unds per cubic foot | | | | | |
| *** COI | ncrete on dry soil conditions | | | | | |

2. The retaining walls should have provisions for drainage so that hydrostatic pressures are not allowed to build up. This is usually accomplished by providing free-draining granular backfill between the wall and retained soil, with a collection drain provided at the bottom of this granular zone (shown in the attached Foundation Excavation Detail), and/or the use of weep holes through the face of the wall. The drain system should be continuous and have a positive outfall which releases the collected water well away from

the wall in a manner that minimizes the erosive energy of concentrated flow. The design engineer should ensure that drainage design is compatible with design assumptions.

- 3. Excavations for retaining and foundation walls should be laid back a minimum of 35° from the vertical prior to backfilling against retaining structures (see attached Foundation Excavation Detail). For safety, excavations should also be in accordance with OSHA Regulations 29 CFR 1926. Consequently, gentler excavation faces may be required.
- 4. Fill material placed behind the walls should consist of free-draining granular material (specified below) compacted as per the design engineer's specifications. Native soil should not be used as backfill due to the fines content and their potentially expansive qualities. Compaction of 85 to 90% of Standard Proctor maximum dry density is typically used to minimize post-construction settlement of the backfill. Over-compaction of the backfill should be avoided so that excessive pressures are not placed against the retaining wall. Unless expressly approved by the design engineer, only hand-operated light-duty compaction equipment should be used within three feet of the wall. The upper one foot of backfill should consist of clayey soil to create a barrier against infiltration of surface runoff.

Concrete

Water-soluble sulfate tests conducted on samples of the clayey overburden soils encountered in our boreholes indicated sulfate concentrations of 0.03%, considered by the American Concrete Institute (ACI) to be a "negligible" sulfate exposure. We recommend Type I/II cement be used in all concrete at this site.

Foundation Drainage and Ventilation

It is important to prevent moisture from penetrating into the soil beneath or adjacent to the structure. Moisture can accumulate as a result of poor surface drainage, over-irrigation of landscaped areas, waterline leaks, melting snow, subsurface seepage, or condensation from vapor transport.

- 1. Provisions should be made to evacuate subsurface moisture accumulation from around foundations and under slabs. This may be accomplished using conventional footing drains in tandem with a positively-vented moisture and radon control system. Positive drainage to daylight or to sump pits with provision for pumping is required for foundation drains.
- 2. Depending on the proposed excavation depths, an aggressive dewatering system may be required both during and after construction.
- 3. All foundation drains should be integrated into the site drainage plan as discussed below for final disposal from the building site. In no case should surface or roof drainage be introduced into the foundation drain system.

4. Floor systems and confined areas above concrete floor slabs should be properly ventilated to allow for the release of radon gas. See the *Radon Gas* Section of this report for more radon information.

Site Preparation and Grading

- 1. The site drainage plan, in tandem with the landscape and grading plans, should ensure that the construction does not impede natural drainage patterns. Surface water should be removed and not allowed to accumulate or stand anywhere near the building foundation either during or after completion of construction. This includes water from landscaped areas, patios, decks, and roofs. Drainage plans should ensure that precipitation, snowmelt, and runoff are conveyed around and away from the building as well as the driveway. This runoff should be dispersed (not concentrated) in a manner consistent with the natural, pre-construction drainage pattern.
- 2. Final grading around the perimeter of the foundation should slope downward with at least one foot of drop within the first 10 feet of horizontal distance. Concrete flatwork adjacent to the foundation should slope away at a grade of at least ¼-inch per foot.
- 3. Development should utilize "best practices" for design and construction so that on-site erosion is minimized. This may include selective thinning of vegetation, construction of temporary diversion ditches, silt fencing, and/or dust suppression. If the cumulative area of disturbance equals or exceeds one acre, on-site erosion control should be planned and executed in conformance with Colorado Department of Public Health and Environment (Water Quality Control Division) stormwater discharge regulations. The local building official will be able to provide specific details regarding these requirements.
- 4. Grading of all permanent cut and fill slopes should not exceed 2H:1V. All slopes greater than 2H:1V and over 3 feet in vertical height should be restrained by an engineered retaining structure/system.
- 5. Backfill placed in utility trenches leading to the building should be compacted in accordance with project specifications. This will inhibit surface water infiltration and migration towards the foundation, as well as minimize post-construction settlement of the trench backfill. Check dams or other measures should be taken to minimizing conveyance of water through permeable backfill materials placed in the utility trenches.
- 6. Disturbed areas should be revegetated as soon as practical to reduce soil erosion.
- 7. Fill used at this site should meet the gradational and compaction requirements listed in Tables 2 and 3 below. Fill should be placed and compacted in **maximum 6-inch lifts**, unless otherwise directed by the design engineer. Structural fill should not be placed on frozen or wet native soil. It is recommended that the foundation excavation be open a minimum period of time to avoid degradation of the foundation soils.

Table 2. Gradation Requirements for Fill Material

| Sieve | %Passing, by weight |
|-----------------|---|
| | |
| 3/4" (19.0 mm) | 100 |
| #4 (4.75 mm) | 30-65 |
| #8 (2.36 mm) | 25-55 |
| #200 (0.075 mm) | 3-12 |
| 2.5" (63.5 mm) | 100 |
| • | 95-100 |
| ` ' | 30-65 |
| #200 (0.075 mm) | 3-15 |
| 3" (75 mm) | 100 |
| #200 (0.075 mm) | 0-5 |
| 3" (75 mm) | 100 |
| ` | 20-90 |
| #4 (4.75 mm) | 0-20 |
| #200 (0.075 mm) | 0-3 |
| | #4 (4.75 mm) #8 (2.36 mm) #200 (0.075 mm) 2.5" (63.5 mm) 2" (50 mm) #4 (4.75 mm) #200 (0.075 mm) 3" (75 mm) #200 (0.075 mm) 3" (75 mm) 4" (19 mm) #4 (4.75 mm) |

Note: The Plasticity Index for all fill soils should be less than 6.

Table 3. Compaction Requirements for Fill Material

| Application | Compaction Requirement | Proctor | Moisture |
|--------------------------|-----------------------------|----------|--------------------|
| Under footings and slabs | 95% max. dry density | Modified | ±2% of optimum |
| Under exterior flatwork | 90% max. dry density | Modified | ±2% of optimum |
| Road Subgrade | 95% max. dry density | Standard | 0-4% above optimum |
| Road Subbase | 95% max. dry density | Modified | ±2% of optimum |
| Road base course | 95% max. dry density | Modified | ±2% of optimum |
| Behind retaining walls | Per project specifications* | | |
| Utility Trenches | Per project specifications* | | |
| General landscaping | Per project specifications* | | |

^{*}As specified by the design engineer on project documents or in accordance with local municipal requirements.

- 8. Any soils containing organics, debris, topsoil, frozen soil, snow, ice, and other deleterious materials shall not be used for anything other than landscaping unless authorized by the foundation engineer.
- 9. A representative of Buckhorn Geotech should be called out to the site to observe placement of structural fill and verify the compacted density. The owner should contact

Buckhorn Geotech in advance of the excavations to discuss the specific testing requirements, budget, and scheduling needed for these services.

Excavation and Shoring

Although we are not aware of the excavation depths proposed for this project, we anticipate some construction below existing grade. The resulting excavation may require temporary support during construction as spatial site constraints may not allow laying back the excavation to the extent needed to create stable slopes. Failure to provide excavation support could endanger construction personnel and could undermine Mountain Village Boulevard, including any utilities buried under the road. There are numerous methods of providing support for the excavation walls. Below some systems are described in brief for planning purposes.

Soil nails may be used efficiently for steep or vertical cuts, provided that they can effectively resist the traffic and structural loads. Soil nails are small-diameter steel bars that are horizontally drilled and grouted into the wall face as excavation progresses downward. As each lift (typically five feet) is excavated in a hillside, the bars are installed, their end bolts are tied together with heavy gauge wire mesh and/or reinforcing steel, and the face is shotcreted. Thus, it is a top-down construction method that allows work to begin immediately at the floor of the excavation once completed. The walls can be designed for temporary use or can be designed to form the first stage of a completed basement wall. In other words, the soil nails can be used to permanently retain the soils around the structure so that the lateral forces against the walls are reduced.

Another option is the use of soldier beams and/or sheet piling that can be driven or drilled into the soil prior to excavation and used to provide cantilevered support for the retained soil in smaller cuts. Larger cuts may be made using this system together with post-tensioned soil anchors installed in a similar manner to the soil nails. The height of the retained cut will dictate the size of piles and the depth to which they are installed. We can assist with this decision once we are provided details regarding building design.

- 1. Temporary excavations should be in accordance with Occupational Safety and Health Administration (OSHA) regulations and with worker safety in mind.
- 2. Construction equipment, materials, and soil stockpiles should be located a minimum horizontal distance equal to the height of the excavation from the crest of the excavation unless otherwise approved by the design engineer.
- 3. Based upon our investigation, the clayey overburden soils encountered to depths of 23 to 29 feet would be most nearly represented by an OSHA Type B soil. Our assessment is based upon the soil and groundwater conditions encountered in limited investigation and sampling. The contractor's "competent person" (defined by OSHA as "an individual capable of identifying existing and predictable hazards...and who has the authorization to take prompt corrective measures to eliminate or control these hazards and conditions) should evaluate the soil materials exposed during excavation based on composition, structure, and environmental conditions per 29 CFR 1926 and recommend appropriate slope laybacks or shoring, as required. Refer to OSHA's Technical Manual

Section V: Chapter 2 on *Excavations: Hazard Recognition in Trenching and Shoring* (available on-line at: *www.osha.gov*) for further excavation guidelines. We are happy to provide these services, as requested.

- 4. If the excavations will be made or remain open during wet weather, it is recommended that polyethylene sheeting be secured over the excavation face to minimize sediment runoff and deterioration of the foundation soils. Surface runoff above the cuts should be directed away from the excavation using berms or diversion ditches. Large rocks exposed in the excavation face should be scaled for worker safety.
- 5. Excavation dewatering may be required if the excavation is made during peak groundwater season (springtime). Dewatering may be accomplished with conventional sump pumping located below the floor of the excavation. The excavation should be designed to accommodate clarification and discharge of this intercepted water. The Town of Mountain Village Building Department should be contacted for specific regulations regarding the discharge of groundwater off-site.
- 6. Excavations should be performed during the low groundwater season (late summer through early spring) to minimize the amount of water that needs to be removed during shoring and construction operations. This will also minimize pumping of the soil so that maximum densities can be achieved.
- 7. We anticipate that the excavation of the site soils can be accomplished by conventional excavating equipment.

Closing Considerations

Standard of Care and Interpretation of Subsurface Data

This report has been prepared in a manner consistent with local standards of professional geotechnical engineering practice. Investigation for environmental contaminants was not part of our scope of services performed at this site. The classification of soils and interpretation of subsurface conditions is based on our training and years of experience, but is necessarily based on limited subsurface observation and testing. As such, inferred ground conditions cannot be guaranteed to be exact. No other warranty, express or implied, is made.

Inspection of the excavation(s) subgrade by Buckhorn Geotech prior to erection of the foundation system is an integral part of these recommendations. If subsurface conditions differing from those described herein are discovered during excavation, construction should be stopped until the situation has been assessed by a representative of Buckhorn Geotech. Construction should be resumed only when remedies or design adjustments, as necessary, have been prescribed.

Use of This Report

This report is intended for use by Juno Stonegate Development, LLC specifically to address the site and subsurface conditions as they relate to the proposed structure(s) described in the

Construction Plans Section. Changes to the site or proposed development plans may alter or invalidate the conclusions and recommendations contained herein.

Buckhorn Geotech retains an ownership and property interest in this report. Consistent with the industry, copies of this document that may be relied upon by Juno Stonegate Development, LLC are limited to printed copies (also known as *hard copies*) that are signed and sealed by the Geotechnical Engineer (*Standard Form of Agreement Between Owner and Geotechnical Engineer for Professional Services*, Engineer's Joint Contract Documents Committee, 1996). Any conclusions or information obtained or derived from electronic files, data, or graphics will be at the user's sole risk. This report together with ancillary data, analyses, test results, and other components and/or supporting parts are not intended or represented to be suitable for reuse by Juno Stonegate Development, LLC or others on extensions to this project or on any other project. Any such reuse or modification invalidates all aspects of the report and excuses the Geotechnical Engineer for all responsibility and liability or legal exposure.

This report is considered valid for a period of two years from the date of issue provided the site conditions and development plans have not changed from what is referenced in this report. Changes to the site may occur due to development or natural processes. Additionally, technological advances made in construction and changes in legislation may alter the recommendations made herein. Depending upon the site and proposed development changes, Buckhorn Geotech may require additional investigation (at additional cost) to update the recommendations contained herein.

Retention of Samples

Samples of soil and rock collected during the course of our geotechnical investigation(s) are routinely held in our laboratory for a period of three months from the date of the investigation, then discarded. A written request by Juno Stonegate Development, LLC is required for samples to be stored for a longer period.

Additional Services

To provide continuity and consistency from project start to finish, we should be retained to make observations and carry out material testing as a service to the owner. As noted above, we recommend the owner contact us to discuss required services and scheduling in advance of the construction phase.

Buckhorn Geotech is a full-service engineering firm providing foundation, on-site wastewater system, site drainage, structural, and retaining structure design services, as well as surveying, construction materials testing, and inspections. Please visit **www.buckhorngeo.com** for a full description of our services.

Thank you for the opportunity to perform this soil investigation for you. If you require any of these services or have any questions regarding this report, please do not hesitate to contact us.

Respectfully Submitted, November 20, 2007

ELECTRONICALLY SUBMITTED

Brett R. Byler, P.E. Geotechnical Engineer

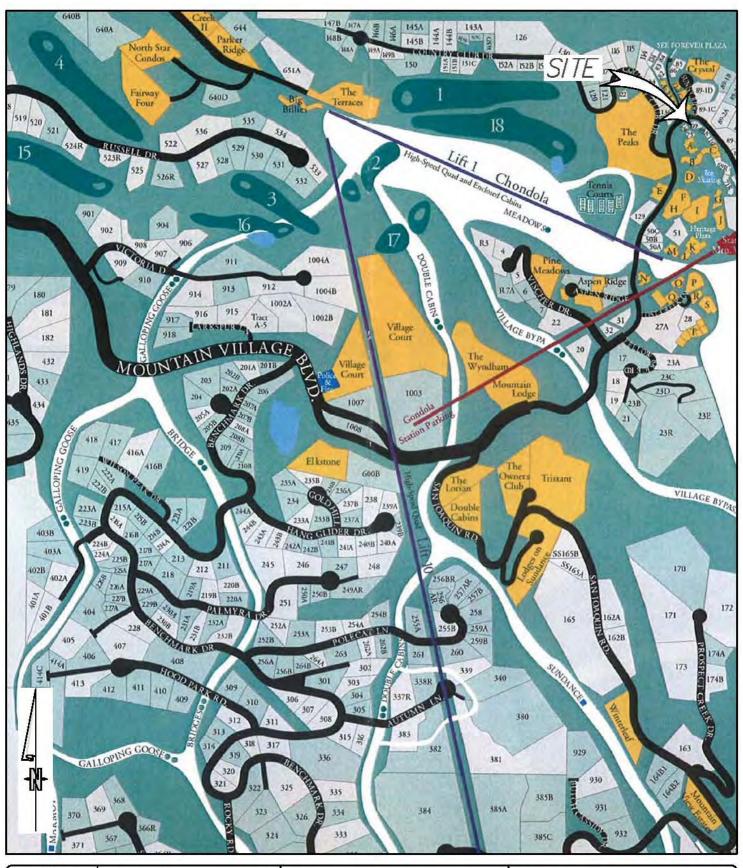
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Enclosures: Vicinity Map, Site Plan, Borehole Logs, Sieve Analysis and Atterberg Limits

results, Swell/Consolidation curves, Corrosivity Series results, Retaining Wall

Schematic

VICINITY MAP



 MAP NUMBER
 INVESTIGATION
 CD

 1
 DRAFTING
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 10/16/07

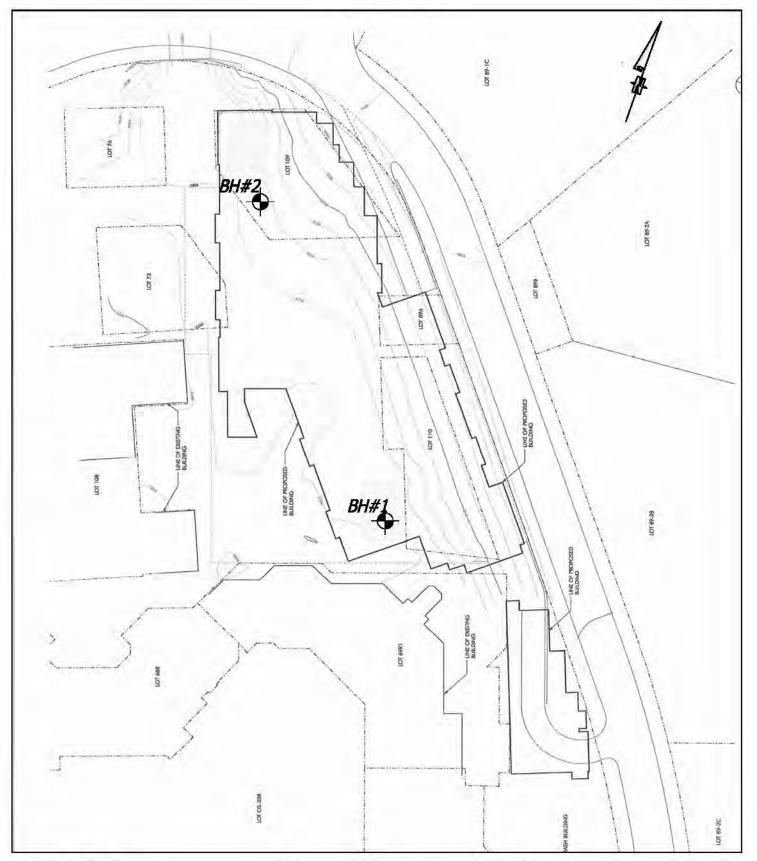
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 JOB NO.
 07-383-GEO

LOTS 109, 110 & 89A JUNO STONEGATE DEVELOPMENT MOUNTAIN VILLAGE, COLORADO

BUCKHORN DEOTECH

CMI, Structural, and Geotechnical Engineers, inc. 222 South Park Avenue Montrose, Colorado 81401 Phone (970) 249-8828 Fax (970) 249-0945

SITE PLAN



| MAP NUMBER | INVESTIGATION | CD |
|---------------|---------------|------------|
| NUMBER | DRAFTING | cc |
| 2 | DATE | 10/16/07 |
| OF 2 | JOB NO. | 07-383-GEO |

LOTS 109, 110 & 89A

JUNO STONEGATE DEVELOPMENT

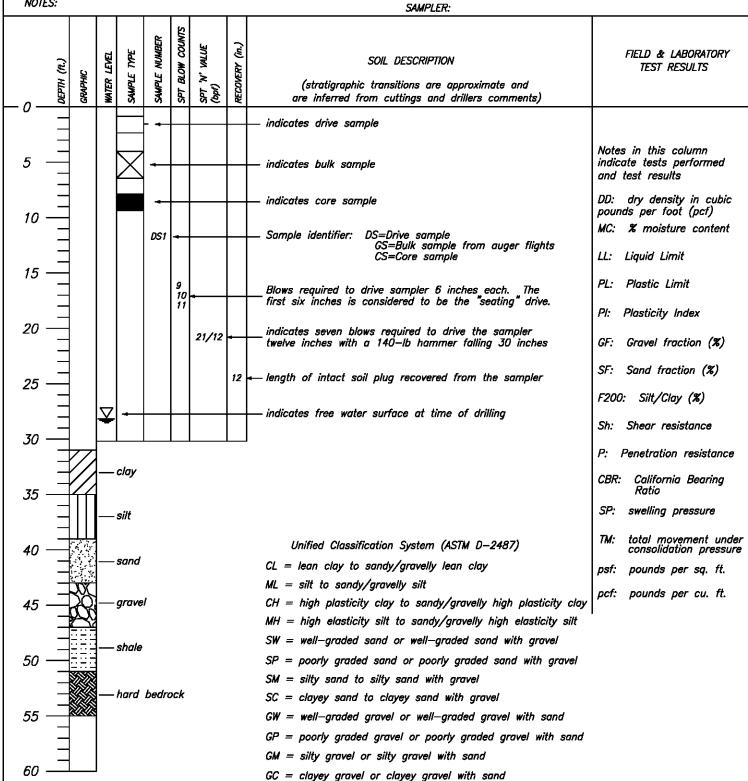
MOUNTAIN VILLAGE, COLORADO

BUCKHORN **GEOTECH**

Civil, Structural, and Geolechnical Engineers, Inc. 222 South Park Avenus Montrose, Colorado 31401 Phone (970) 249-6828 Fax (970) 249-0945

BOREHOLE LOG KEY

BOREHOLE LOCATION: SURFACE ELEVATION: NOTES: DRILLER: DRILL RIG: DRILL STEM:



| BOREHOLE | INVESTIGATION |
|----------|---------------|
| LOG KEY | DRAFTING |
| 1 | FIELD DATE |
| of 1 | JOB NO. |

BOREHOLE LOG KEY

* SPT N-values not corrected for energy or depth

BUCKHORN GEOTECH

Civil, Structural, and Geotechnical Engineers, Inc. 222 South Park Avenue Montrose, Colorado 81401 Phone (970) 249-8828 Fax (970) 249-0945

BOREHOLE LOG - BOREHOLE #1 (BH#1)

BOREHOLE LOCATION: Southeast corner of parking lot, just off asphalt

APPROX LAT/LON. (GPS): 37.93881°N 107.84601°W +/- 30'

NOTES: 5' from sidewalk, 22' from lightpole

DRILLER: S. McCracken

DRILL RIG: Simco 2800 HS

DRILL STEM: 4" Solid-stem continuous flight auger

SAMPLER: 2" I.D. California split spoon

| minoramora en la companya de la comp | | 50.30Y (20.50Y) | (Control of Control of | 99012000 | 20.000000000000000000000000000000000000 | | SAMPLER: 2" I.D. California split spoon | |
|--|--|-----------------|--|-------------------------|---|--------------------|--|---|
| О− | GRAPHIC | WATER LEVEL | SAMPLE TYPE | SAMPLENUMBER | SPT BLOW COUNTS | *SPT W VALUE (bpt) | SUBSURFACE DESCRIPTION | FIELD & LABORATORY TEST RESULTS |
| - - - | | | | | | | reddish brown to brown, moist, soft to firm, gravelly SILT and CLAY, some sand, sandstone gravel, iron oxide staining (0–5') | |
| 5 — | | 1000 | CA | -DS1 | 3 2 2 | 4 | drive sample DS1 © 5.5-6.5° | DS1 LL=29 PL=17 PI=12 MC=15.1% TM=0.9% @3000psf +H ₂ 0 |
| | P d | | | | | | drive sample DS2 © 10.5–11.5' | DD=112 pcf |
| 10 — | | V | CA | - <i>0</i> 52 | 4 4 4 | 8 | dark brown to black, wet (free water), firm to stiff, gravelly CLAY (weathered SHALE) and SAND, some silt, iron oxide staining | DS2 (SC) LL=26 PL=17 Pl=9 GF=18% SF=46% F200=36% |
| 15 — | | | CA | -DS3 | 5 6 7 | 13 | drive sample DS3 © 15.5–16.5' black, wet, stiff, silty, weathered, jumbled, shale CLAY and sandstone GRAVEL/COBBLES (angular), iron oxide staining | MC=19.6% 0.14% collapse @1000psf+H ₂ 0 TM=1.9% @3000psf +H ₂ 0 DD=108 pcf Sulfates=0.03% Chlorides=10 ppm EC=15 µS/cm |
| 20 — | | | CA | -DS4 | 6 6 | 12 | drive sample DS4 © 21-21.5' black, wet, stiff, silty, weathered, jumbled, CLAY and sandstone GRAVEL/GOBBLES (angular), iron oxide staining | pH=7.10 DS3 0.08% collapse @1000psf+H ₂ 0 TM=0.9% @3000psf +H ₂ 0 DD=124 pcf MC=12.1% |
| 25 — — | | | | | | | drilling very "stiff" 9 23' and below | MU=12.1% |
| - | ine: | | | | | | | |
| 30 — | | | | -SPT Ø 30' | 50/ 0 | >100 | black, wet SHALE cuttings on bit | |
| 35 — | | | | | | | installed standpipe "SP#1" to approximately 39'; slotted lower 30', concealed with flush—mount manhole cover; groundwater @3' immediately after drilling; groundwater @10.8' on 11/14/07 | |
| - | | | | SPT | | | | |
| 40 — | | | | | | >100 | shale in shoe, gray, hard, dry to damp SHALE [jumbled] | |
| | <u> Heer</u> | | | -40.2° | 2" | | end of borehole @4 0' in SHALE bedrock | |
| | * SPT N-values not corrected for energy or depth; stratigraphic transitions are approximate and are inferred from cuttings & drillers comments | | | | | | | |

| BOREHOLE | INVESTIGATION | CD |
|----------|---------------|------------|
| LOG | DRAFTING | сс |
| 1 | DATE | 10/18/07 |
| OF 2 | JOB NO. | 07-383-GEO |

LOTS 109, 110, & 89A

JUNO STONEGATE DEVELOPMENT

MOUNTAIN VILLAGE, COLORADO

BUCKHORNGEOTECH

Civil, Structural, and Geotechnical Engineers, inc. 222 South Park Avenue Montrose, Colorado 81401 Phone (970) 249-8828 Fax (970) 249-0945

BOREHOLE LOG - BOREHOLE #2 (BH#2)

BOREHOLE LOCATION: West side of entrance to parking lot

APPROX LAT/LON. (GPS): 37.93908°N 107.84640°W +/- 30'

NOTES: In line with lower break in grade in lot entrance ramp; 8' west of "no parking" sign

DRILLER: S. McCracken

DRILL RIG: Simco 2800 HS

DRILL STEM: 4" Solid-stem continuous flight auger

SAMPLER: 2" I.D. California split spoon

| \neg | T | _ | _ | | SAMPLER: 2" I.D. California split spoon | * |
|--|-------------|--------------|------------------------------------|---|--|---|
| WATERLEVEL | SAMPLE TYPE | SAMPLENUMBER | SPT BLOW COUNTS | *SPT N° VALUE (bpf) | SUBSURFACE DESCRIPTION | FIELD & LABORATORY TEST RESULTS |
| INMERSE DE LA CONTRACTOR DE LA CONTRACTO | CA CA CA | -DS8 | 1266 656 356 855 1" 40/ | 12 | dark brown, moist, soft to stiff, sandy SILT and CLAY, little gravel, some shale chips, iron oxide staining (0–5') drive sample DS6 66–6.5'; large chunk of wood in sampler drive sample DS7 610–11.5' reddish brown, moist to wet, stiff, sandy, silty CLAY and SHALE chips, iron oxide staining, some sandstone gravel and cobbles (angular) drive sample DS8 615.5–16.5' dark brown—red—black, wet, clayey SHALE (jumbled), highly weathered; wet sand, gravel, and cobbles in shoe; iron oxide staining drive sample DS9 620.5–21.5' black, stiff, wet, clayey SAND and GRAVEL, some cobbles, gravel and cobbles are hard angular sandstone and shale with iron oxide staining drilling stiffened up significantly 629' wet SHALE in shoe near refusal 6 31.5'; | DS6 (CL) LL=35 PL=17 Pl=18 GF=8% SF=37% F200=55% MC=16.3% DS7 (SC) LL=29 PL=17 Pl=12 GF=20% SF=32% F200=48% MC=17.5% 0.04% swell Φ1000psf +H ₂ 0 SP=1130psf Φ1000psf +H ₂ 0 DD=109 pcf Sulfates=0.03% Chlorides=35 ppm EC=80 μS/cm pH=7.05 DS8 0.06% swell Φ1000psf +H ₂ 0 SP=1130 Φ1000psf +H ₂ 0 TM=1.0% Φ3000psf +H ₂ 0 DD=104 pcf MC=20.1% |
| | | | | | installed standpipe SP#2 to approximately 34' slotted lower 25'; concealed with flush-mount manhole cover; groundwater @ 12' immediately after drilling; groundwater @15.1' on 11/14/07 | |
| | MATERIENE. | CA CA CA | CA -058 CA -058 CA -059 CA -059 | CA -058 6 6 CA -058 5 6 CA -058 5 5 CA -058 | CA -056 6 12 CA -057 6 11 CA -058 5 11 CA -058 5 10 SPT 50/>100 30' SPT 40/ | dark brown, moist, soft to stiff, sandy SILT and CLAY, little gravel, some shale chips, iron oxide staining (0–5') A DS8 6 12 drive sample DS6 06-6.5'; large chunk of wood in sampler drive sample DS7 010-11.5' reddish brown, moist to wet, stiff, sandy, silty CLAY and SHALE chips, iron oxide staining, some sandstone gravel and cobbles (angular) drive sample DS8 015.5-18.5' dark brown-red-block, wet, clayey SHALE (jumbled), highly weathered, wet sand, gravel, and cobbles in shoe; iron oxide staining CA DS9 5 10 gravel and cobbles are hard angular sandstone and shale with iron oxide staining drilling stiffened up significantly 029' wet SHALE in shoe near refusal 0 31.5'; spor 1 10 2100 and GRAVEL, some cobbles, gravel and cobbles are hard angular sandstone and shale with iron oxide staining drilling stiffened up significantly 029' wet SHALE in shoe near refusal 0 31.5'; spor 1 10 2100 and GRAVEL, some cobbles, gravel and cobbles are hard angular sandstone and shale with iron oxide staining |

| BOREHOLE | INVESTIGATION | CD |
|----------|---------------|------------|
| LOG | DRAFTING | СС |
| 2 | DATE | 10/18/07 |
| 0F 2 | JOB NO. | 07-383-GEO |

LOTS 109, 110, & 89A JUNO STONEGATE DEVELOPMENT MOUNTAIN VILLAGE, COLORADO

BUCKHORNGEOTECH

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Sieve Analysis and Atterberg Limits

Project Name Lots 89A, 109, and 110 TMV 11/5/2007 Date Project Location Lots 89A, 109, and 110 TMV 07-383-GEO Project # Client Juno Stonegate Development Sample by BB BH#1 @5.5-6.5' Tested by **Test Location** DJ/CC Sample # DS1

Sieve Analysis

ASTM C136 / C117

| Sieve | Opening (mm) | % Passing |
|-------|--------------|-----------|
| 3" | 76.2 | NR |
| 3/4" | 19.0 | NR |
| 3/8" | 9.5 | NR |
| #4 | 4.750 | NR |
| #10 | 2.000 | NR |
| #40 | 0.425 | NR |
| #200 | 0.075 | NR |

NR = Not Requested

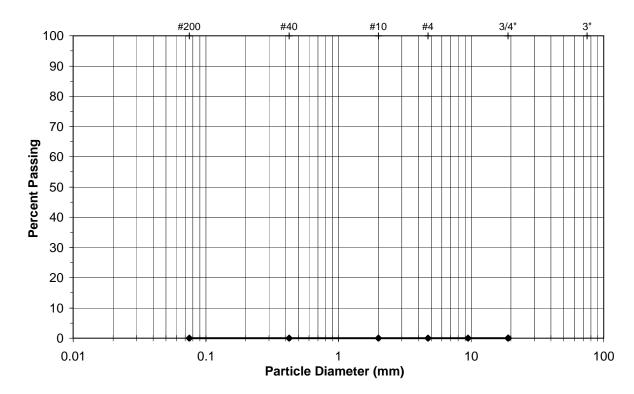
Atterberg Limits

ASTM D4318

| Liquid Limit (LL) | 29 |
|-----------------------|----|
| Plastic Limit (PL) | 17 |
| Plasticity Index (PI) | 12 |
| | , |

Natural Moisture Content (%) = 15.1%

Soil Description dark brown clayey SAND with gravel



| Clay/Silt | Fine | Medium | Coarse | Fine | Coarse |
|-----------|------|--------|--------|------|--------|
| FINES | | GRA | VEL | | |

% Fines = NR

% Sand = NR

% Gravel = NR



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Sieve Analysis and Atterberg Limits

Project NameLots 89A, 109, and 110 TMVDate11/1/2007Project LocationLots 89A, 109, and 110 TMVProject #07-383-GEOClientJuno Stonegate DevelopmentSample by
Test LocationBBTest LocationBH#1 @10.5-11.5'Tested byDM/CC

Sample # DS2

Sieve Analysis

ASTM C136 / C117

| Sieve | Opening (mm) | % Passing |
|-------|--------------|-----------|
| 3" | 76.2 | 100.0 |
| 3/4" | 19.0 | 100.0 |
| 3/8" | 9.5 | 89.9 |
| #4 | 4.750 | 81.9 |
| #10 | 2.000 | 67.7 |
| #40 | 0.425 | 52.8 |
| #200 | 0.075 | 35.8 |

Atterberg Limits

ASTM D4318

| Liquid Limit (LL) | 26 |
|-----------------------|----|
| Plastic Limit (PL) | 17 |
| Plasticity Index (PI) | 9 |
| | |

Natural Moisture Content (%) = 19.6%

Soil Description very dark gray clayey SAND with gravel **USCS** Classification SC #200 #40 #10 3/4" 100 90 80 70 Percent Passing 60 50 40 30 20 10

| Clay/Silt | Fine | Medium | Coarse | Fine | Coarse |
|-----------|------|--------|--------|------|--------|
| FINES | | SAND | | GRA | VEL |

Particle Diameter (mm)

% Fines = 35.8

0.1

0.01

% Sand = 46.1

% Gravel = 18.1

100

10



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Sieve Analysis and Atterberg Limits

Project Name Lots 89A, 109, and 110 TMV Date 11/13/2007 Project Location Lots 89A, 109, and 110 TMV 07-383-GEO Project # Juno Stonegate Development Client Sample by BB **Test Location** BH#2 @6-6.5' Tested by DJ/VB Sample # DS6

Sieve Analysis

ASTM C136 / C117

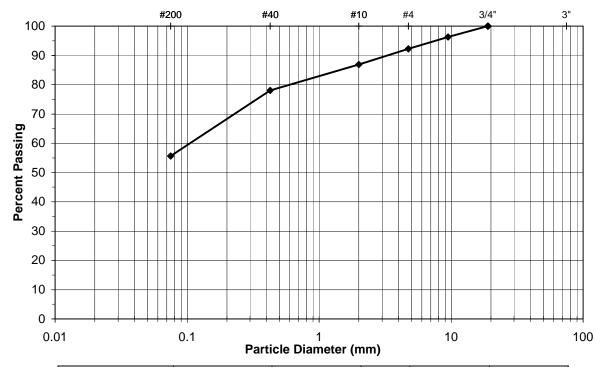
| Sieve | Opening (mm) | % Passing |
|-------|--------------|-----------|
| 3" | 76.2 | 100.0 |
| 3/4" | 19.0 | 100.0 |
| 3/8" | 9.5 | 96.3 |
| #4 | 4.750 | 92.2 |
| #10 | 2.000 | 86.9 |
| #40 | 0.425 | 78.1 |
| #200 | 0.075 | 55.6 |

Atterberg Limits ASTM D4318

| Liquid Limit (LL) | 35 |
|-----------------------|----|
| Plastic Limit (PL) | 17 |
| Plasticity Index (PI) | 18 |
| | |

Natural Moisture Content (%) = 16.3%

Soil Description dark brown sandy lean CLAY **USCS** Classification CL



| Clay/Silt | Fine | Medium | Coarse | Fine | Coarse |
|-----------|------|--------|--------|------|--------|
| FINES | | SAND | | GRA | VEL |

% Fines = ___55.6__

% Sand = 36.6

% Gravel = 7.8



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Sieve Analysis and Atterberg Limits

Project Name Lots 89A, 109, and 110 TMV 11/6/2007 Date Project Location Lots 89A, 109, and 110 TMV 07-383-GEO Project # Client Juno Stonegate Development Sample by BB BH#2 @10-11.5' Tested by DM/CC **Test Location** Sample # DS7

Sieve Analysis

ASTM C136 / C117

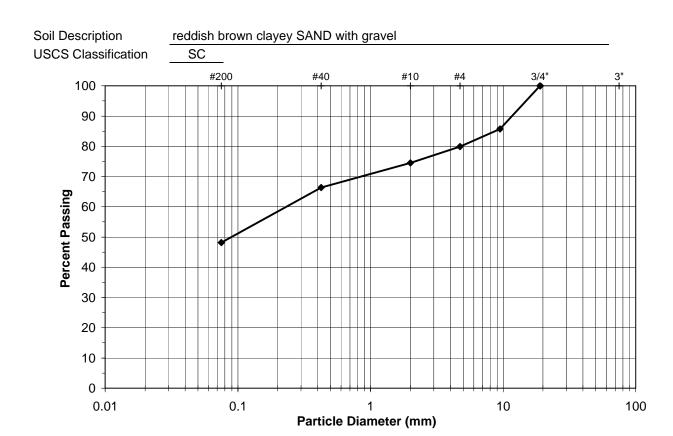
| Sieve | Opening (mm) | % Passing |
|-------|--------------|-----------|
| 3" | 76.2 | 100.0 |
| 3/4" | 19.0 | 100.0 |
| 3/8" | 9.5 | 85.7 |
| #4 | 4.750 | 79.9 |
| #10 | 2.000 | 74.5 |
| #40 | 0.425 | 66.4 |
| #200 | 0.075 | 48.2 |

Atterberg Limits

ASTM D4318

| Liquid Limit (LL) | 29 |
|-----------------------|----|
| Plastic Limit (PL) | 17 |
| Plasticity Index (PI) | 12 |
| | |

Natural Moisture Content (%) = 17.5%



| Clay/Silt | Fine | Medium | Coarse | Fine | Coarse |
|-----------|------|--------|--------|------|--------|
| FINES | | SAND | | GRA | VEL |

% Fines = 48.2

% Sand = 31.7

% Gravel = 20.1



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Swell/Consolidation Test

ASTM D4546

 Project Name
 Lots 89A, 109, and 110 TMV
 Date
 10/30/07

 Project Location
 Lots 89A, 109, and 110 TMV
 Project #
 07-383-GEO

 Client
 Juno Stonegate Developmer
 Sampled by
 BB

 Sample Location
 BH#1 @5.5-6.5'
 Tested by
 DJ

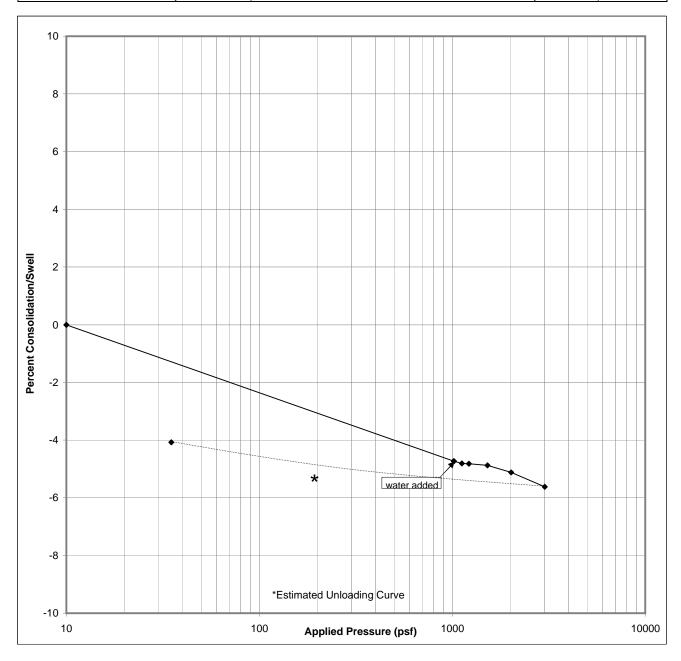
 Sample #
 DS1

 Soil Description
 dark brown clayey SAND with gravel
 DA

Initial compression due to 1000 psf pressure = 4.73%

Total consolidation due to water and 3000 psf pressure = 0.89%

Initial Moisture Content 16.1 % Final Moisture Content 17.7 % Initial Dry Density 112.2 pcf Final Dry Density 114.4 pcf Initial Wet Density 130.2 pcf Final Saturated Density 134.7 pcf





Soil Description dark grayish brown lean CLAY (shale)

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Swell/Consolidation Test

ASTM D4546

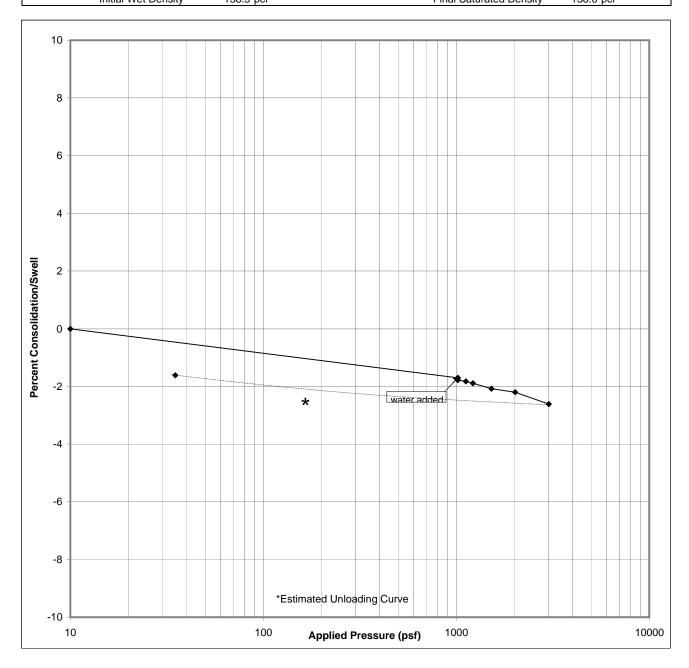
 Project Name
 Lots 89A, 109, and 110 TMV
 Date
 11/09/07

 Project Location
 Lots 89A, 109, and 110 TMV
 Project #
 07-383-GEO

 Client
 Juno Stonegate Developmer
 Sampled by
 BB

 Sample Location
 BH#1 @15.5-16.5'
 Tested by
 DJ

 Sample #
 DS3





Soil Description very dark gray clayey SAND with gravel

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Swell/Consolidation Test

ASTM D4546

 Project Name
 Lots 89A, 109, and 110 TMV
 Date
 10/30/07

 Project Location
 Lots 89A, 109, and 110 TMV
 Project #
 07-383-GEO

 Client
 Juno Stonegate Developmer
 Sampled by
 BB

 Sample Location
 BH#1 @10.5-11.5'
 Tested by
 DJ

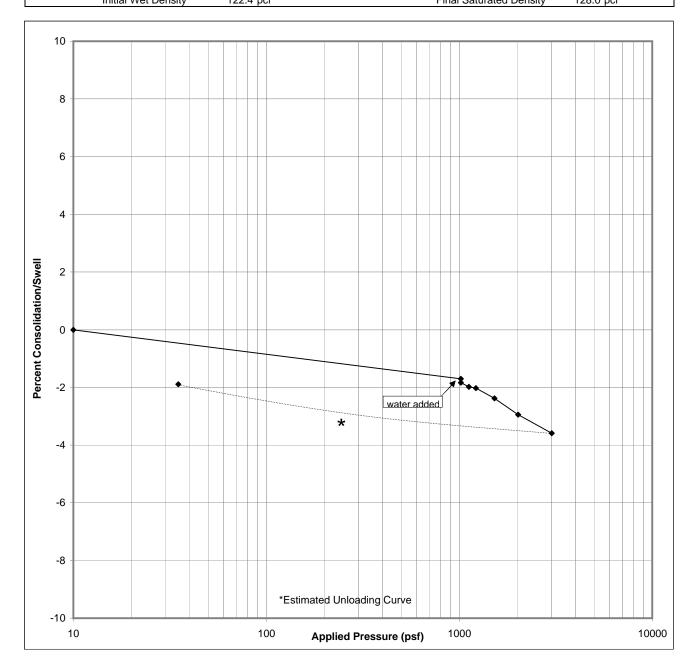
 Sample #
 DS2

Initial compression due to 1000 psf pressure = 1.7%

Collapse potential due to water and 1000 psf pressure = 0.14%

Total consolidation due to water and 3000 psf pressure = 1.89%

Initial Moisture Content 13.2 % Final Moisture Content 16.0 % Initial Dry Density 108.1 pcf Final Dry Density 110.4 pcf Initial Wet Density 122.4 pcf Final Saturated Density 128.0 pcf





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Swell/Consolidation Test

ASTM D4546

Lots 89A, 109, and 110 TMV Project Name Date 10/30/07 Project Location Lots 89A, 109, and 110 TMV Project # 07-383-GEO Client Juno Stonegate Developmen Sampled by Sample Location BH#2 @10.5-11.5' Tested by DS7 Sample #

Initial compression due to 1000 psf pressure = 1.92% Swell potential due to water and 1000 psf pressure = 0.04%

Soil Description olive yellow clayey SAND with gravel

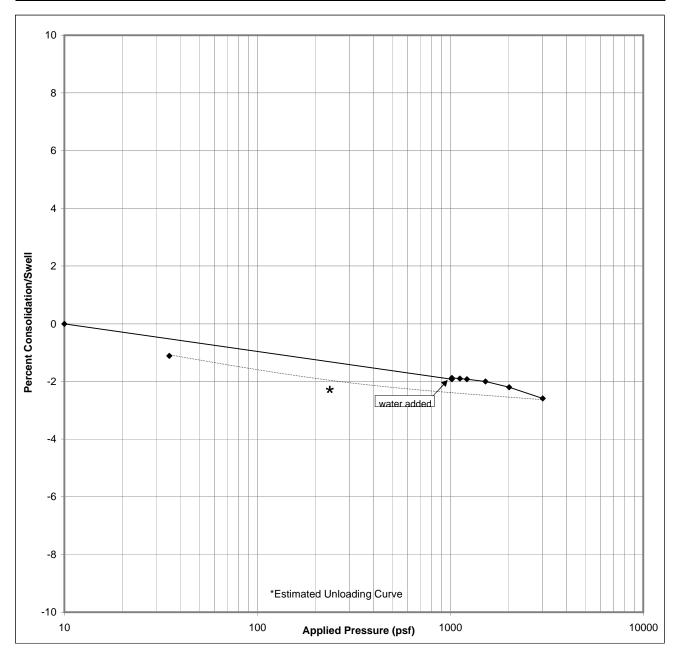
Total consolidation due to water and 3000 psf pressure = 0.71%

Estimated swell pressure = 1130 psf

Initial Moisture Content 18.6 % Initial Dry Density 109.3 pcf Initial Wet Density 129.7 pcf Final Moisture Content Final Dry Density

17.0 % 112.9 pcf

Final Saturated Density 132.1 pcf





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Swell/Consolidation Test

ASTM D4546

 Project Name
 Lots 89A, 109, and 110 TMV
 Date
 10/30/07

 Project Location Client
 Lots 89A, 109, and 110 TMV
 Project # 07-383-GEO

 Client
 Juno Stonegate Developmer
 Sampled by BB

 Sample Location Sample # Soil Description
 DS8

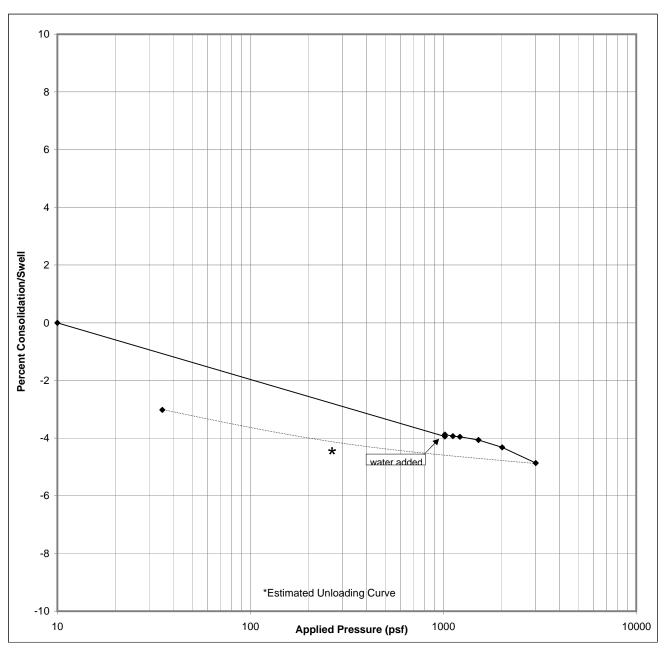
 Soil Description
 olive brown lean CLAY with sand

Initial compression due to 1000 psf pressure = 3.94% Swell potential due to water and 1000 psf pressure = 0.06%

Total consolidation due to water and 3000 psf pressure = 0.98%

Estimated swell pressure = 1130 psf

Initial Moisture Content
Initial Dry Density20.1 %
104.1 pcfFinal Moisture Content
Final Dry Density21.6 %
105.0 pcfInitial Wet Density124.9 pcfFinal Saturated Density127.6 pcf







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Corrosivity Series

Based on HACH methods

| Project Name | Lots 89A, 109, and 110 TMV | Date Tested_ | 11/6/2007 |
|------------------|---|--------------|------------|
| Project Location | Lots 89A, 109, and 110 TMV | Project # | 07-383-GEO |
| Client | Juno Stonegate Development | Sample by | BB |
| Test Location | BH#1 @10.5-11.5' | Tested by | DJ |
| Sample # | DS2 | | _ |
| Soil Description | very dark gray clayey SAND with gravel (SC) | | |

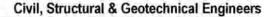
In-situ Moisture Content 19.6 %

Water-soluble sulfates, dry soil basis 0.030 %

Chlorides 10 ppm

Electro-conductivity 15 µS/cm

pH 7.10





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Corrosivity Series

Based on HACH methods

| Project Name | Lots 89A, 109, and 110 TMV | Date Tested | 11/6/2007 |
|------------------|--|-------------|------------|
| Project Location | Lots 89A, 109, and 110 TMV | Project # | 07-383-GEO |
| Client | Juno Stonegate Development | Sample by | BB |
| Test Location | BH#2 @10-11.5' | Tested by | DJ |
| Sample # | DS7 | | _ |
| Soil Description | reddish brown clayey SAND with gravel (SC) | | |

In-situ Moisture Content 17.5 %

Water-soluble sulfates, dry soil basis 0.030 %

Chlorides 35 ppm

Electro-conductivity 80 µS/cm

pH 7.05

RETAINING WALL SCHEMATIC "BACKSLOPE ZONE" IS THE AREA IN WHICH LOADS APPLIED TO THE GROUND SURFACE OR INCREASE IN SLOPE ANGLE WILL INCREASE THE TOTAL LATERAL LOAD ON THE RETAINING WALL. BASEMENT OR RETAINING WALL BACKSLOPE ZONE "ZONE OF INFLUENCE" IS THE APPROXIMATE AREA IN WHICH THE RETAINED MATERIAL PROPERTIES AFFECT THE LATERAL EARTH PRESSURES TRANSMITTED TO THE RETAINING WALL. ZONE OF BACKFILL MATERIAL SHOULD BE PLACED INFLUENCE AND COMPACTED IN ACCORDANCE WITH RECOMMENDATIONS OF SOIL REPORT. HINGE POINT IS BOTTOM OF EXCAVATION FOR WALL WITHOUT FOOTING OR OUTSIDE EDGE OF CONCRETE IF FOOTING IS USED. SCHEMATIC - NOT TO SCALE

| DRAWING | DESIGNER | TG |
|---------|----------|-----------------|
| NUMBER | DRAFTER | SD |
| 7 | DATE | 27 JANUARY 2003 |

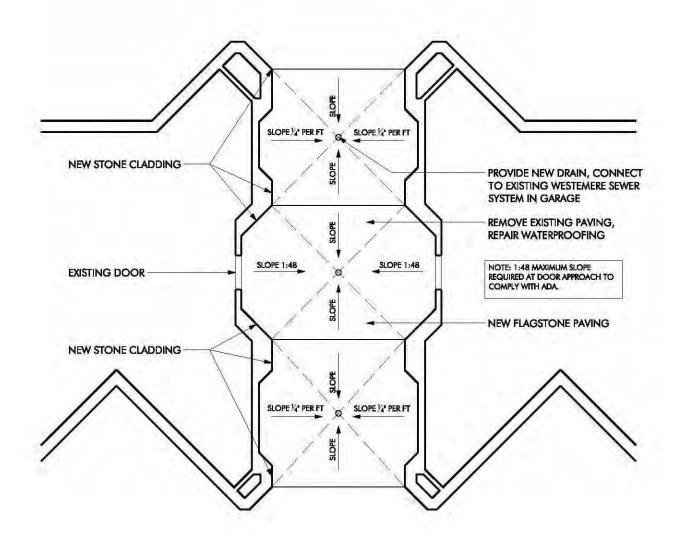
JOB NO.

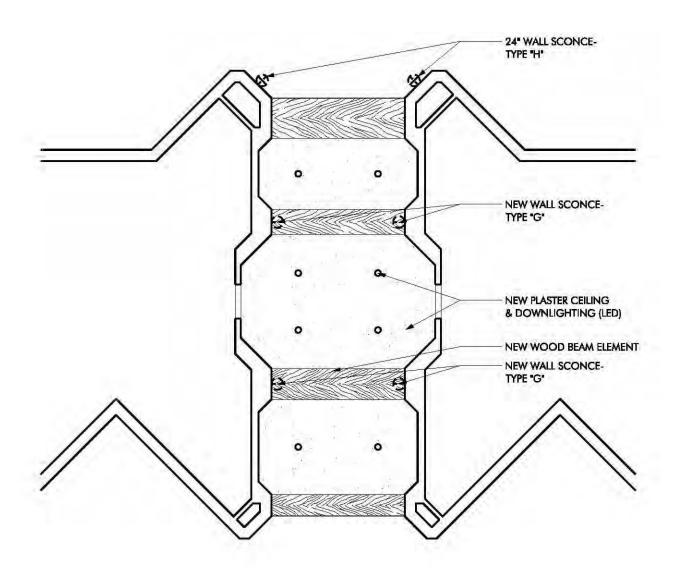
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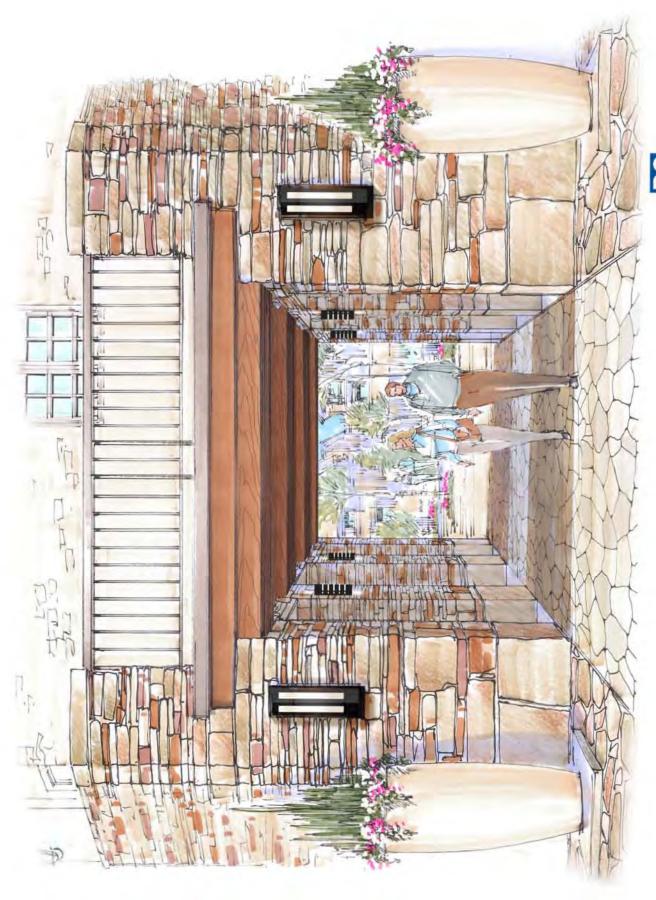
RETAINING WALL SCHEMATIC

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MESTEMERE IMAGE 11.18.2010 11.18.2010 PROJECT NO: 08131.100

MOUNTAIN VILLAGE HOTEL 628, 632, 636, 642, 683 Mountain Village Blvd, Mountain Village, CO 81435; LOTS 73-76R, 89A, 109,110

L1.016 LANDSCAPE PLAN

12.01 IRRIGATION DETAILS

ARCHITECTURAL DRAWINGS

A1.01b CONSTRUCTION STAGING PLAN

A1.01e SITE OWNERSHIP DIAGRAM

A1.08 LEVEL 2 LIGHTING PLAN

A1.09 LEVEL 3 LIGHTING PLAN

A1.11 LEVEL 5 LIGHTING PLAN A1.12 LEVEL 6 LIGHTING PLAN

A1.13 LEVEL 7 LIGHTING PLAN

E1.00 LIGHTING CUT SHEETS

E1.08 LEVEL 1 LIGHTING PLAN

PTP.200 GARAGE BASEMENT POINT TO POINT

PTP.201 LOWER GARAGE POINT TO POINT PTP.202 UPPER GARAGE POINT TO POINT

PARKING DIAGRAM PLAN (REFERENCE ONLY)

L1.02 WESTEMERE IMPROVEMENT PLAN

| GENERA | ************************************** | |
|---------------|--|----------------|
| 1000-1000-000 | COVER SHEET/INDEX | Toward Company |
| A0.00 | SHEET INDEX & PROJECT INFORMATION | • |
| CIVIL DI | RAWINGS (UNDER SEPERATE COVER) | |
| CO.OO | SHEET INDEX & PROJECT INFORMATION | |
| DM1 | DEMOLITION PLAN | • |
| SP1 | SITE PLAN | • |
| SP2 | SITE PLAN | • |
| OUI | OVERALL UTILITY PLAN | • |
| GR1 | GRADING PLAN | • |
| ECI | EROSION CONTROL PLAN | • |
| SDI | STORM DRAIN PLAN AND PROFILE | • |
| SD2 | STORM DRAIN PLAN AND PROFILE | |
| 1022 | SANITARY SEWER PLAN AND PROFILE | • |
| WTOI | WATER MAIN PLAN AND PROFILE | |
| UR1 | S.M.P.A. UTILITY RELOCATION PLAN | • |
| UR2 | QWEST UTILITY RELOCATION PLAN | • |
| UR3 | CABLE TY LITILITY RELOCATION PLAN | • |
| DT1 | DETAILS - GRADING AND EROSION CONTROL | • |
| DT2 | DETAILS - STORM DRAINAGE | • |
| DT3 | DETAILS - STORM DRAINAGE & ROADWAY | • |
| DT4 | DETAILS - SANITARY SEWER | • |
| DT5 | DETAILS - WATER | • |
| LANDSO | CAPE DRAWINGS | |
| L1.01 | LANDSCAPE PLAN | • |
| L1.01a | LANDSCAPE PLAN | • |
| 25 | | |

| ARCHITI | ECTURAL DRAWINGS | ا ا |
|---------|--------------------------------------|-----|
| A2.04 | LEVEL 1 FLOOR PLAN - OVERALL | • |
| A2.05 | LEVEL 2 FLOOR PLAN - OVERALL | |
| A2.06 | LEVEL 3 FLOOR PLAN - OVERALL | |
| A2,07 | LEVEL 4 FLOOR PLAN - OVERALL | |
| A2.08 | LEVEL 5 FLOOR PLAN - OVERALL | |
| A2.09 | LEVEL 6 FLOOR PLAN - OVERALL | • |
| A2.10 | LEVEL 7 FLOOR PLAN - OVERALL | |
| A2.11 | ROOF PLAN - OVERALL | • |
| A2.12 | AVERAGE HEIGHT TARGA PLAN | |
| A213 | MAXIMUM HEIGHT PLAN | • |
| A3.02 | SITE CIRCULATION PLAN | • |
| A4.01 | EXTERIOR ELEVATION - OVERALL | |
| A4.02 | EXTERIOR ELEVATION - OVERALL | |
| A4.03 | EXTERIOR ELEVATION - OVERALL | |
| A4.04 | EXTERIOR ELEVATION - OVERALL | 9 |
| A4.05 | EXTERIOR ELEVATION - OVERALL | - 4 |
| A4.06 | EXTERIOR ELEVATION - OVERALL | |
| A4.07 | EXTERIOR ELEVATION - OVERALL | |
| A4.08 | EXTERIOR ELEVATION - OVERALL | |
| A4.09 | EXTERIOR ELEVATION - OVERALL | - |
| A4,10 | EXTERIOR ELEVATION - OVERALL | |
| 4.21 | EXTERIOR ELEVATION - SNOW MELT STUDY | |
| 4.22 | EXTERIOR ELEVATION - SNOW MELT STUDY | |
| 4.23 | EXTERIOR ELEVATION - SNOW MELT STUDY | |
| 4.24 | EXTERIOR ELEVATION - SNOW MELT STUDY | 9 |
| 4.25 | EXTERIOR ELEVATION - SNOW MELT STUDY | |
| 4.26 | EXTERIOR ELEVATION - SNOW MELT STUDY | 9 |
| 4.27 | EXTERIOR ELEVATION - SNOW MELT STUDY | |
| 4.28 | EXTERIOR ELEVATION - SNOW MELT STUDY | |
| 4.29 | EXTERIOR ELEVATION - SNOW MELT STUDY | |
| 4.30 | EXTERIOR ELEVATION - SNOW MELT STUDY | • |
| A5.01 | BUILDING SECTION | |
| A5.02 | BUILDING SECTION | |
| A5.03 | BUILDING SECTION | • |
| A5.04 | BUILDING SECTION | |
| A5.05 | BUILDING SECTION | |
| A6.01 | TYPICAL EXTERIOR DETAILS | 9 |
| A6.01a | TYPICAL EXTERIOR DETAILS | |
| A6.01b | TYPICAL EXTERIOR DETAILS | |
| A6 02 | MISCELLAMENTS DETAILS | 1.0 |

A6.04 UPPER MOUNTAIN VILLAGE BLVD SITE DETAILS

A6.05 UPPER MOUNTAIN VILLAGE BLVD SITE DETAILS

MOUNTAIN VILLAGE HOTEL TOWN OF MOUNTAIN VILLAGE, COLORADO

SIGNATURE OF PROPERTY OWNER:

APPROVAL BY DESIGN REVIEW BOARD:

APPROVAL BY PLANNING DIRECTOR:

FINAL PUD APPLICATION-**TOWN COUNCIL REVIEW**

ISSUED: NOVEMBER 18, 2010

Project Number 08131.100



architecture· interiors· planning· graphics 8070 Park Lane, Suite 300 Dallas TX 75231 Tel. 972.701.9000 Fax 972.991.3008 www.bokapowell.com

| COUNTRY CLUB DR | | VICINITY MAP SCALE: 1"=1000' | NORTH AGE BLYD. | NOTE: GROSS RESIDENTIAL FLOOR AREA IS CALCULATED AS THE ENTIRE FLOOR TO THE EXTERIOR FINISH FACE OF EXTERIOR WALL |
|--|------------|---------------------------------------|-------------------|---|
| DEVELOPMENT STANDA | RDS | CONCEPTUAL PUD APPROVA | L PROVIDED SKETCH | PROVIDED FINAL |
| BUILDING FOOTPRINT AREA | | 24,882 SF 24,881 SF | | 24,881 SF |
| GROSS LOT AREA EXISTING (SF & ACREAGE) | | | | 14374.8 SF .33 ACRES |
| GROSS LOT AREA PROPOSED (SF & ACREAGE) | | 36080 SF .83 ACRES 36080 SF .83 ACRES | | 35928 SF .825 ACRES |
| GROSS FLOOR AREA - BUILDING | ł | 167,790 SF | 167,790 SF | 167,790 SF |
| GROSS FLOOR AREA - GARAGE | | 82359 SF | 88054 SF | 88054 SF |
| BUILDING SETBACKS | | 15'-0" | 15'-0" | 15'-0" |
| MAXIMUM BUILDING HT. APPRO AT CONCEPTUAL PUD | VED | 89'-0" | 88'-9" | 88'-9" |
| MAXIMUM AVERAGE BUILDING APPROVED AT CONCEPTUAL PU | | 65'-3.75" | 65'-2.9" | 65'-2.9" |
| COMMERCIAL SF | | 20213 SF | 20164 SF | 20164 SF |
| PLAZA SF | | 10156.56 SF | 10156.56 SF | 10156.56 SF |
| UNITS # UNI | | DENSITY PER | TOTAL DENSITY | TOTAL DENSITY |
| DEDICATED EFFICIENCY LODGI | 40 | .5 | 20 | 20 |
| EFFICIENCY LODGE UNITS 2 | | .5 | 13 | 13 |
| LODGE UNITS | | .75 | 28.5 | 28.5 |
| CONDOMINIUMS 20 | | 3 | 60 | 60 |
| DRU | 1 | 3 | 3 | 3 |
| TOTA REQUIRED PARKING | AL DENSITY | | 124.5 | 124.5 |
| TOTAL THE A PARTICLE STOCK STO | | | | |
| DEDICATED EFFICIENCY LODGE, EFFICIENCY LODGE, AND LODGE UNITS | | 55 | | |
| CONDOMINIUMS | | 20 | | |
| DRU | | 1 | | |
| COMMERCIAL SPACES | | 21 | | |
| AGREED DEDICATEDTO TOWN AT CONCEPT PUD APPROVAL | | 48 | | |
| НОА | | 5 | | |
| TOTAL REQ.PARKING | | 150 | 161 | |
| HANDICAPPED PARKING (INCL. IN TOTAL) | | 4 | | |
| SURVEY CONSULTANT | CIVII EI | ICINEED LANDS | CAPE CONSULTANT | STRUCTURAL ENGI |

LANDWORKS DESIGN, INC.

Denver, CO 80216

3457 Ringsby Court, Unit 110

CALIBRE ENGINEERING, INC.

Littleton, CO 80120

8201 South Park Lane, Ste 200

FOLEY ASSOCIATES

Telluride, CO 81435

P.O. Box 1385

| EXISTING | ZONING | }: | | | | | LEGAL DESCRIPTION OF PROPERTY: |
|------------------|--------------|-----------------------|-----------------------|-------|---------------------|--|--|
| LOT | ACREAGE | ZONE DISTRICT | ZONING DESIGNATION | UNITS | DENSITY PER UNIT | TOTAL DENSITY | LOT 73-76R: LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF LOTS 73 AND 78, TOWN OF MOUNTAIN VILLAGE AND REPLAT OF TRACT OS-3BR, TOWN O MOINTAIN VILLAGE, ACCORDING TO THE PLAT RECORDED JANUARY 25, 2607 IN PLAT BOOK 1 AT PAGE 3807 AT RECEPTION NO. 389801, COUNTY OF SAN MIGUEL, STATE OF COLORADO |
| LOT 73-76R: .131 | .131 VILLAGE | CONDO | 12 | 3 | 36 | LOT 109: | |
| | | CENTER | COMMERCIAL | | | | LOT 109, REPLAT NO. 3, TELLURIDE MOUNTAIN VILLAGE, FILING 1, ACCORDING TO THE PLAT RECORDER JULY 31, 1985 IN PLAT BOOK 1 AT PAGE 577, COUNTY OF SAN MIGUEL, STATE OF COLORADO. LOT 110: |
| | | | DRU | 1 | 3 | 3 | LOT 110, REPLAT NO. 3, TELLURIDE INCURTAIN VILLAGE, FILING 1, ACCORDING TO THE PLAT RECORDED JULY 31, 1985 IN PLAT BOOK 1 AT PAGE 577, COUNTY OF SAN INCUEL, STATE OF COLORADO. |
| LOT 109: .092 | VILLAGE | CONDO | 8 | 3 | 24 | LOT 89-A: | |
| | CENTER | COMMERCIAL | | | | ACCESS TRACT 89-A, TELLURIDE MOUNTAIN VILLAGE, ACCORDING TO THE FIRST REPLAT OF COMBINED LOTS 133 AND 83-1. TELLURIDE MOUNTAIN VILLAGE, FILING 1 RECORDED DECEMBER 28, 1988 IN PLAT | |
| LOT 110: | OT 110: .077 | VILLAGE | CONDO | 6 | 3 | 18 | BOOK 1 AT PAGE 980, COUNTY OF SAN, MIGUEL, STATE OF COLORADO |
| | CENTER | COMMERCIAL | | | | GENERAL NOTES/ CONDITION OF APPROVAL: | |
| LOT 89A: | .020 | VILLAGE | CONDO | | | | CONDITION OF APPROVAL: |
| | | CENTER | COMMERCIAL | | | | |
| DS3-BR: | 2.489 | OPEN SPACE | ACTIVE OPEN SPACE | | | | DATE OF APPROVAL: |
| TOTAL: | | 336 (20) (3 3) (40) | 1000 CO 1000 CO | 27 | | 81 | SKETCH PLAN EXPIRATION: |

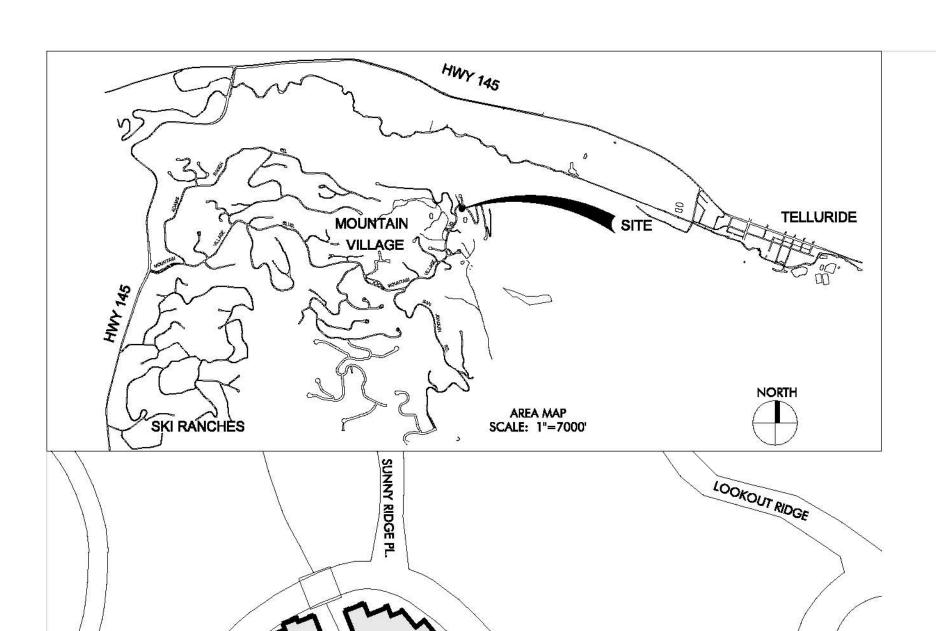
JCAA CONSULTING ENGINEERS LLC 13772 Denver West Parkway Lakewood, CO 80401

R J C CONSULTING ENGINEERS

1285 West Broadway, Sulte 300

Vancouver, BC V6H 3X8 Canada

MV COLORADO DEVELOPMENT PARTNERS, LLC; C/O UNITY HUNT INC. 1601 ELM ST. STE. 4000 **DALLAS, TX 75201**



| OB DR | | SCALE: 1"=1000' | | OF EXTERIOR WALL |
|---|------------|-------------------------|----------------------|---------------------|
| DEVELOPMENT STANDA | ARDS | CONCEPTUAL PUD APPROVAL | PROVIDED SKETCH | PROVIDED FINAL |
| BUILDING FOOTPRINT AREA | | 24,882 SF | 24,881 SF | 24,881 SF |
| GROSS LOT AREA EXISTING (SF & ACREAGE) | | 14374.8 SF .33 ACRES | 14374.8 SF .33 ACRES | 14374.8 SF .33 ACRE |
| GROSS LOT AREA PROPOSED (SF & ACREAGE) | | 36080 SF .83 ACRES | 36080 SF .83 ACRES | 35928 SF .825 ACRE |
| GROSS FLOOR AREA - BUILDIN | IG | 167,790 SF | 167,790 SF | 167,790 SF |
| GROSS FLOOR AREA - GARAGE | u C | 82359 SF | 88054 SF | 88054 SF |
| BUILDING SETBACKS | | 15'-0" | 15'-0" | 15'-0" |
| MAXIMUM BUILDING HT. APPI AT CONCEPTUAL PUD | ROVED | 89'-0" | 88'-9" | 88'-9" |
| MAXIMUM AVERAGE BUILDING APPROVED AT CONCEPTUAL I | HT. PUD | 65'-3.75" | 65'-2.9" | 65'-2.9" |
| COMMERCIAL SF | | 20213 SF | 20164 SF | 20164 SF |
| PLAZA SF | | 10156.56 SF | 10156.56 SF | 10156.56 SF |
| UNITS | # UNITS | DENSITY PER | TOTAL DENSITY | TOTAL DENSITY |
| DEDICATED EFFICIENCY LODG | Œ 40 | .5 | 20 | 20 |
| EFFICIENCY LODGE UNITS | 26 | .5 | 13 | 13 |
| LODGE UNITS | 38 | .75 | 28.5 | 28.5 |
| CONDOMINIUMS | 20 | 3 | 60 | 60 |
| DRU | 1 | 3 | 3 | 3 |
| TOTAL DENSITY | | | 124.5 | 124.5 |
| REQUIRED PARKING | | | | |
| DEDICATED EFFICIENCY LODGE, EFFICIENCY LODGE, AND LODGE UNITS | | 55 | | |
| CONDOMINIUMS | | 20 | | |
| DRU | | 1 | | |
| COMMERCIAL SPACES | | 21 | | |
| AGREED DEDICATEDTO TOWN AT CONCEPT PUD APPROVAL | | 48 | | |
| НОА | | 5 | | |
| TOTAL REQ.PARKING | | 150 | 161 | |
| HANDICAPPED PARKING (INCL. IN TOTAL) | | 4 | | |

VICINITY MAP

| SURVEY CONSULTANT | CIVIL ENGINEER | LANDSCAPE CONSULTANT | STRUCTURAL ENGINEER |
|-------------------|----------------|----------------------|---------------------|
| | | | |

FOLEY ASSOCIATES R J C CONSULTING ENGINEERS CALIBRE ENGINEERING, INC. LANDWORKS DESIGN, INC. P.O. Box 1385 8201 South Park Lane, Ste 200 3457 Ringsby Court, Unit 110 1285 West Broadway, Sulte 300 Telluride, CO 81435 Littleton, CO 80120 Denver, CO 80216 Vancouver, BC V6H 3X8 Canada

JCAA CONSULTING ENGINEERS LLC 13772 Denver West Parkway Lakewood, CO 80401

MEP ENGINEER

EXISTING ZONING:

.092

.077

.020

2.489

LOT 73-76R: .131

LOT 109:

LOT 110:

LOT 89A:

OS3-BR:

TOTAL:

SHEET INDEX

EROSION CONTROL PLAN

27 OWNER/APPLICANT

DISTRICT DESIGNATION

CONDO COMMERCIAL

CONDO

COMMERCIAL

CONDO

COMMERCIAL

CONDO COMMERCIAL

ACTIVE OPEN

SPACE

VILLAGE

VILLAGE

VILLAGE

CENTER

VILLAGE

SPACE

MV COLORADO DEVELOPMENT PARTNERS, LLC; C/O UNITY HUNT INC. 1601 ELM ST. STE. 4000 **DALLAS, TX 75201**

DENSITY TOTAL

36

24

UNITS PER UNIT DENSITY

12 3

1 3

8 3

6 3

LEGAL DESCRIPTION OF PROPERTY:

ACCESS TRACT 59-A, TELLURIDE MOUNTAIN VILLAGE, ACCORDING TO THE FIRST REPLAT OF COMBINED LOTS 133 AND 69-1, TELLURIDE MOUNTAIN VILLAGE, FILING 1 RECORDED DECEMBER 26, 1989 IN PLAT BOOK 1 AT PAGE 980, COUNTY OF SAN, MIGUEL, STATE OF COLORADO

LOT 73-76R:

LOT 110:

LOT 89-A:

GENERAL NOTES/

EXPIRATION DATE:

SKETCH PLAN EXPIRATION:

DATE OF APPROVAL:

CONDITION OF APPROVAL:

MOUNTAIN VILLAGE HOTEL TOWN OF MOUNTAIN VILLAGE, COLORADO

| SIGNATURE OF PROPERTY OWNER: |
|----------------------------------|
| Robert Augen, De |
| APPROVAL BY DESIGN REVIEW BOARD: |

APPROVAL BY PLANNING DIRECTOR:

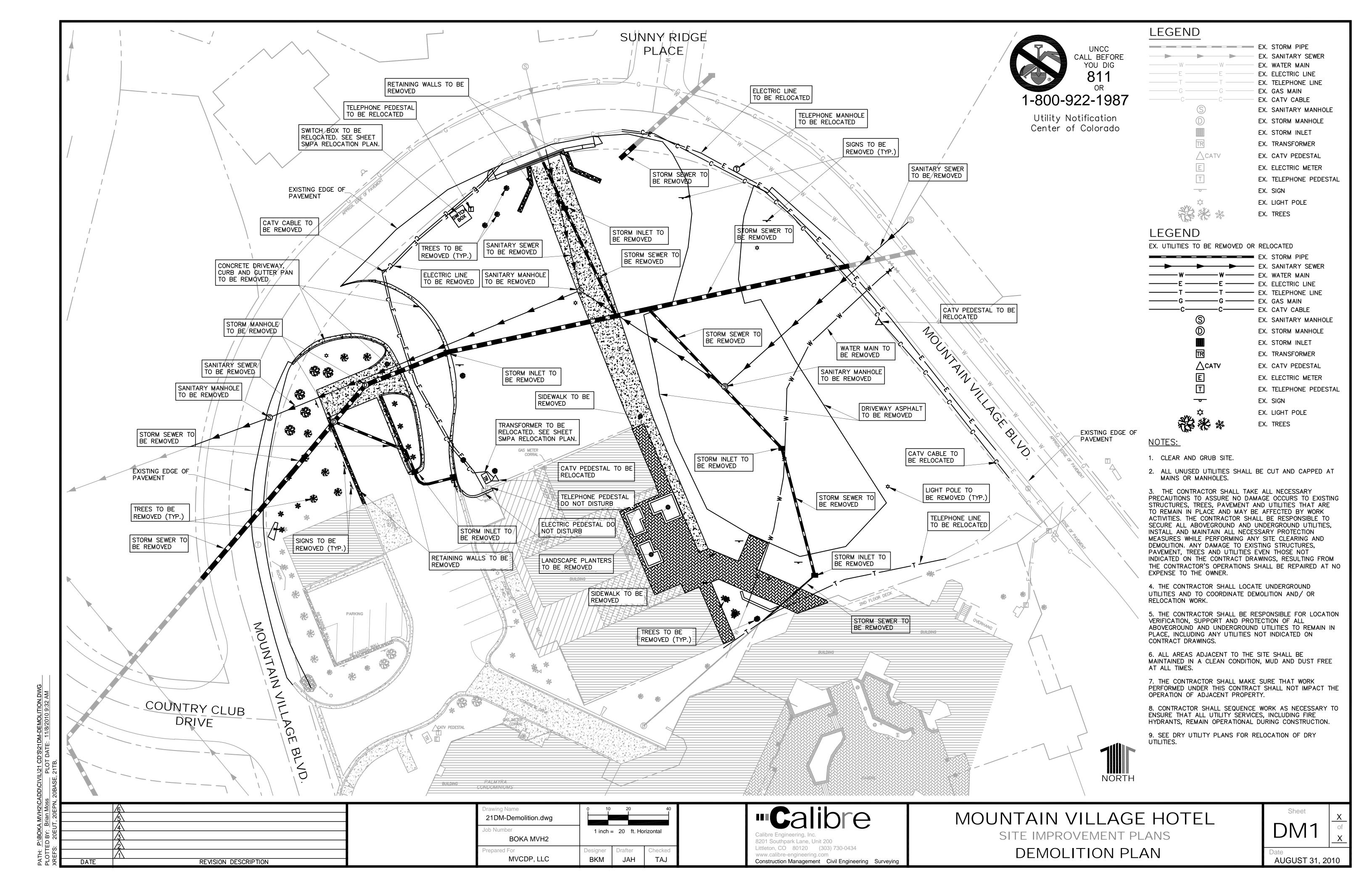
FINAL PUD APPLICATION-**TOWN COUNCIL REVIEW**

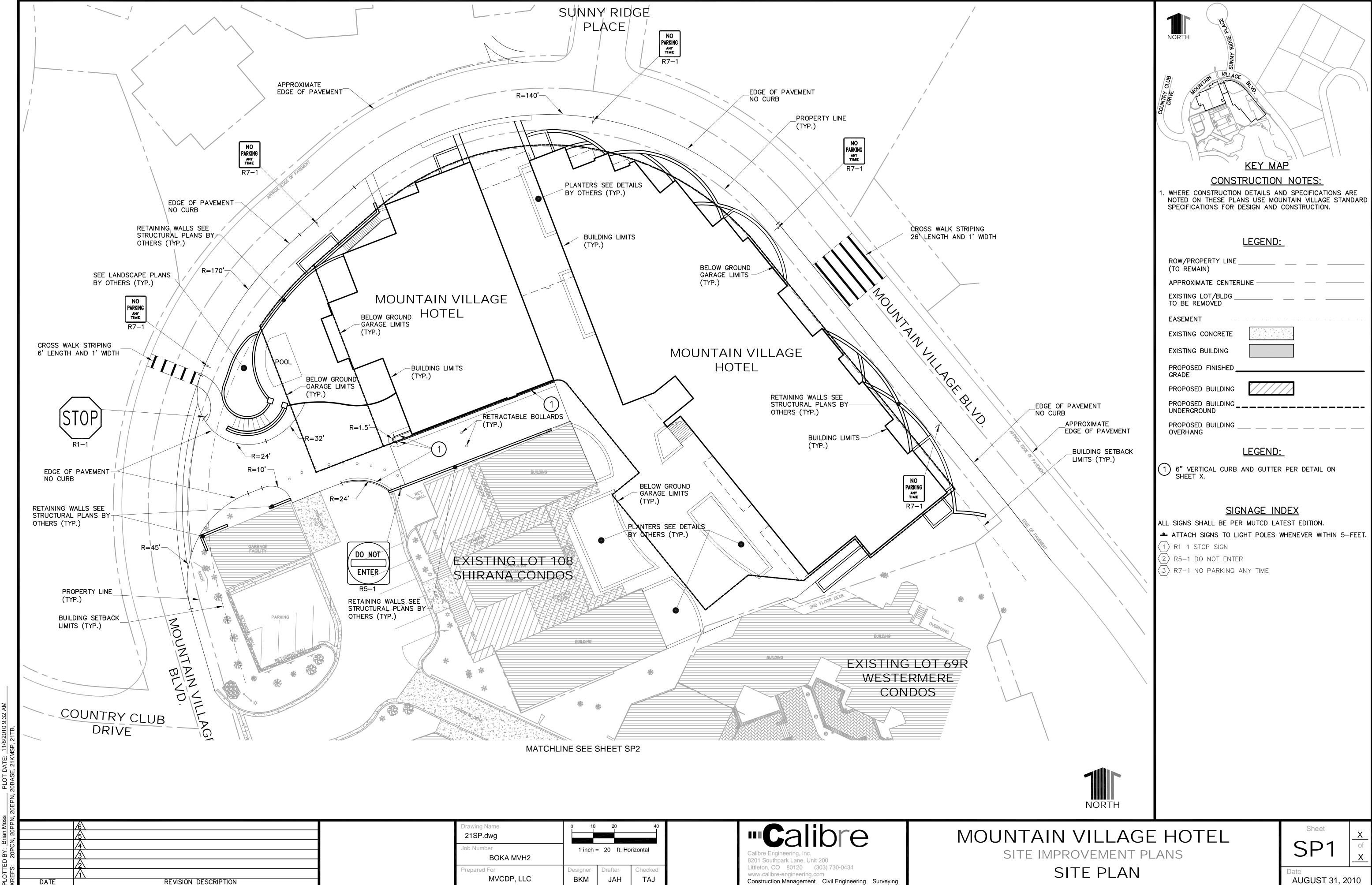
ISSUED: NOVEMBER 18, 2010

Project Number 08131.100



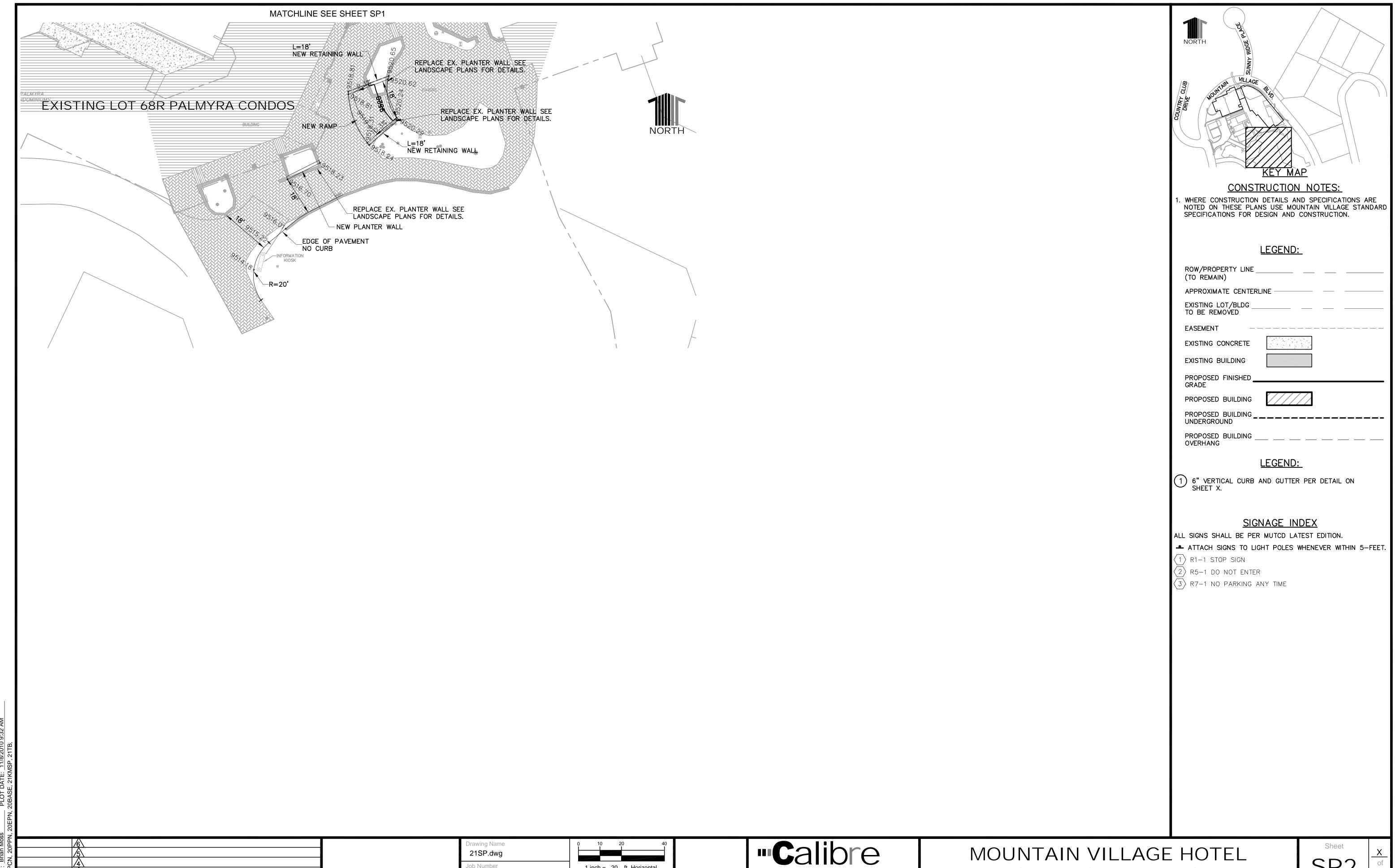
architecture· interiors· planning· graphics 8070 Park Lane, Suite 300 Dallas TX 75231 Tel. 972.701.9000 Fax 972.991.3008 www.bokapowell.com





MVCDP, LLC BKM TAJ JAH

AUGUST 31, 2010



21SP.dwg

Job Number

REVISION DESCRIPTION

BOKA MVH2

MVCDP, LLC

1 inch = 20 ft. Horizontal

JAH

TAJ

BKM

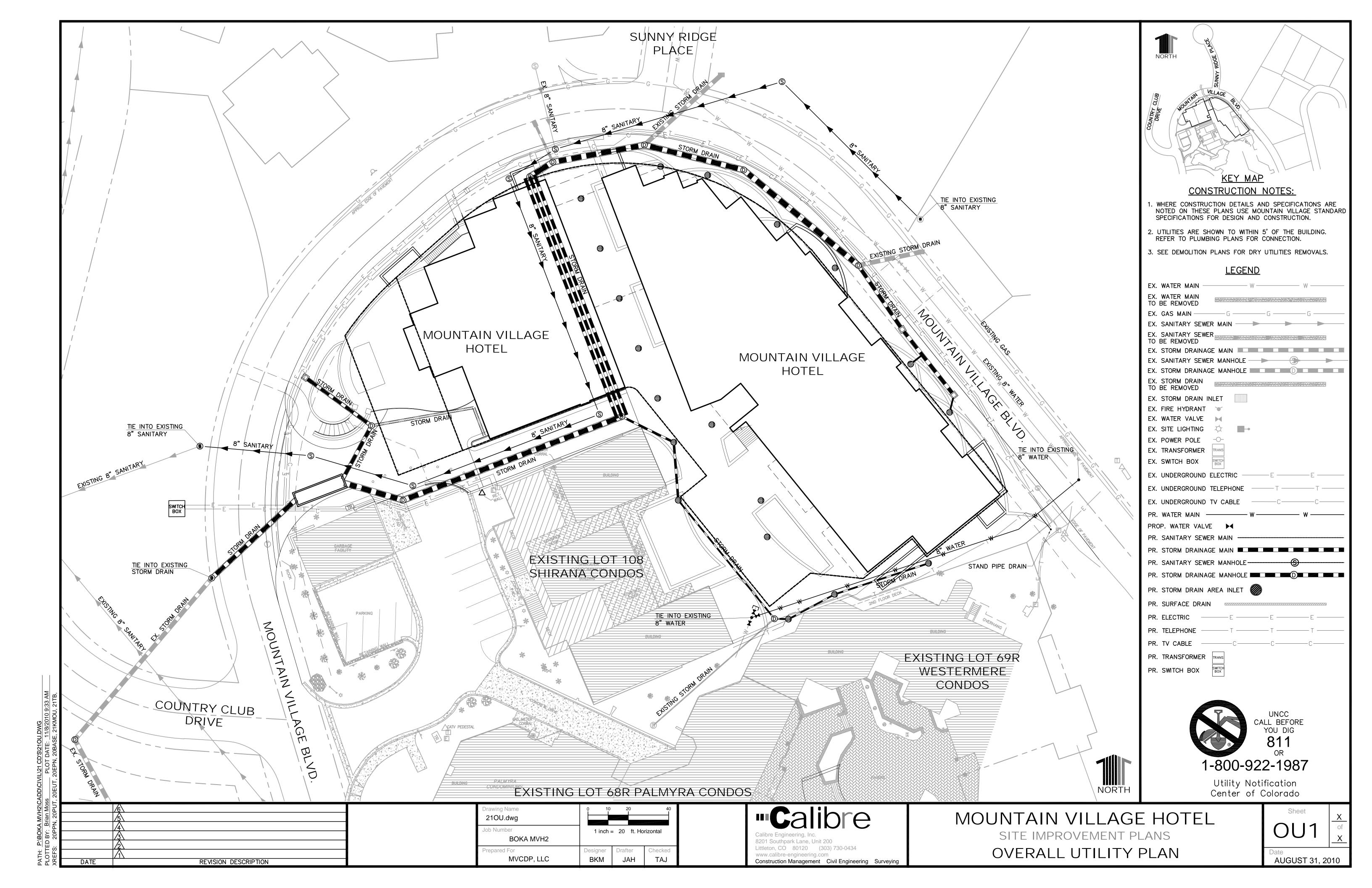
8201 Southpark Lane, Unit 200 Littleton, CO 80120 (303) 730-0434

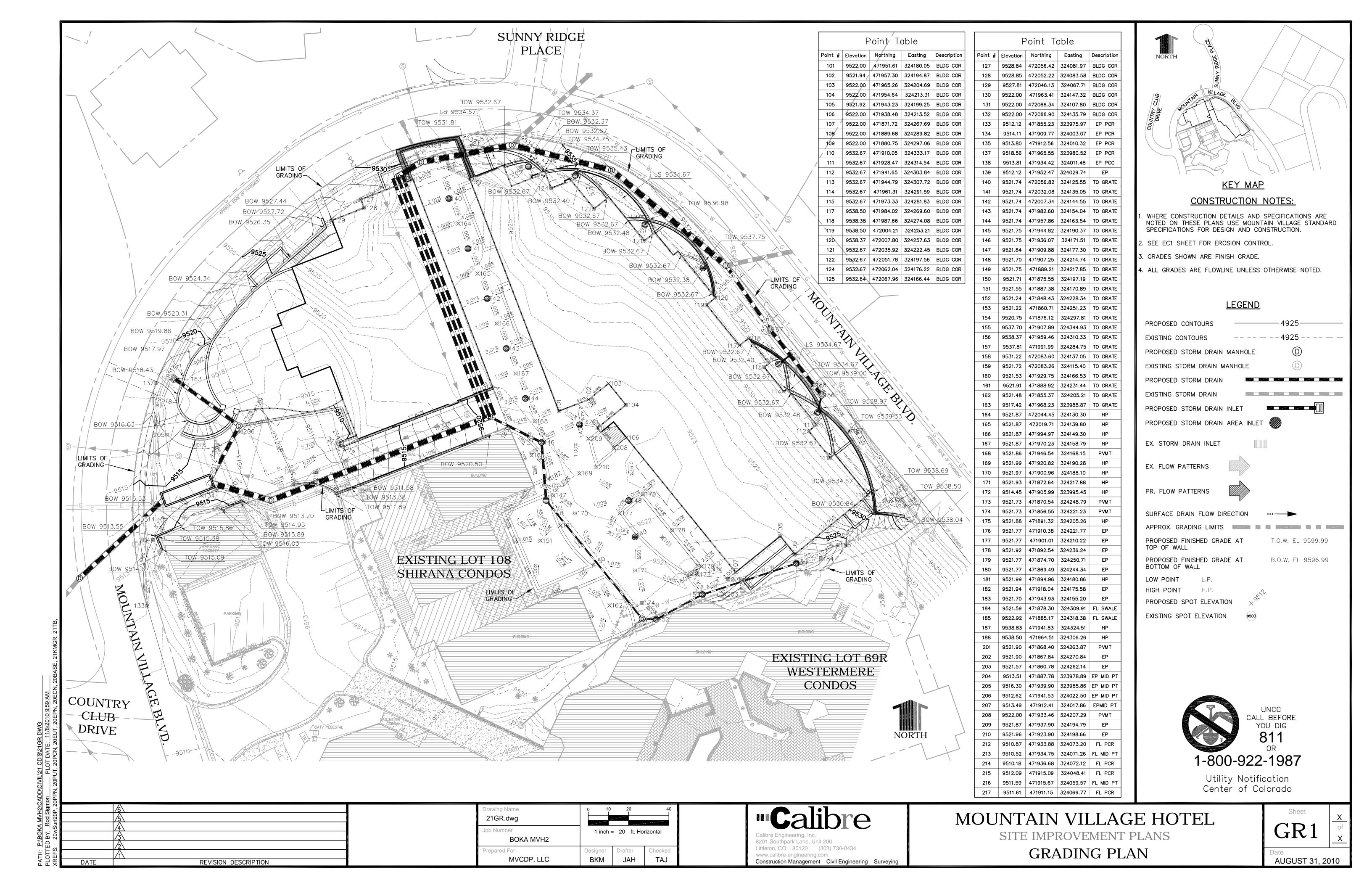
www.calibre-engineering.com

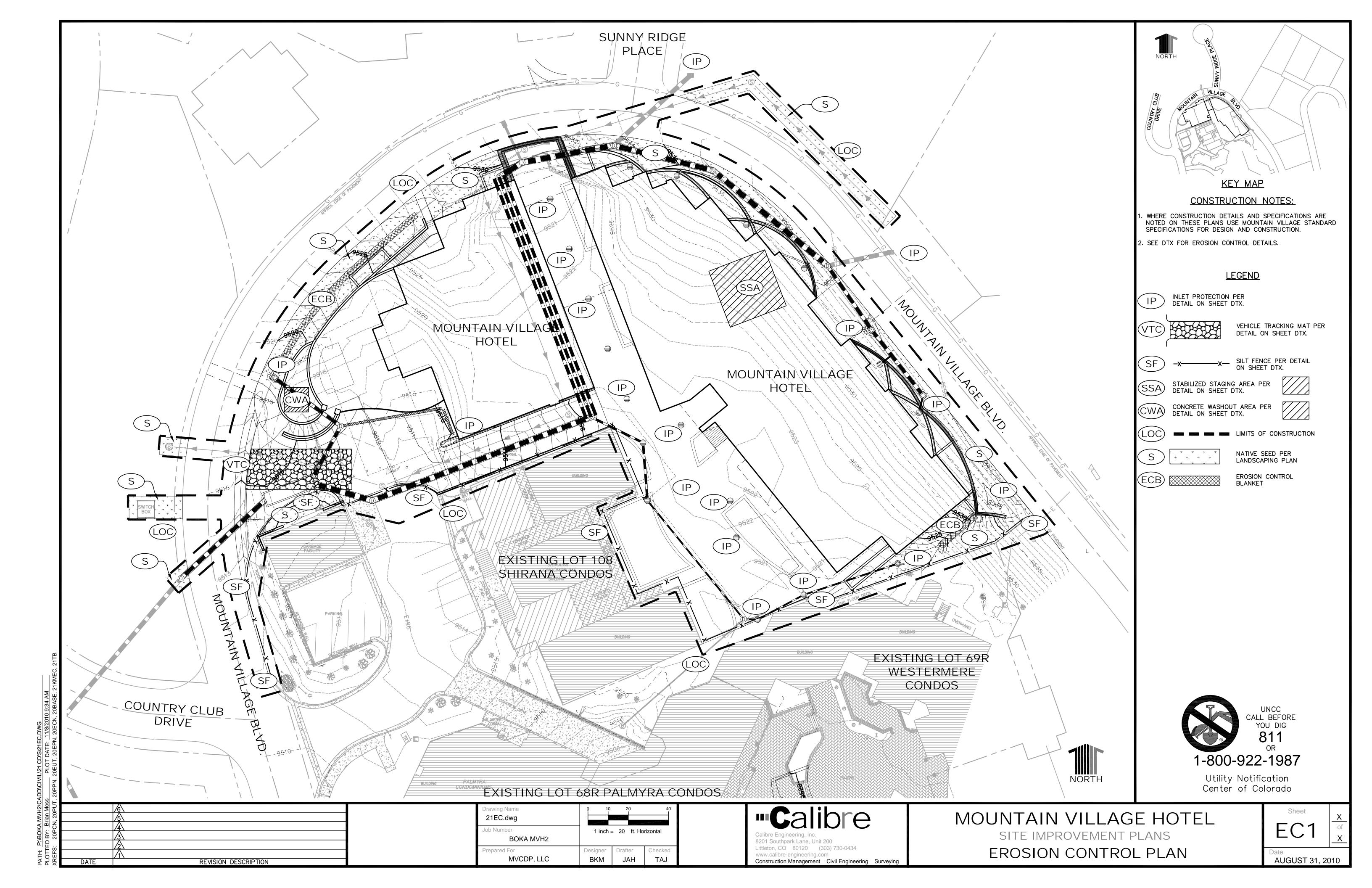
Construction Management Civil Engineering Surveying

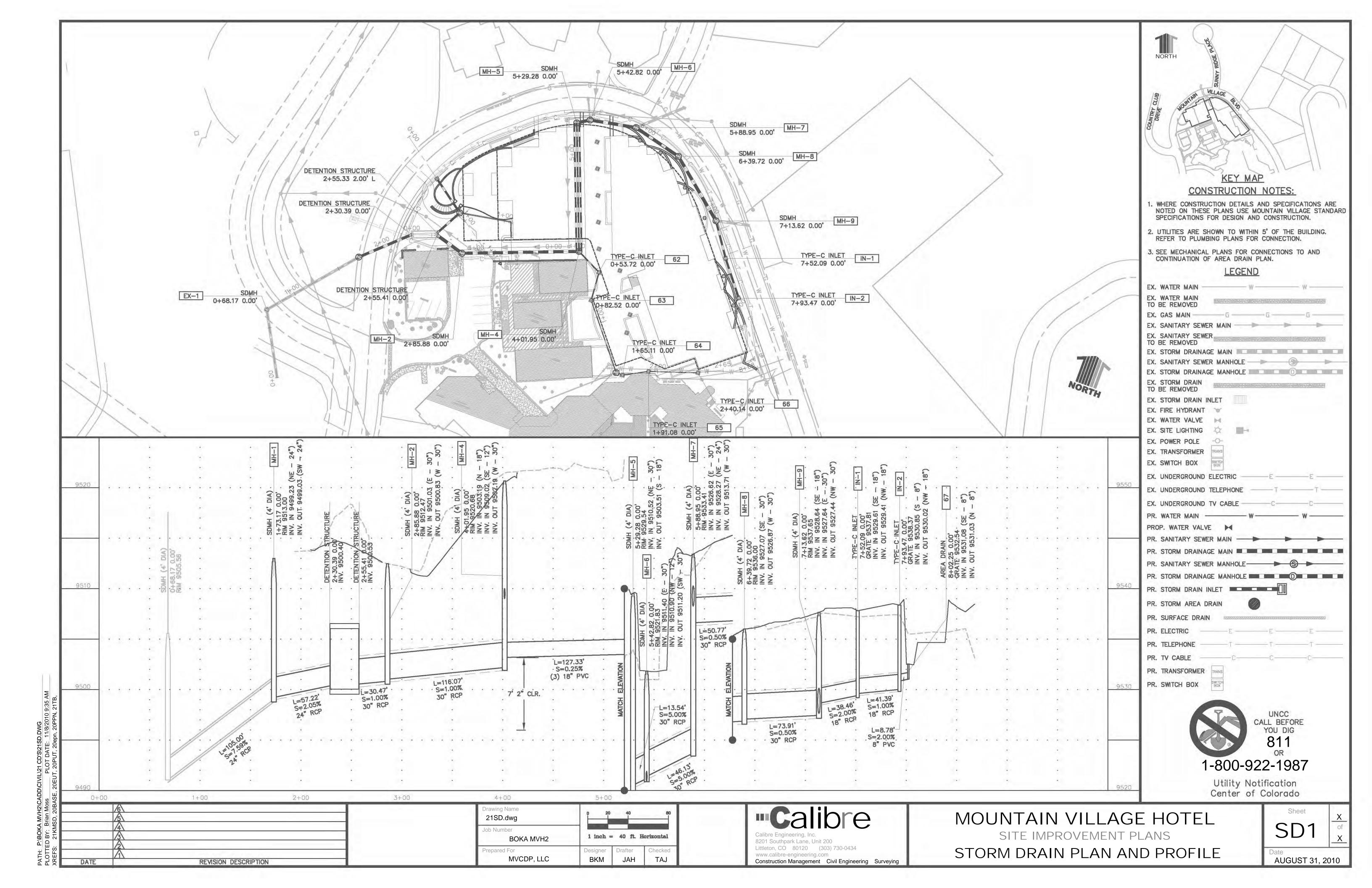
MOUNTAIN VILLAGE HOTEL SITE IMPROVEMENT PLANS SITE PLAN

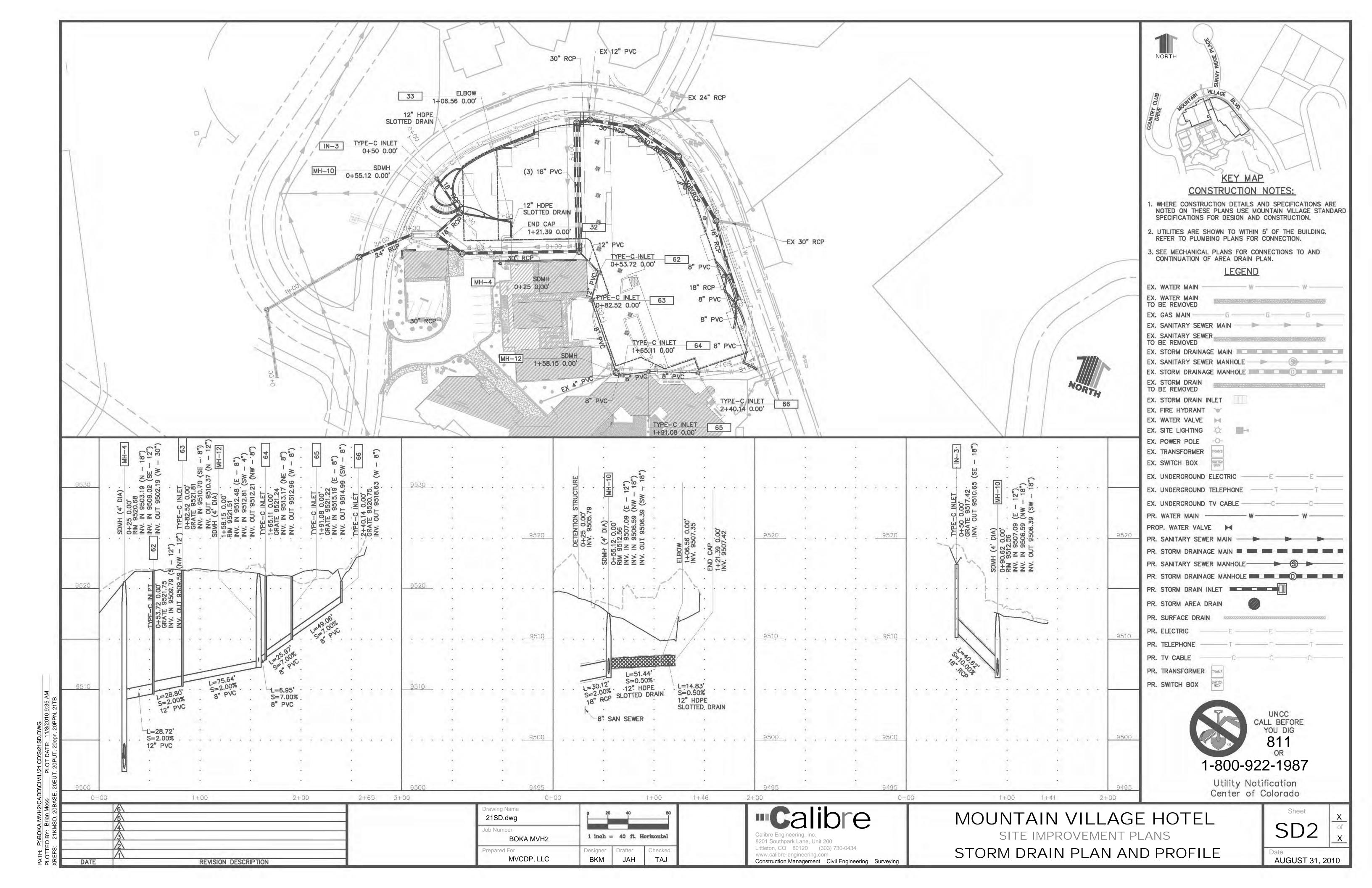
SP2 AUGUST 31, 2010

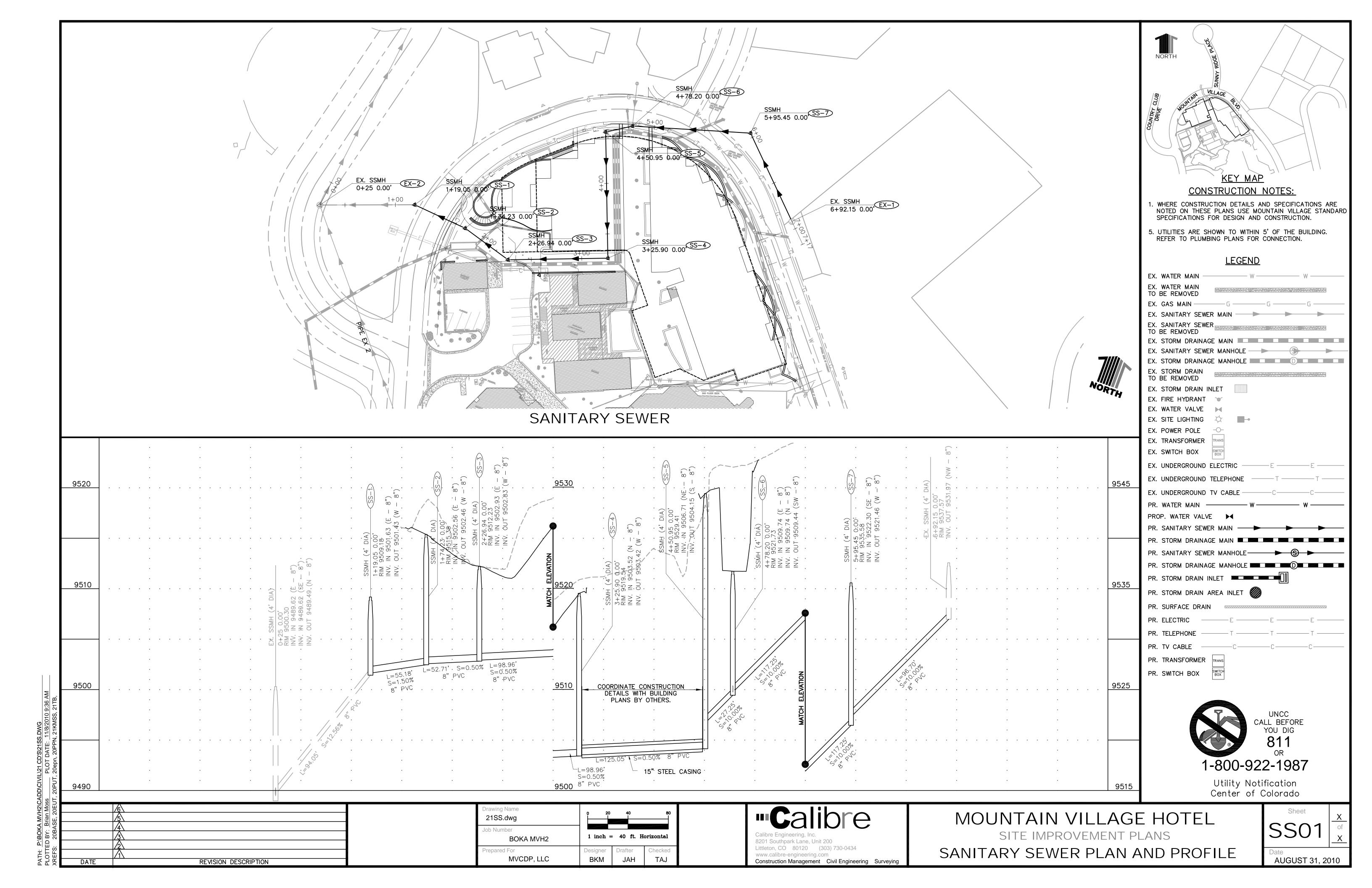


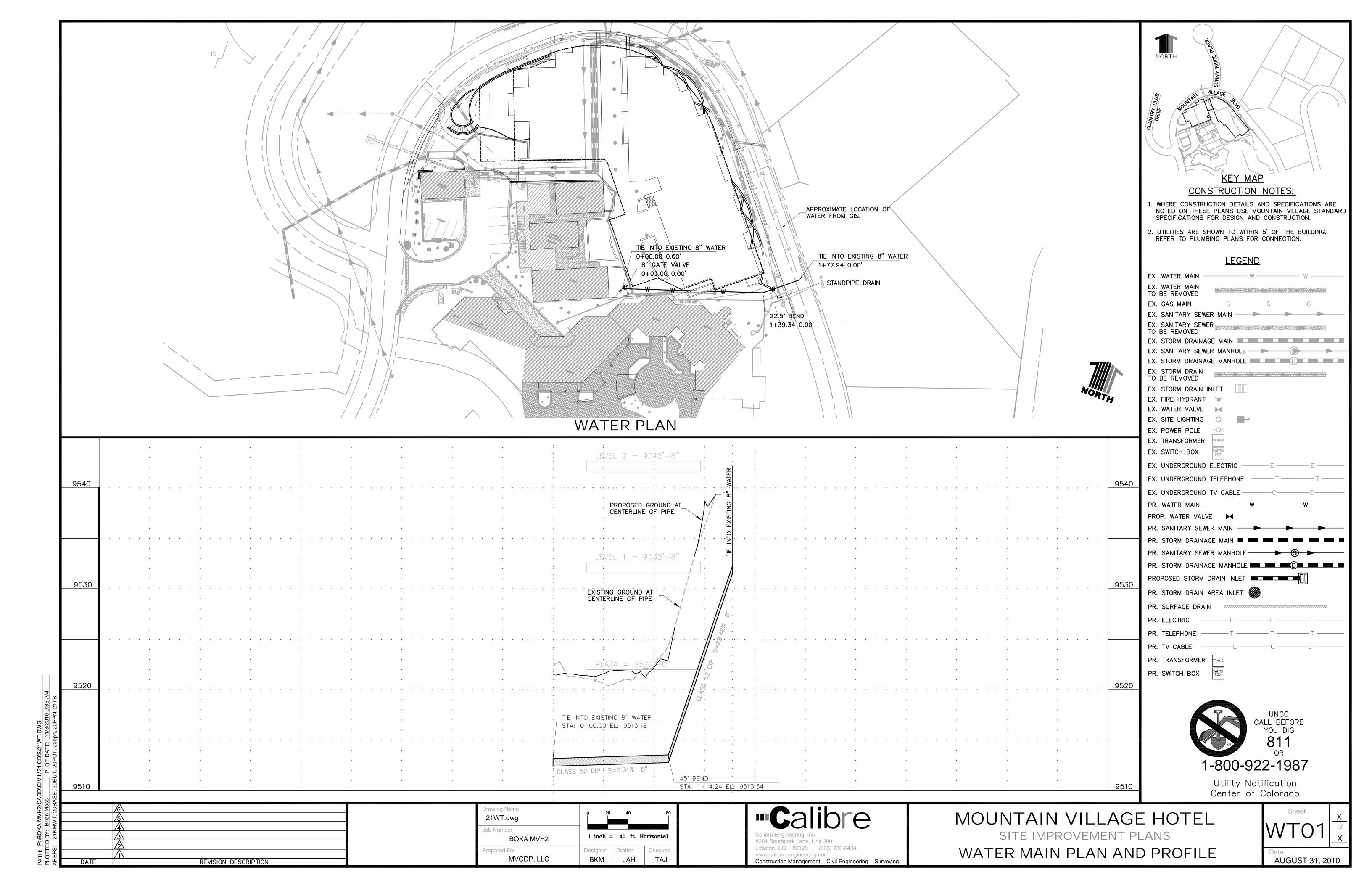


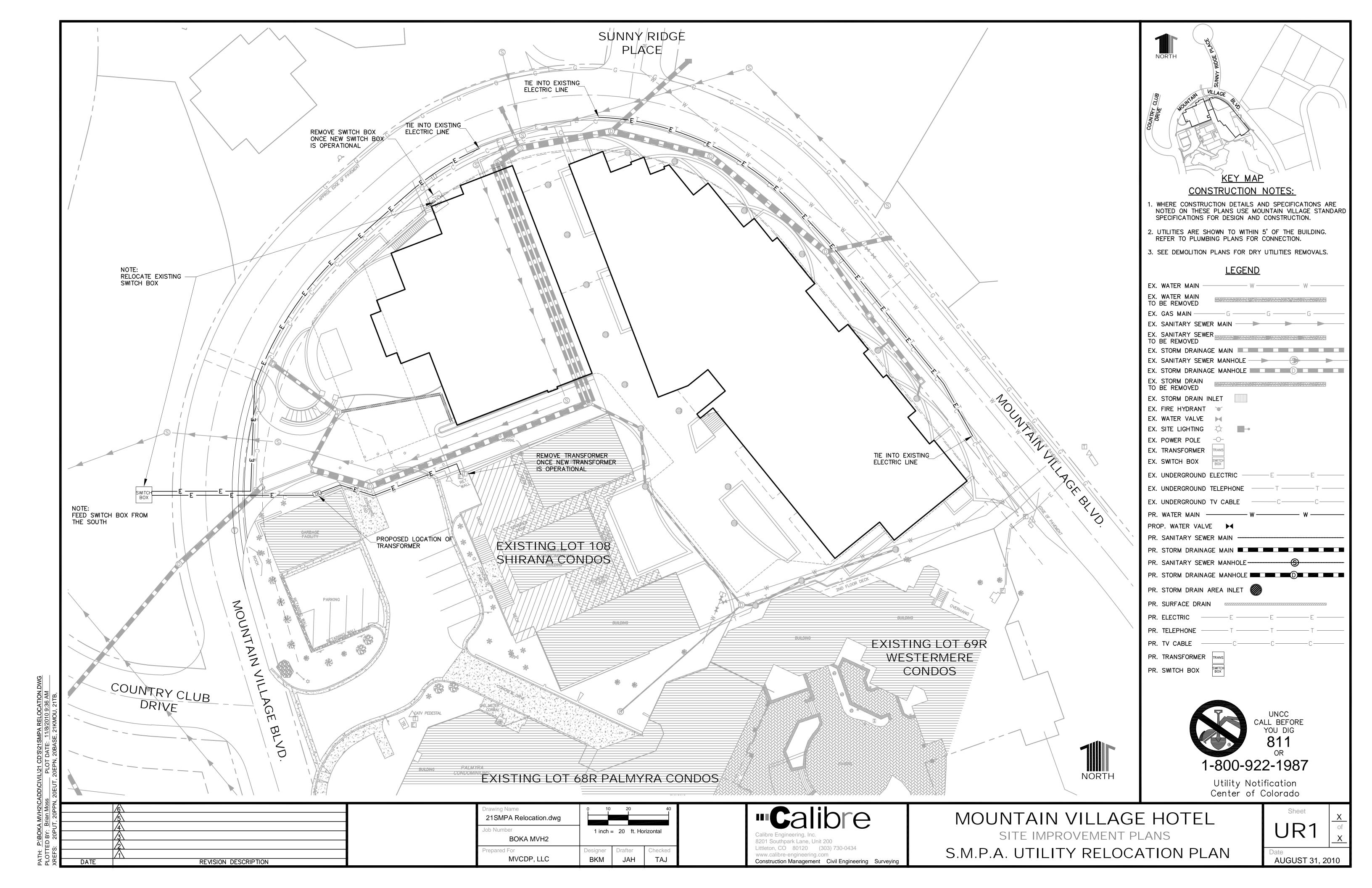


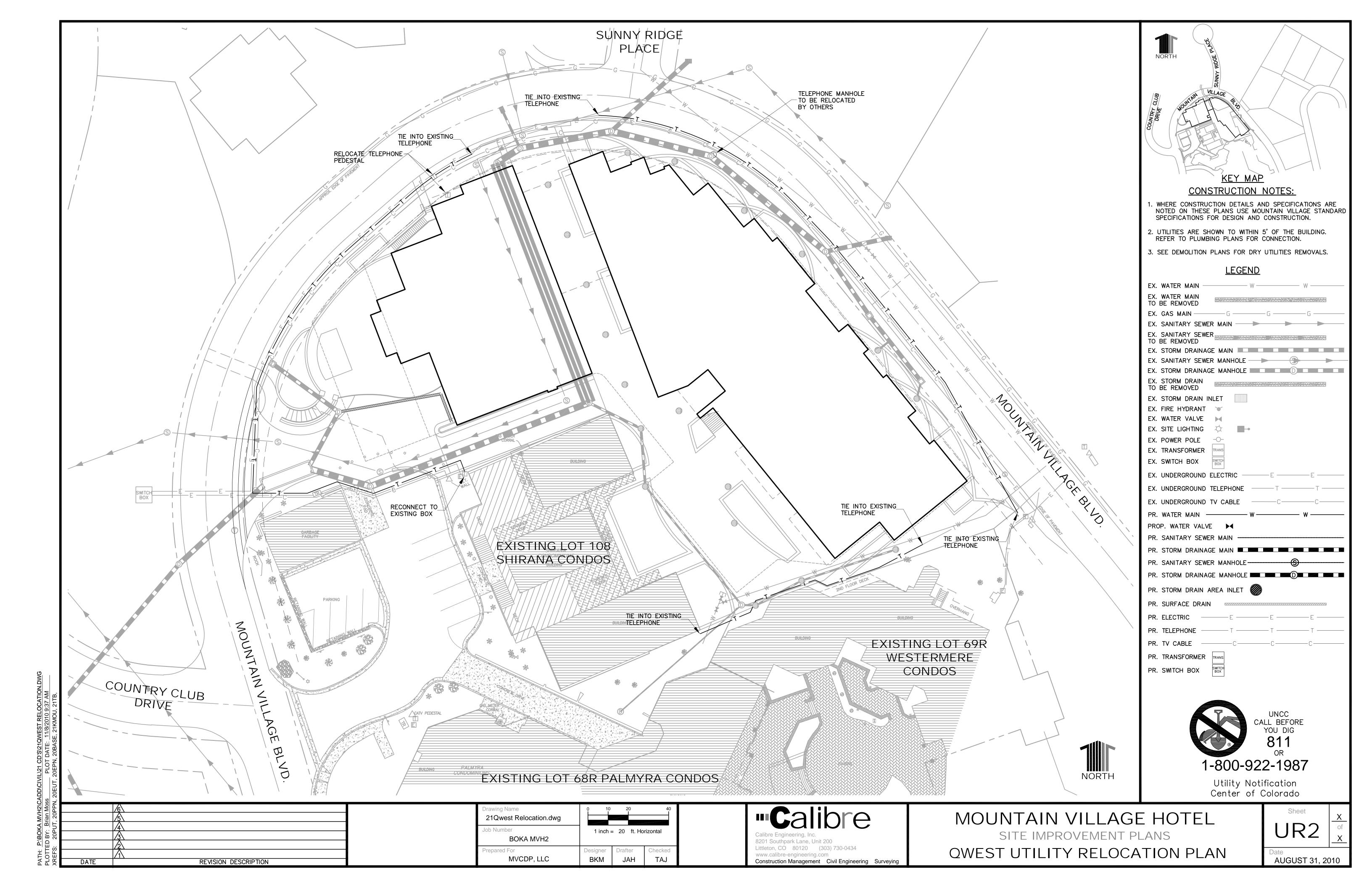


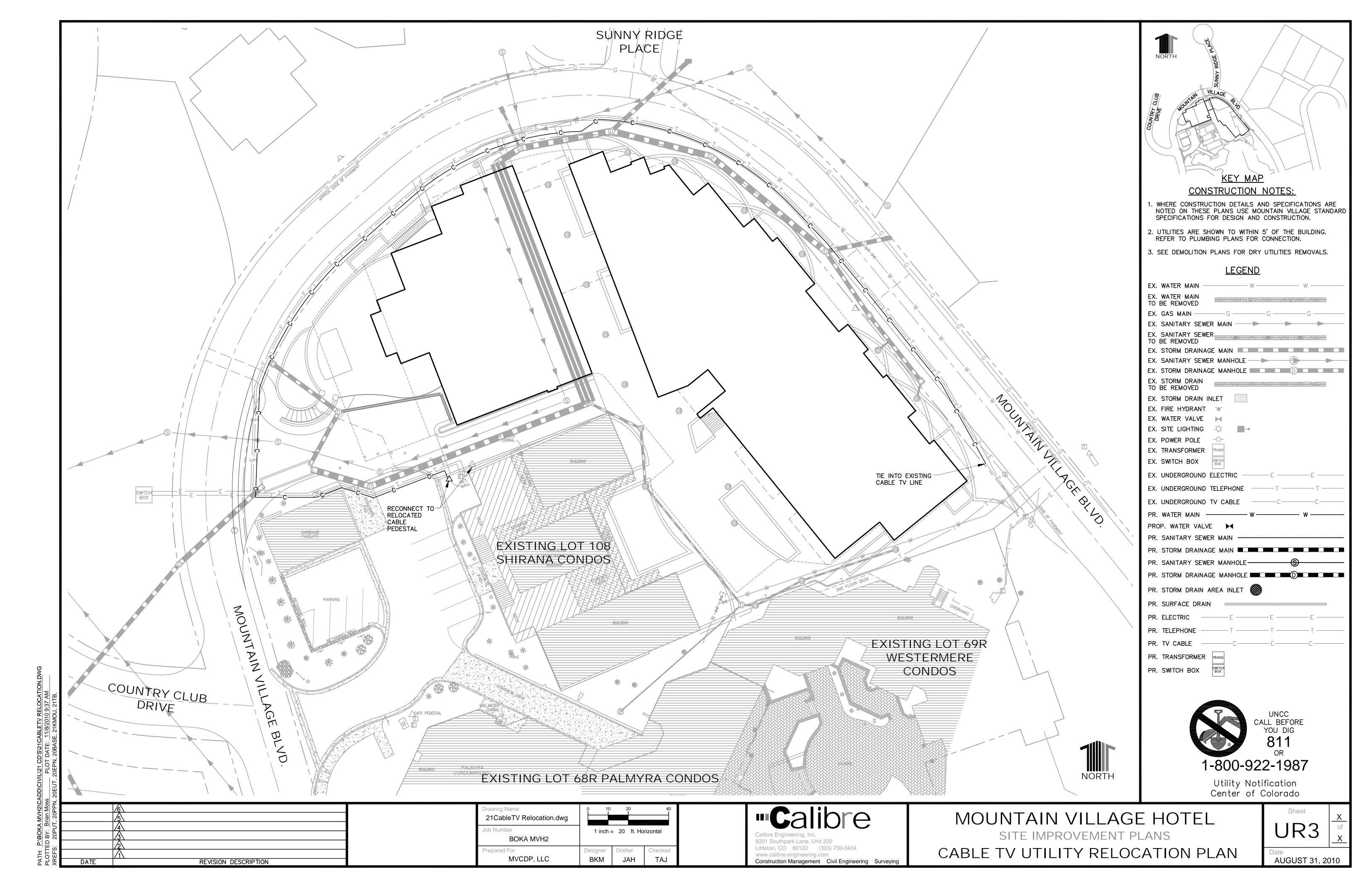


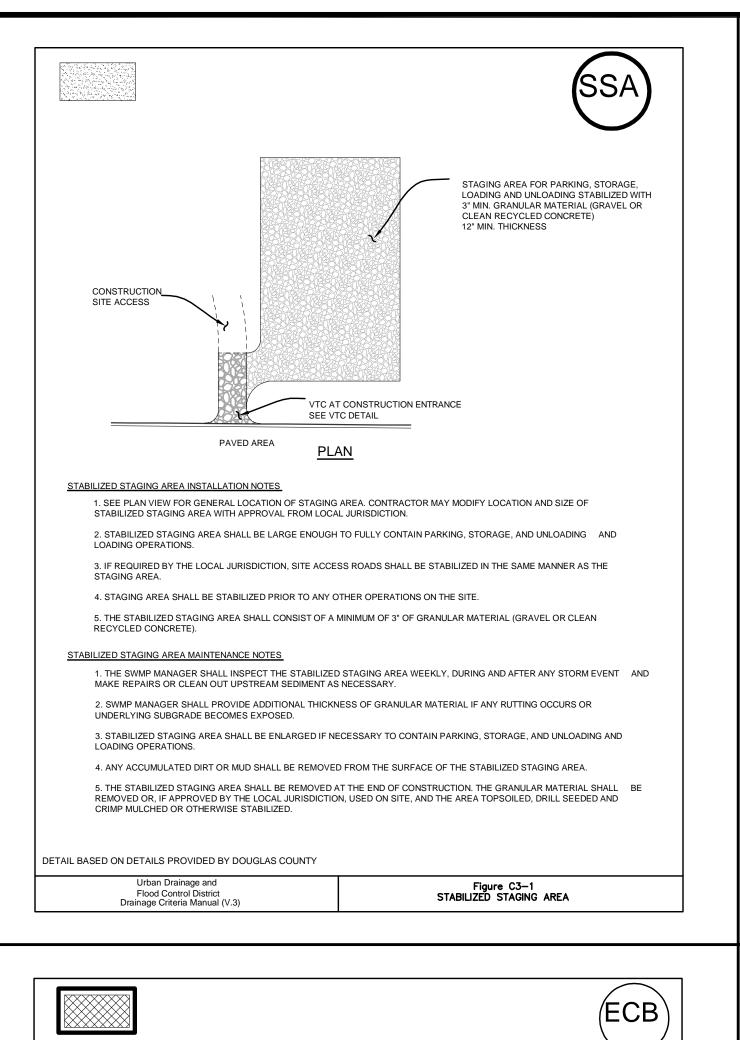


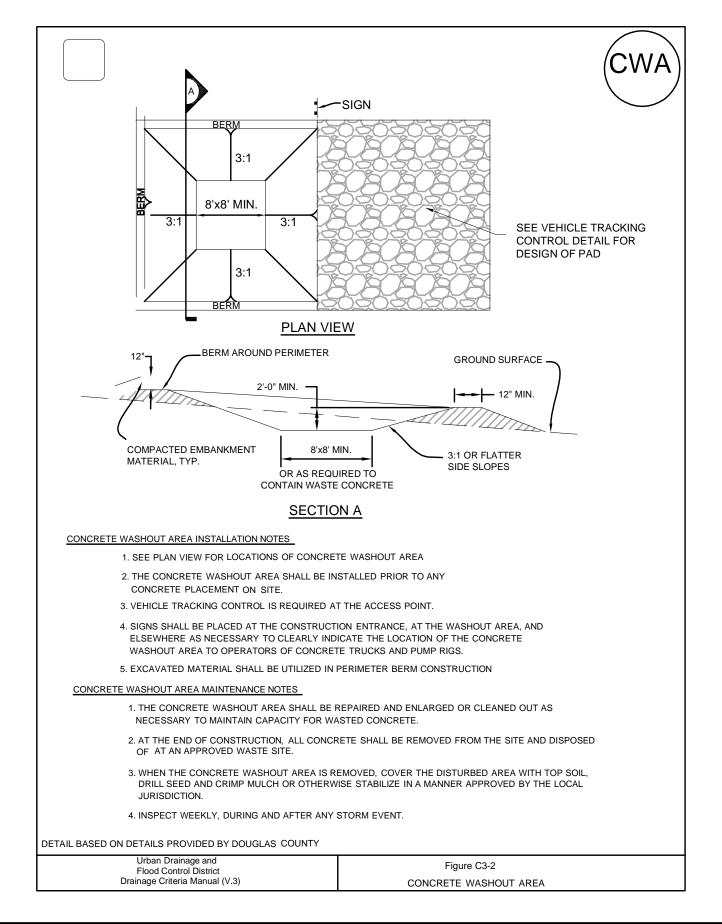


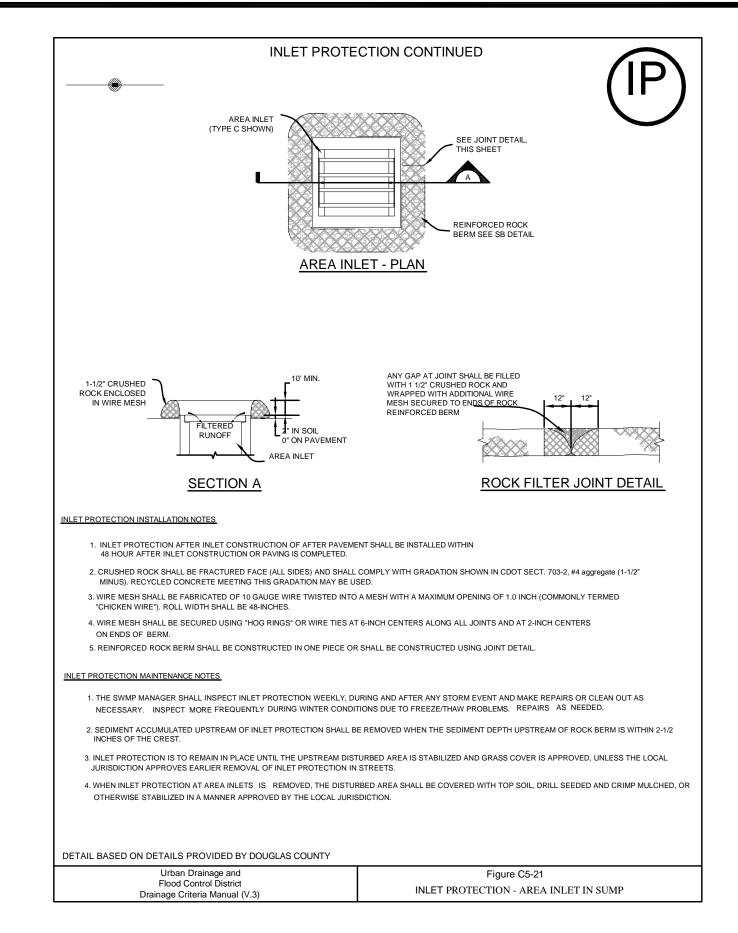


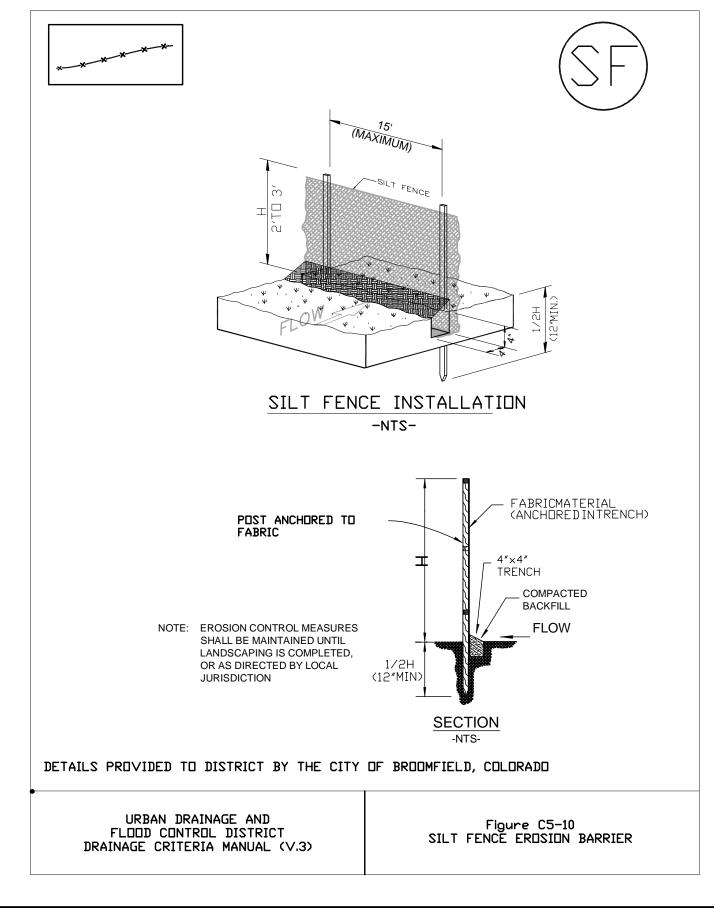


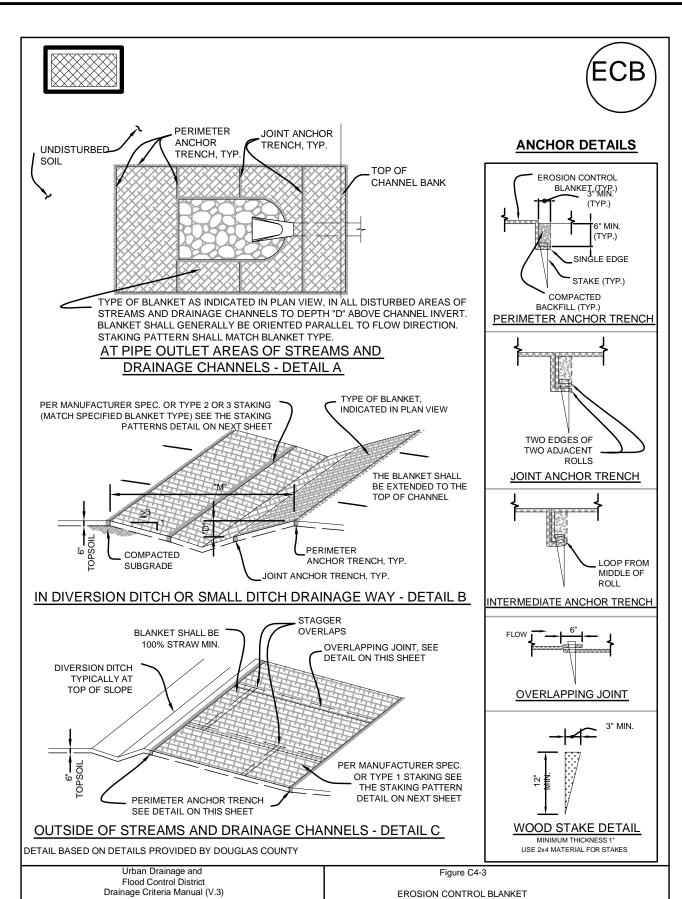


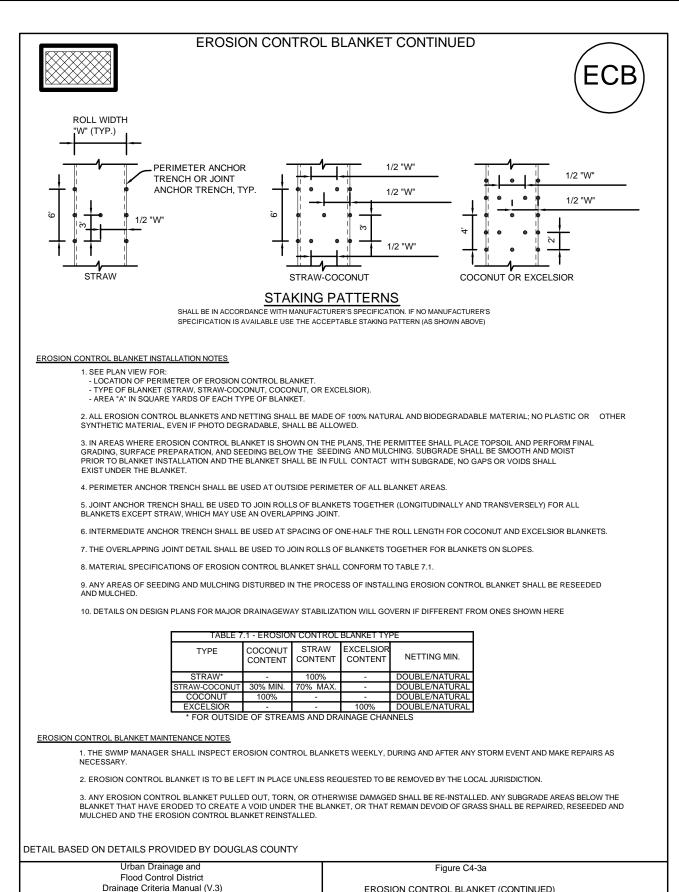


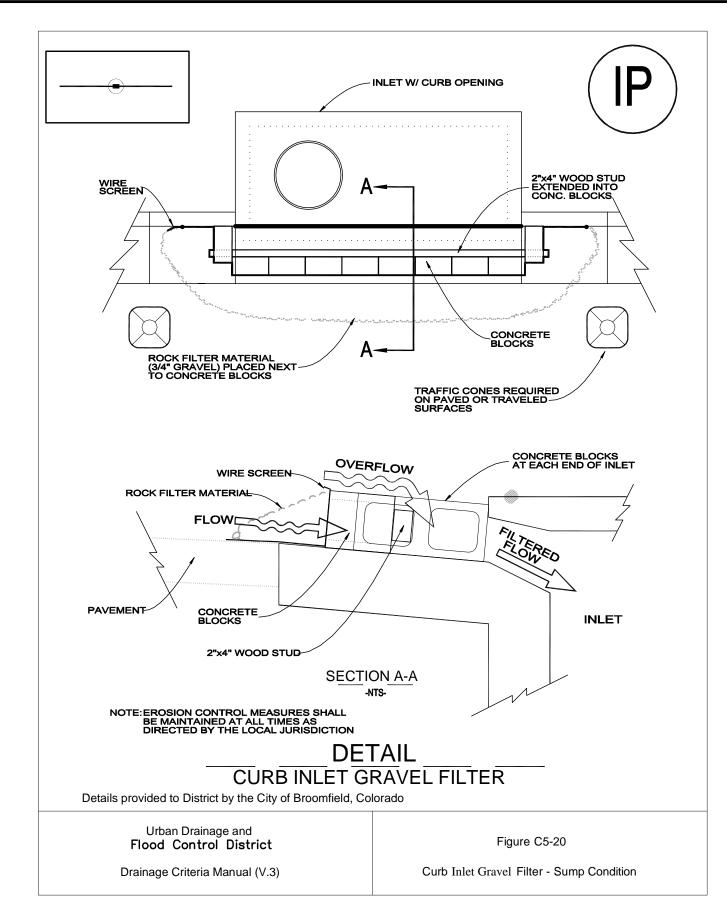


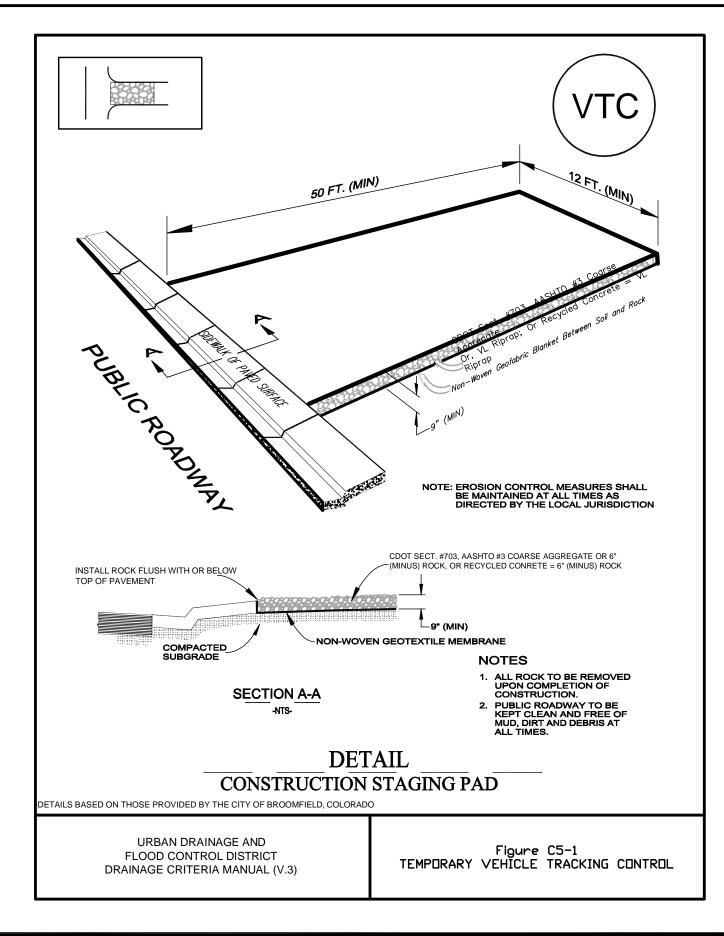


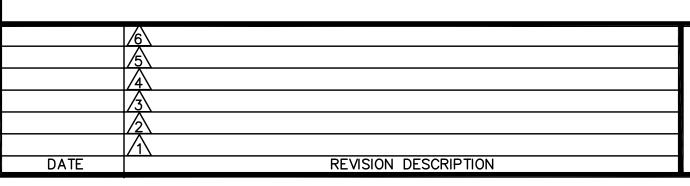










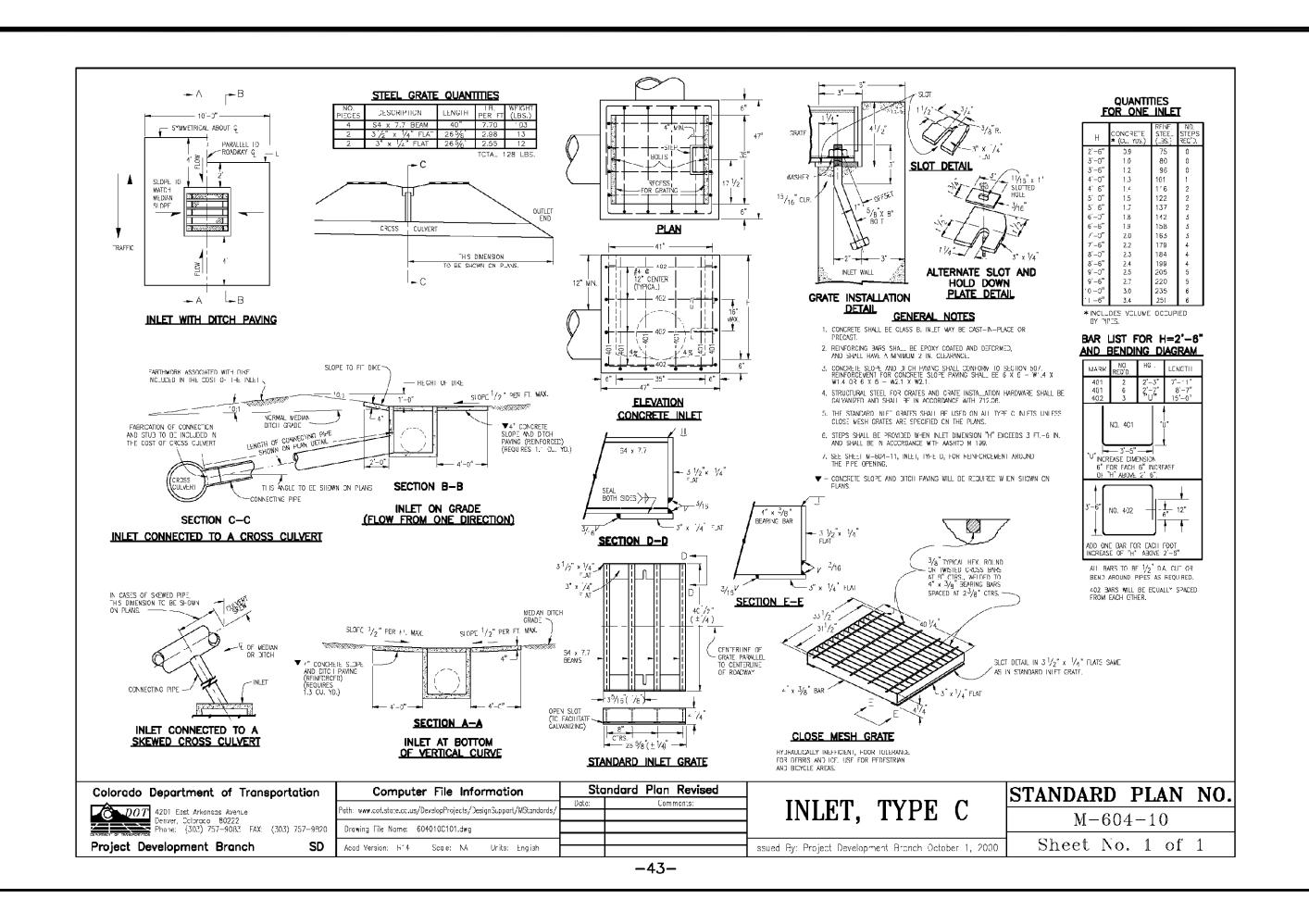


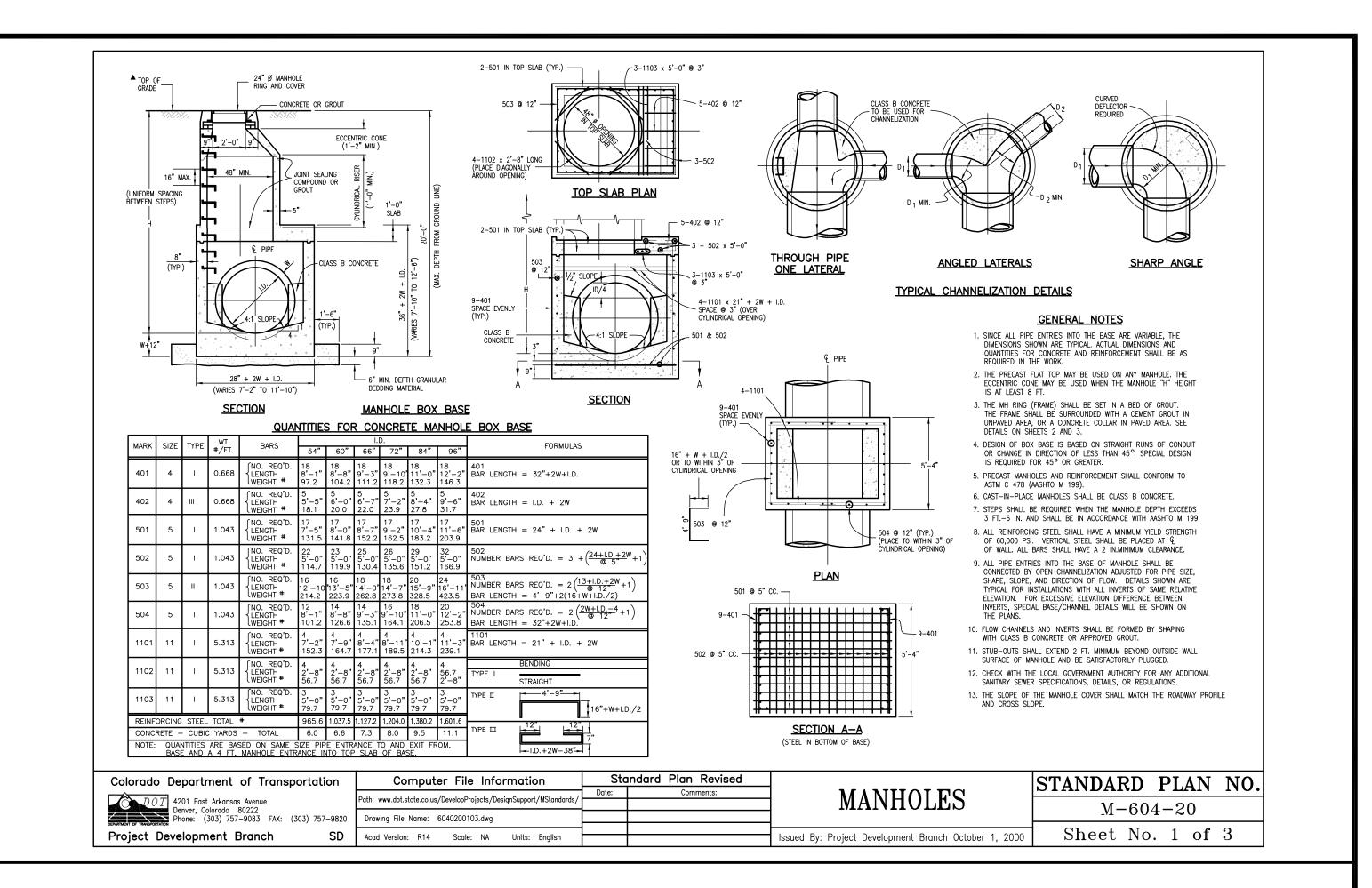


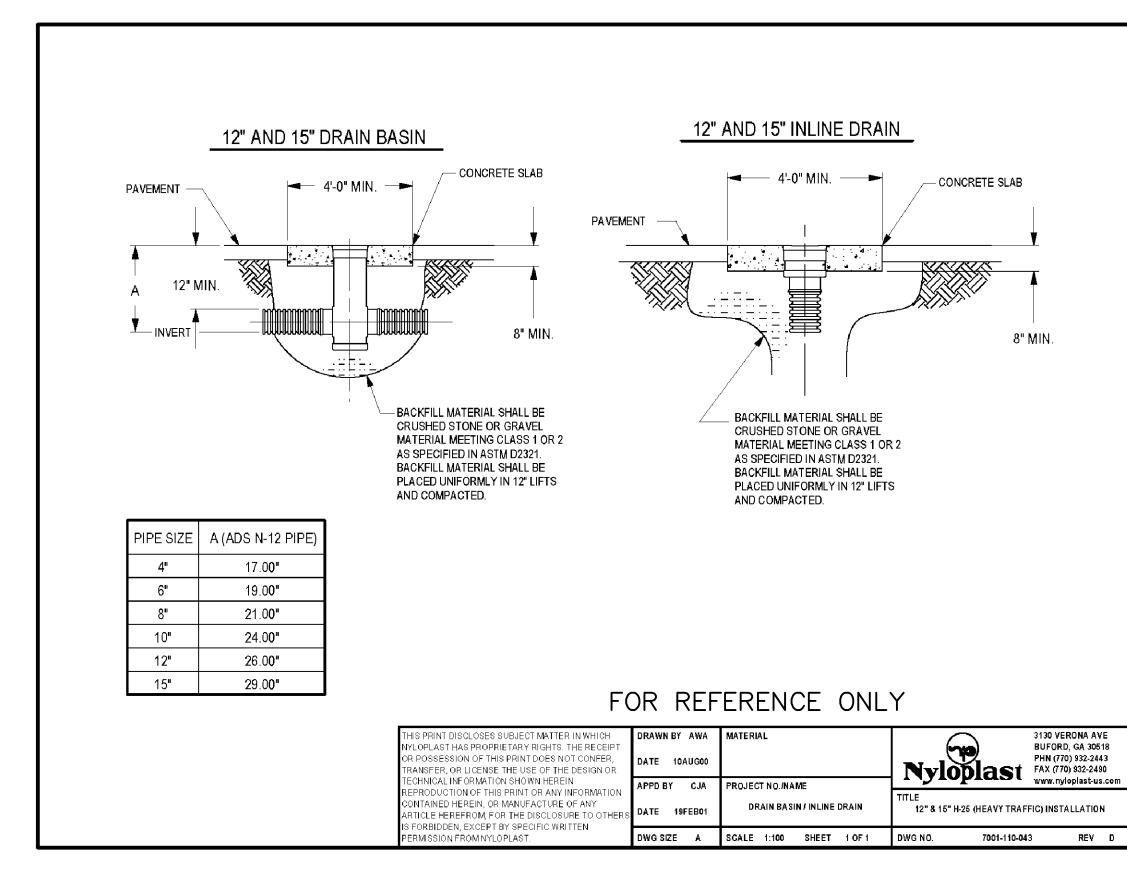
8201 Southpark Lane, Unit 200 Littleton, CO 80120 (303) 730-0434

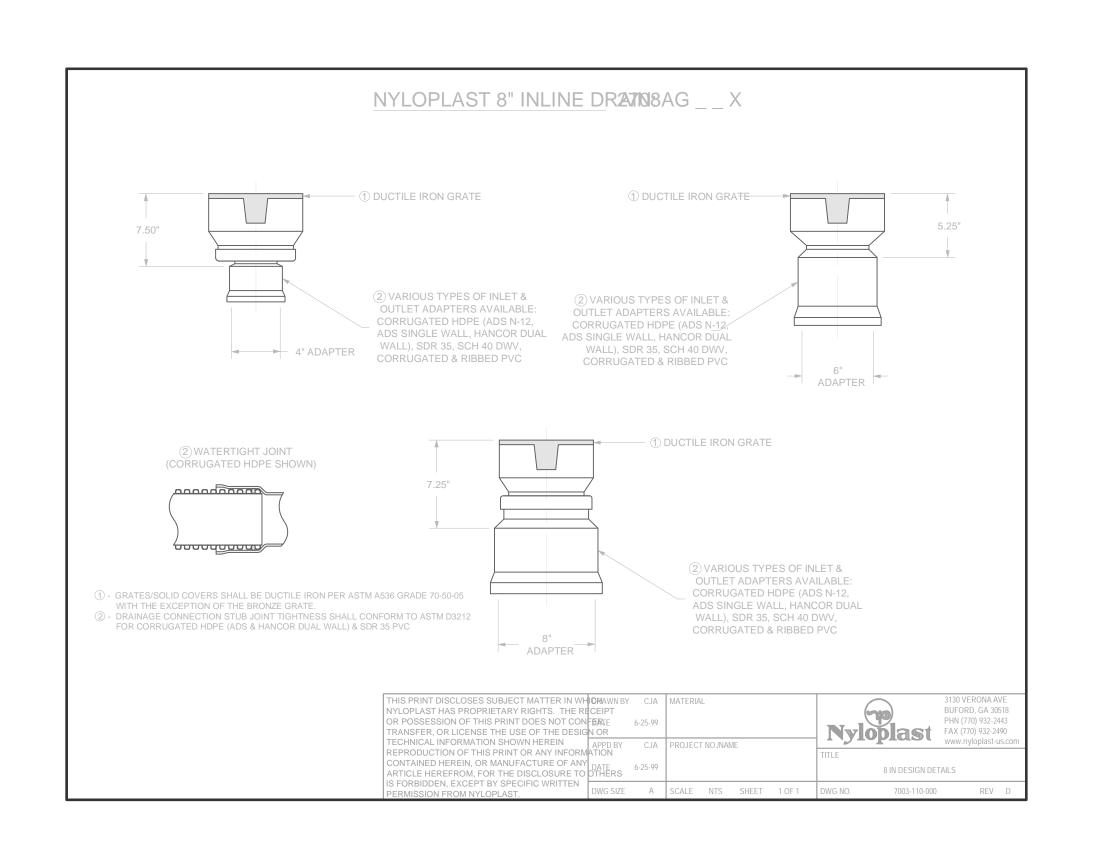
Construction Management Civil Engineering Surveying

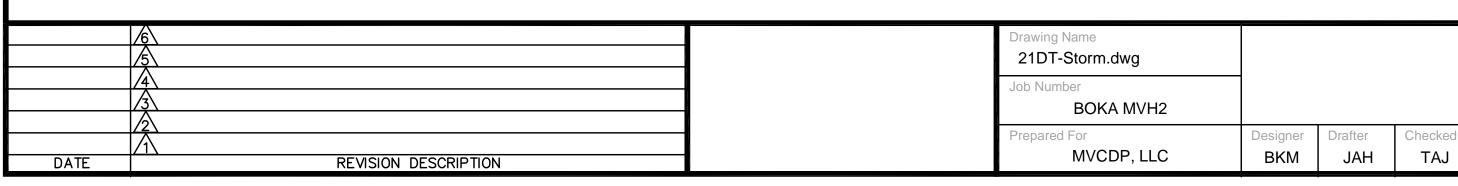
MOUNTAIN VILLAGE HOTEL SITE IMPROVEMENT PLANS DETAILS-GRADING AND EROSION CONTROL AUGUST 31, 2010







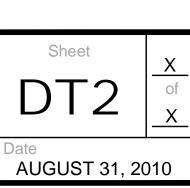


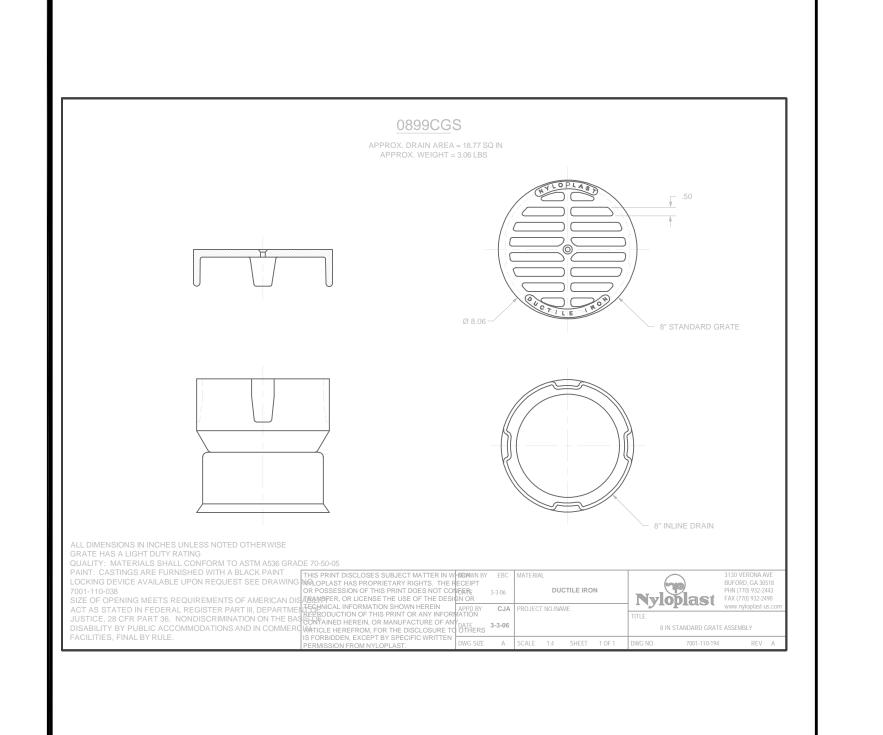


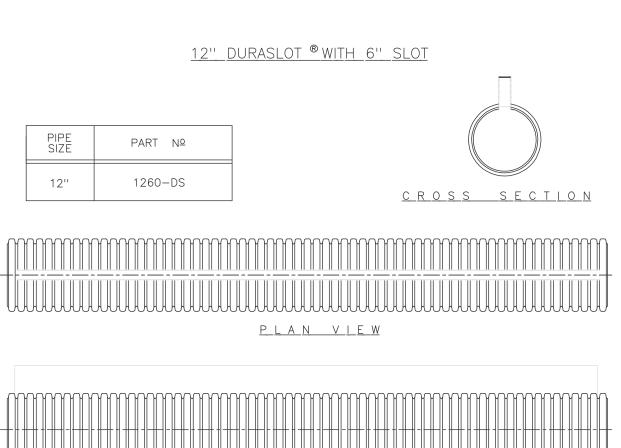


MOUNTAIN VILLAGE HOTEL
SITE IMPROVEMENT PLANS

SITE IMPROVEMENT PLANS
DETAILS - STORM DRAINAGE

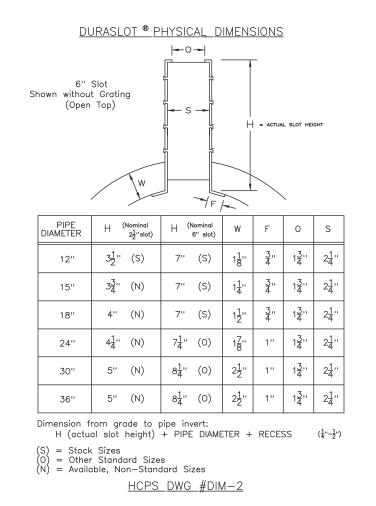


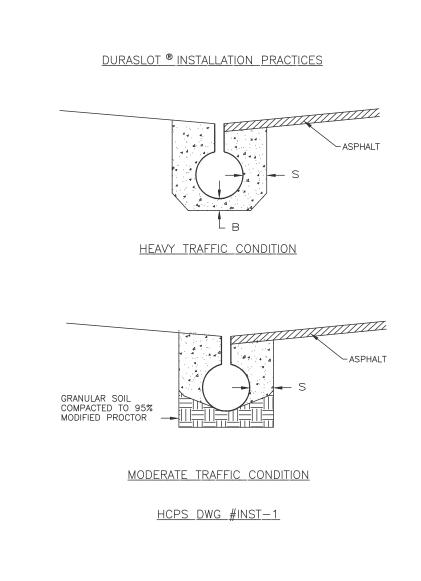


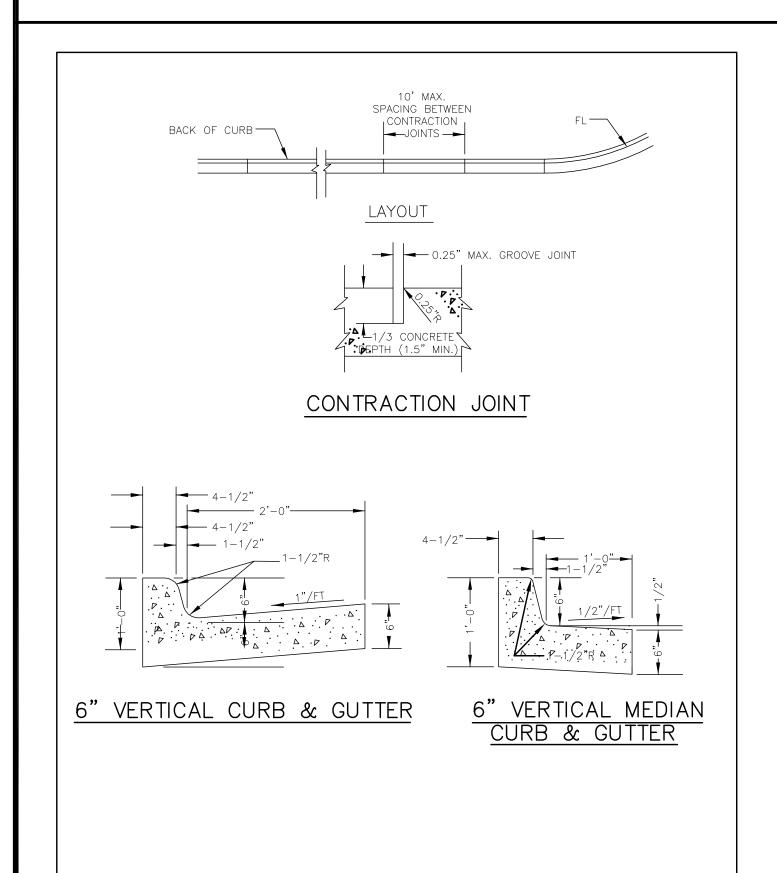


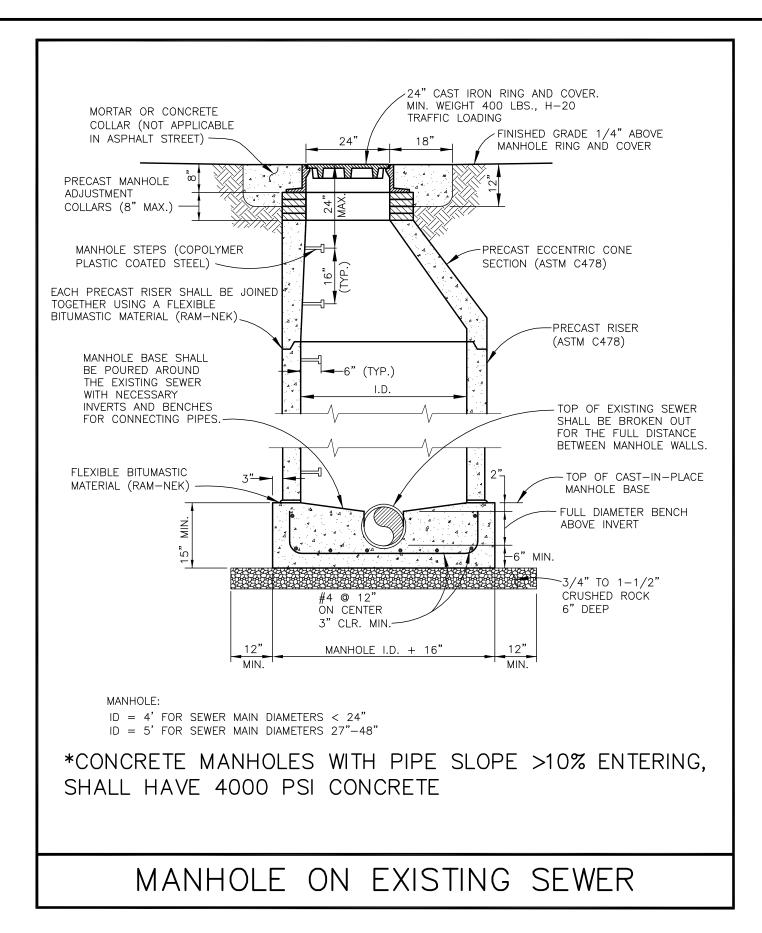
<u>ELEVATION</u>

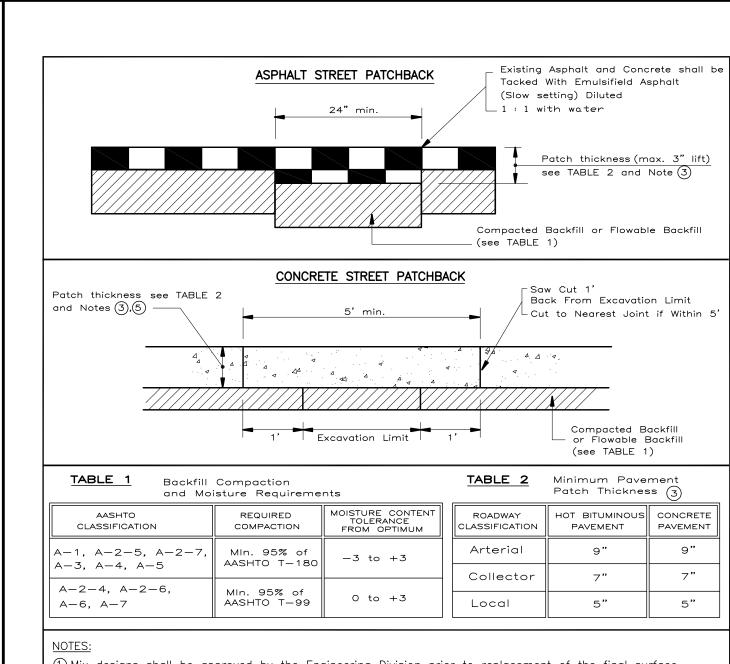
HCPS DWG #PI-7



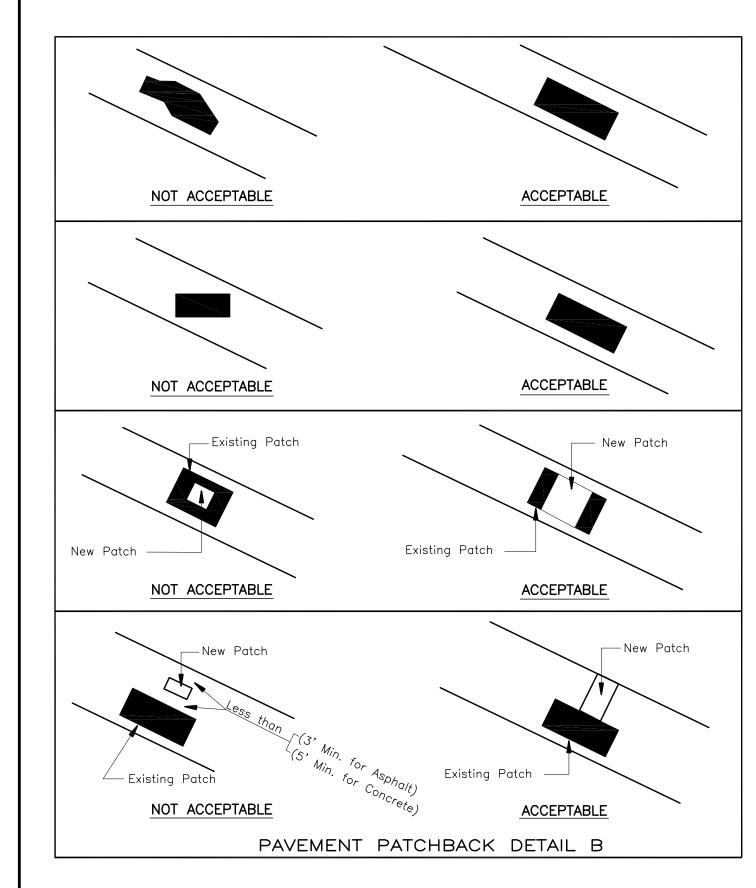


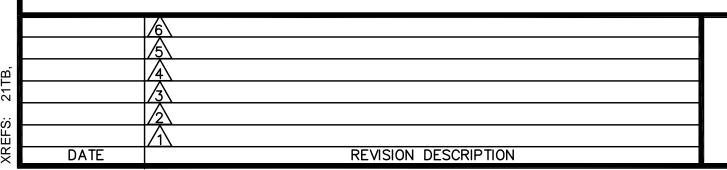






① Mix designs shall be approved by the Engineering Division prior to replacement of the final surface. The City may test the pavement during or after final placement to determine compliance with these specifications, and require removal and replacement of any pavement which does not meet specifications. 2) Cuts in asphalt streets that are over 8 feet in both dimensions shall require the use of a paving box. Cuts over 12 feet in width and 200 feet in length shall require the use of a self-propelled paver. B) Pavement Patch Thickness shall not be less than the existing section. A Refer to Pavement Utility Cut and Patch Specifications, Section III B. 5) Concrete Street Pavement to be Min. 7 sack mix, 5000 psi at 28 days. ⑥ All cuts to be squared off prior to restoration. Saw cut full depth for concrete. Saw cut or rotomill minimum of 6" for asphalt. Additional asphalt depth shall be cut by jackhammer or cutting wheel. 🕜 Arterial streets and major Collectors must be patched the same day the cut occurs, unless approved by the Engineering Division. PAVEMENT PATCHBACK DETAIL A





21DT-Storm.dwg Job Number **BOKA MVH2** MVCDP, LLC BKM JAH TAJ



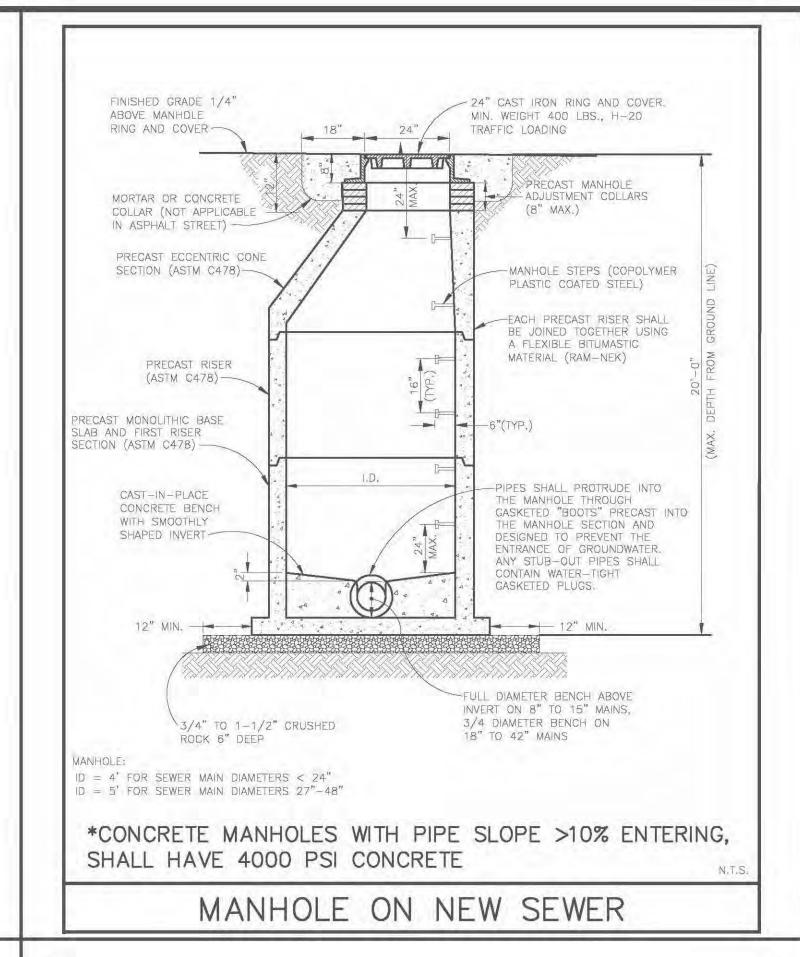
8201 Southpark Lane, Unit 200 Littleton, CO 80120 (303) 730-0434 MOUNTAIN VILLAGE HOTEL

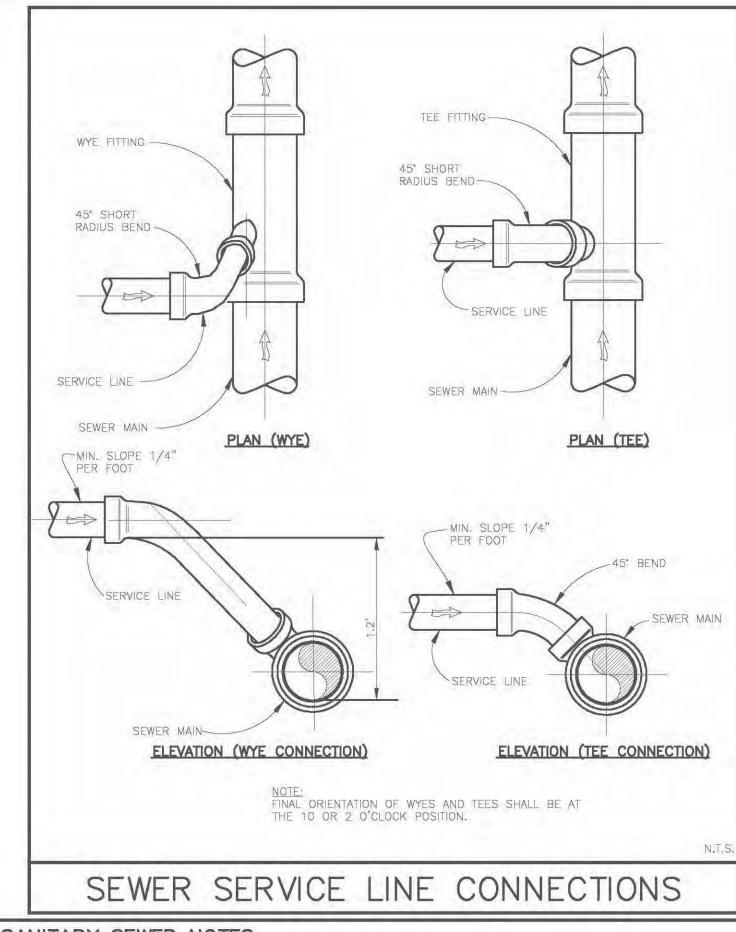
SITE IMPROVEMENT PLANS

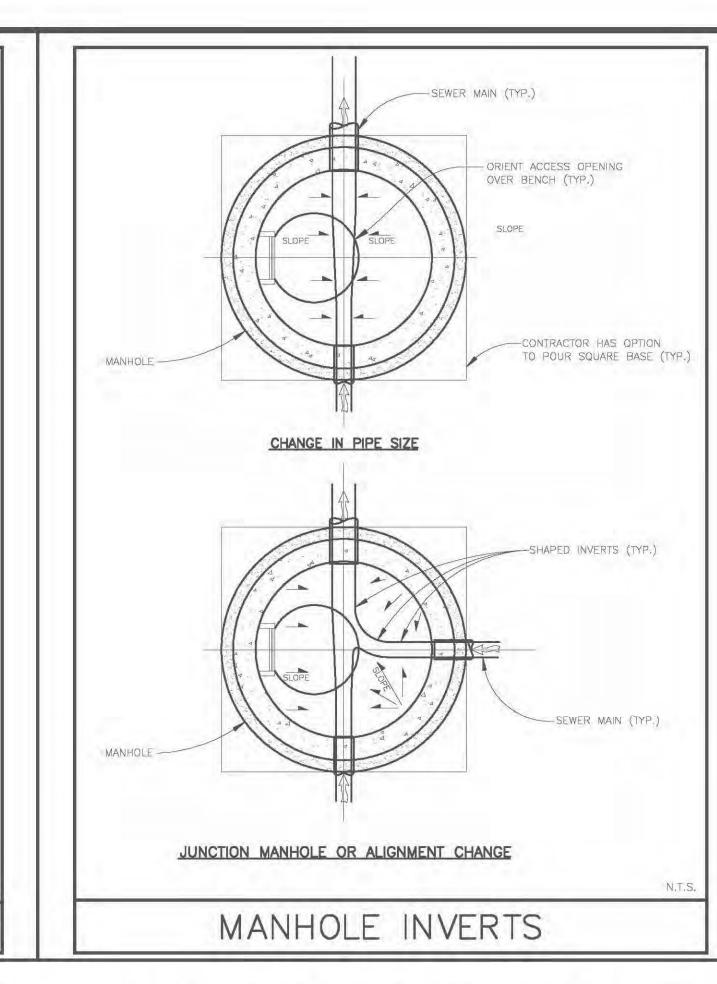
AUGUST 31, 2010

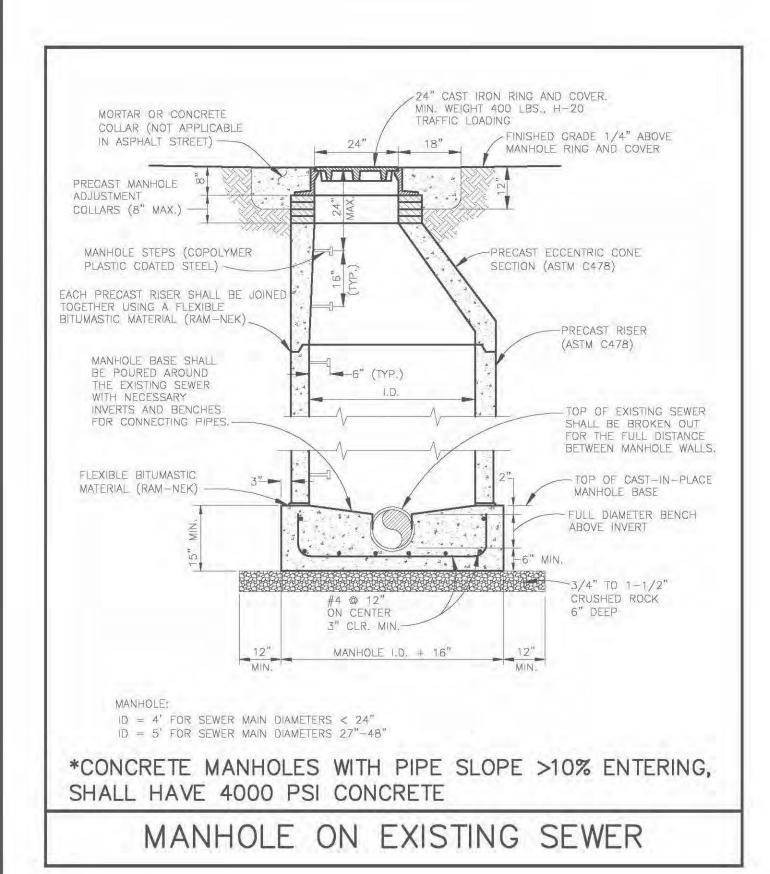
Construction Management Civil Engineering Surveying

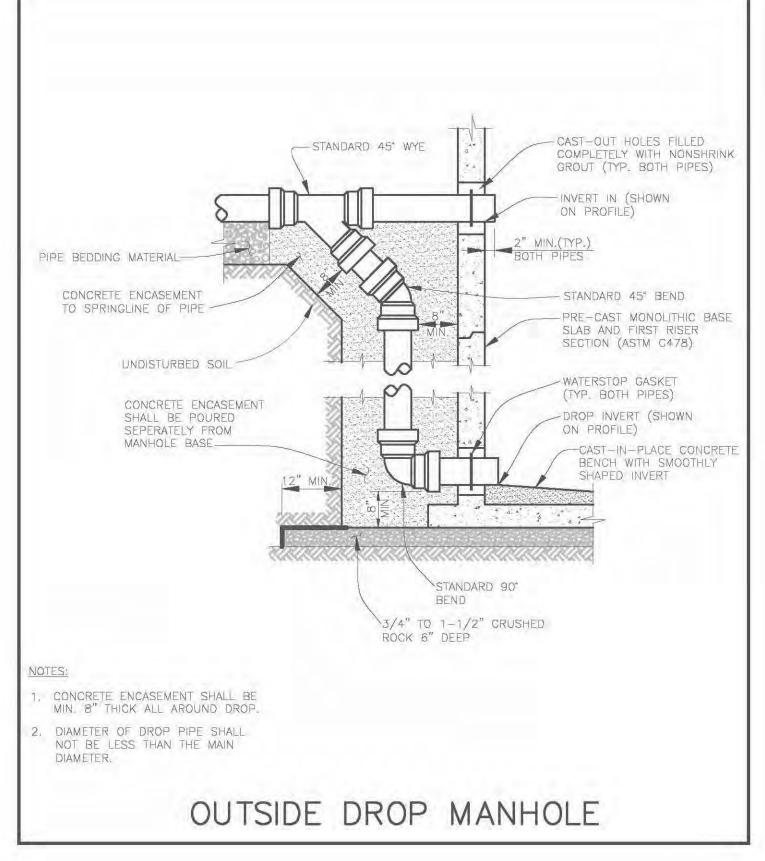
DETAILS - STORM DRAINAGE & ROADWAY











SANITARY SEWER NOTES:

- . ALL SANITARY SEWER LINES SHALL BE A MINIMUM OF AN 8 INCH PVC PIPE, ASTM D-3034 SDR 35. SANITARY SEWER LINE MATERIALS AND CONSTRUCTION SHALL BE IN CONFORMANCE WITH THE TOWN OF MOUNTAIN VILLAGE WATER AND SANITATION DISTRICT FOR DESIGN AND CONSTRUCTION OF PUBLIC IMPROVEMENTS.
- 2. ENGINEERS SHALL FURNISH THE TOWN WITH A SET OF PLANS MARKED "RECORD DRAWING" WITH LOCATIONS OF MANHOLES, DISTANCES AND GRADES BEFORE FINAL ACCEPTANCE BY THE TOWN.
- 3. PIPELINE CONTRACTOR SHALL USE PRECAST CONCRETE ADJUSTMENT RINGS (6-INCH MINIMUM, 12-INCH MAXIMUM) TO ADJUST THE MANHOLE FRAME TO THE REQUIRED FINAL GRADE, SUCH THAT THERE IS NO MORE THAN 18 INCHES FROM FINISHED GRADE TO TOP OF THE CONE SECTION. THE RIM SHALL BE LEFT 1/8-INCH TO 1/2-INCH BELOW FINISHED ASPHALT.
- 4. CONTRACTOR SHALL NOTIFY THE TOWN OF MOUNTAIN VILLAGE WATER & SANITATION DISTRICT ENGINEERING INSPECTOR 48 HOURS PRIOR TO THE START OF CONSTRUCTION.
- ALL SANITARY SEWER MAINS TESTING SHALL BE DONE IN ACCORDANCE WITH MOUNTAIN VILLAGE WATER & SANITATION DISTRICT.
- 6. MANHOLE CONES SHALL BE ALIGNED TO PROVIDE A MINIMUM OF THREE FEET CLEARANCE BETWEEN THE MANHOLE RING AND THE LIP OF GUTTER.
- 7. ALL WORK, INCLUDING CORRECTION WORK, SHALL BE INSPECTED BY A TOWN REPRESENTATIVE WHO SHALL HAVE THE AUTHORITY TO HALT CONSTRUCTION WHEN STANDARD CONSTRUCTION PRACTICES ARE NOT BEING ADHERED TO.
- 8. MAINTAIN A MINIMUM OF TEN FEET SEPARATION BETWEEN ALL SANITARY SEWER AND WATER MAINS AND SERVICES.
- 9. LOCATION OF EXISTING UTILITIES SHALL BE VERIFIED BY CONTRACTOR PRIOR TO START OF CONSTRUCTION.
- 10. ALL UTILITY LOCATIONS SHOWN ARE BASED ON MAPS PROVIDED BY THE APPROPRIATE UTILITY COMPANY, AND DALLAS RIDGE CONSTRUCTION PLANS IS TO BE CONSIDERED AN APPROXIMATE LOCATION ONLY. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FIELD VERIFY THE FIELD LOCATION OF ALL UTILITIES PUBLIC OR PRIVATE, WHETHER SHOWN ON THE PLANS OR NOT, PRIOR TO CONSTRUCTION. REPORT ANY DISCREPANCIES TO THE ENGINEER PRIOR TO CONSTRUCTION.
- 11. ALL SEWER SHALL HAVE CLASS "B" BEDDING UNLESS OTHERWISE SHOWN. BEDDING MATERIAL SHALL CONFORM TO ASTM C-33 OR D-448, GRADATION NO. 67.
- 12. MANHOLES, BARRELS AND CONES SHALL BE CONSTRUCTED OF PRECAST CONCRETE, CAST-IN-PLACE MANHOLES SHALL NOT BE ALLOWED.
- 13. PRECAST MANHOLES AND RISERS SHALL BE MANUFACTURED IN CONFORMITY WITH ASTM DESIGNATION C-478. ALL CONES SHALL BE ECCENTRIC, EXCEPT FOR MANHOLE DEPTHS SIX FEET OR LESS A FLAT TOP SECTION SHALL BE USED.
- 14. THE FLEXIBLE PLASTIC JOINT SEALING COMPOUND SHALL BE "RAMNEK," RUBBERNECK OR APPROVED EQUAL.
- 15. NO BACKFILL MATERIAL SHALL BE PLACED ABOVE THE SPRINGLINE OF THE PIPE UNTIL A TOWN REPRESENTATIVE HAS AUTHORIZED BACKFILLING. IT SHALL BE THE DUTY OF THE CONTRACTOR TO NOTIFY THE TOWN 48 HOURS IN ADVANCE OF PROPOSED BACKFILL OPERATIONS SO A CITY REPRESENTATIVE MAY INSPECT THE PIPE AND THE BEDDING PRIOR TO BACKFILLING.
- 16. ALL JOINTS BETWEEN BARREL SECTIONS SHALL BE GROUTED INSIDE AND OUTSIDE WITH A NON-SHRINK GROUT PRIOR TO BACKFILLING.
- 17. STUB SEWER SERVICE LINES AT 2.0% MINIMUM. ALL SEWER SERVICES ARE PRIVATE FROM THE MAIN TO THE BUILDING. CONTRACTOR TO FIELD COORDINATE SERVICES WITH WALL AND UTILITY CROSSINGS.
- 18. SEWER MAINS ABOVE OR LESS THAN 1.5 FEET BELOW A WATER MAIN SHALL BE ENCASED IN CONCRETE OR SHALL BE CONSTRUCTED WITH DUCTILE IRON PIPE FOR 10 FEET EITHER SIDE OF THE WATER MAIN WITH NO JOINTS IN BETWEEN.
- 19. ANY EQUIPMENT UTILIZING ELECTRICAL RECEPTACLES SHALL COMPLY WITH NEC (LATEST EDITION)
- 20. CONTRACTOR SHALL NOTIFY ENGINEER IF INVERT ELEVATIONS OF EXISTING UTILITIES DIFFER FROM WHAT IS SHOWN ON PLANS.
- 21. SEE MEP PLANS FOR SIZE OF SANITARY SEWER SERVICES
- 22. TOW=TOP OF WALL BOW=BOTTOM OF WALL TOP=TOP OF PIPE

| | /6\ | | |
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| BOKA MVH2 Prepared For MVCDP, LLC | Designer BKM | Drafter JAH | Checked TAJ |
|-------------------------------------|-----------------|----------------|-------------|
| Job Number | | | |
| Drawing Name 21DT-Sanitary.dwg | | | |

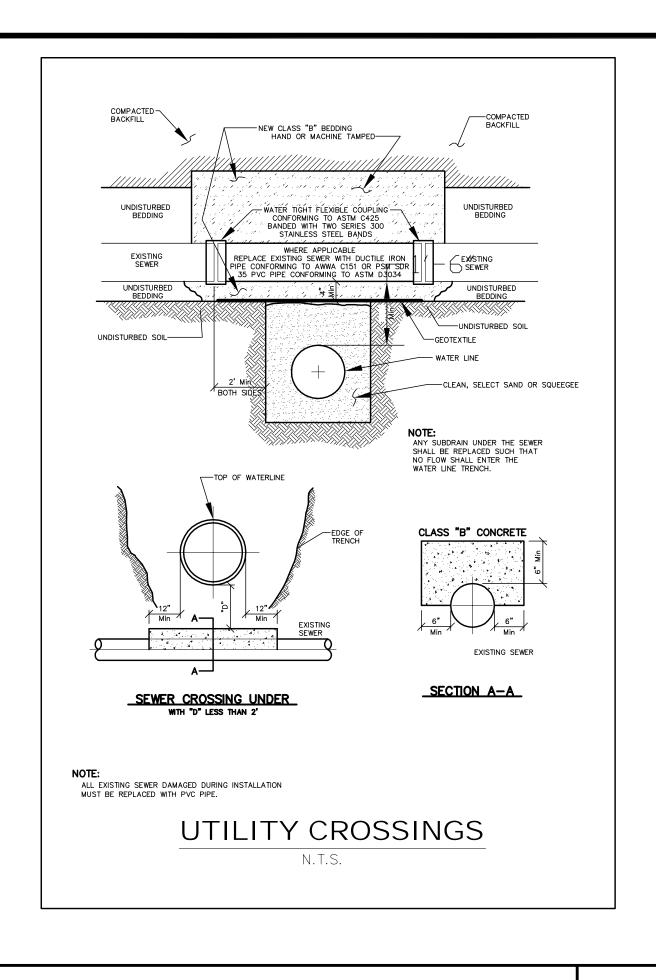


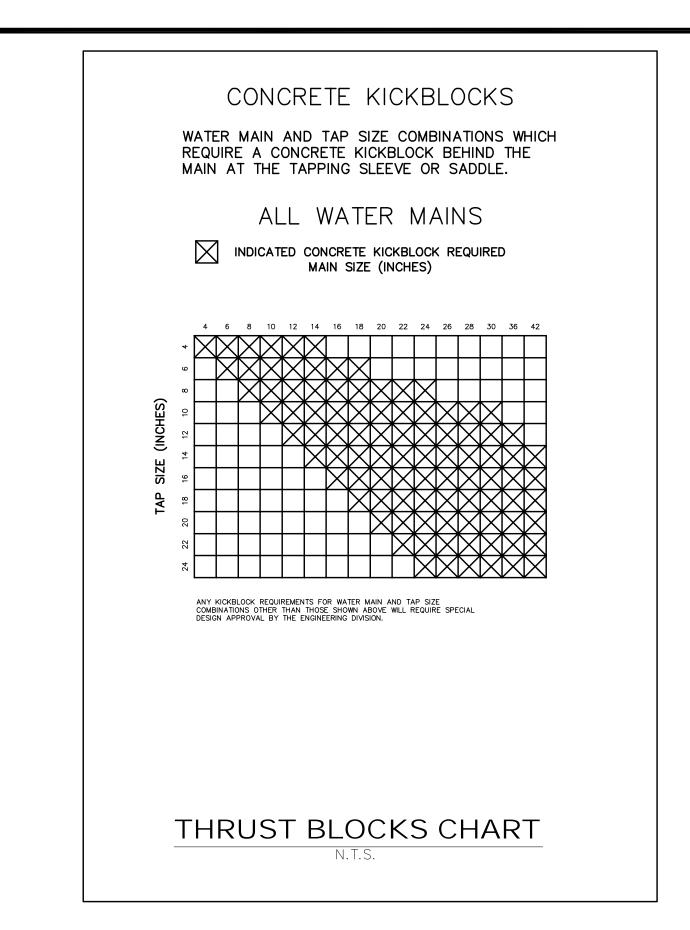
MOUNTAIN VILLAGE HOTEL SITE IMPROVEMENT PLANS

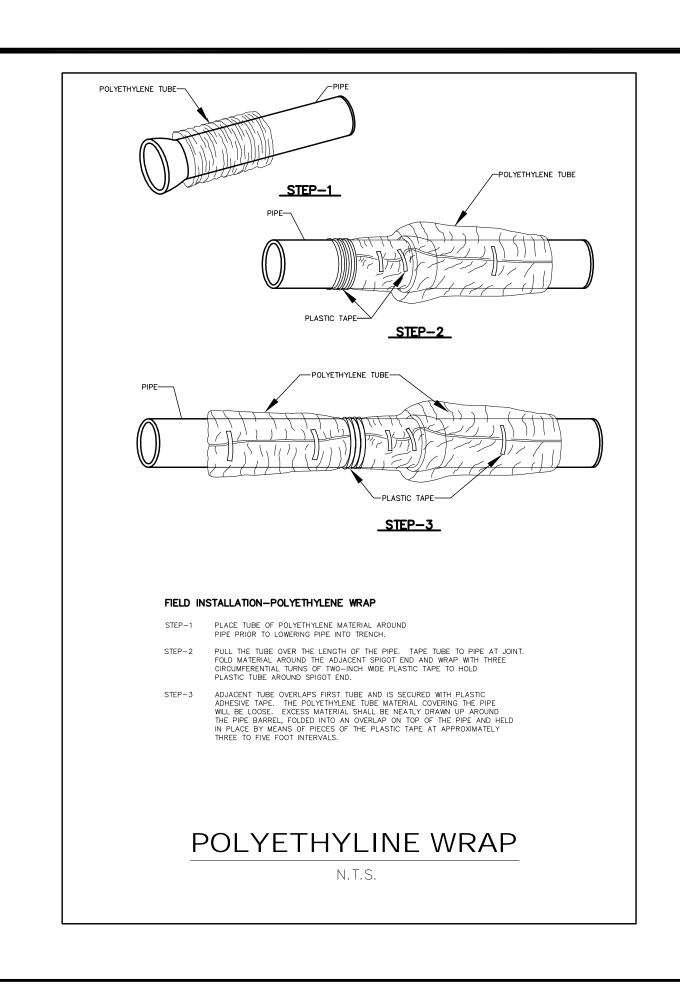
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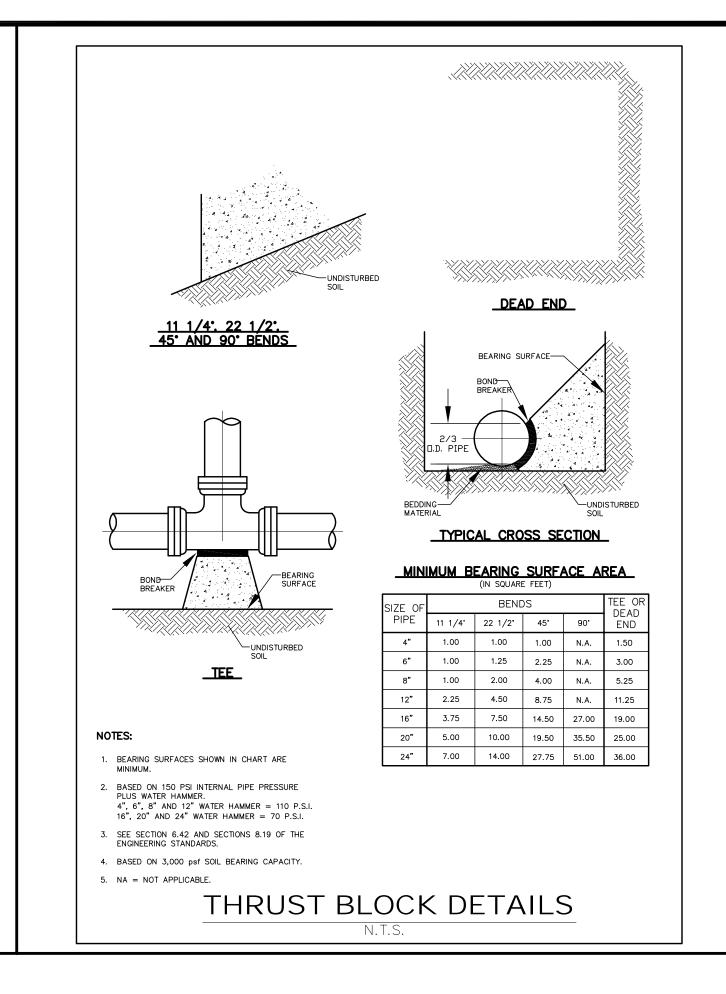
Construction Management Civil Engineering Surveying

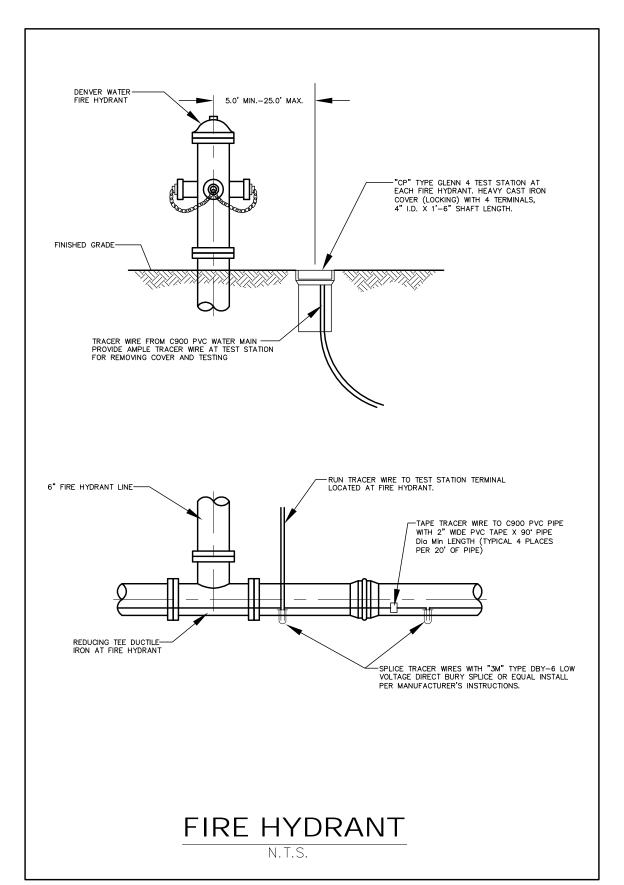
DETAILS - SANITARY SEWER

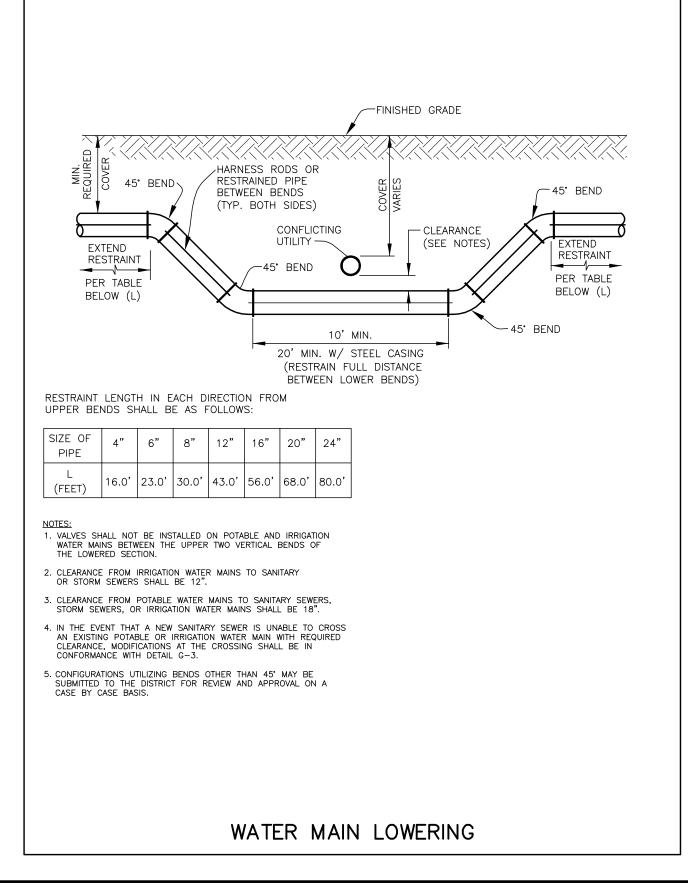












GENERAL NOTES - WATER

1. ALL PIPE LENGTHS ARE APPROXIMATE. ACTUAL PIPE LENGTHS AND BENDS TO BE DETERMINED BY CONTRACTOR.

2. SURFACED ACCESS ROADS SHALL BE CAPABLE OF WITHSTANDING THE IMPOSED LOADS OF FIRE APPARATUS AND ALL REQUIRED FIRE HYDRANTS SHALL BE INSTALLED AND SERVICEABLE PRIOR TO AND DURING CONSTRUCTION.

3. PRIOR TO INSTALLATION OF WATER MAINS, ROAD CONSTRUCTION MUST HAVE PROGRESSED TO AT LEAST THE "SUB-GRADE" STAGE. SUB-GRADE IS DEFINED AS AN ELEVATION OF NO MORE THAN SEVEN INCHES BELOW THE FINISHED STREET GRADE.

4. ALL WATER MAINS SHALL BE DUCTILE IRON PIPE (DIP) PRESSURE PIPE, MANUFACTURED IN ACCORDANCE WITH AWWA DUCTILE IRON STANDARD: AWWA/ANSI A21-51/C151, PRESSURE CLASS 350, THICKNESS CLASS 52. FIRE HYDRANT LATERALS TO BE DUCTILE IRON, CLASS 50 PIPE WITH POLY WRAP. PIPE JOINTS SHALL BE MADE USING AN INTEGRAL BELL WITH AN ELASTOMERIC GASKET PUSH ON TYPE JOINT. SOLVENT CEMENT JOINTS ARE STRICTLY PROHIBITED.

5. ALL WATER PIPE SHALL HAVE A TRACER WIRE. INSTALL 12 GAUGE SINGLE STRAND COPPER TRACER WIRE TO PIPE WITH 2-INCH WIDE PVC TAPE. SPLICING OF TRACER WIRE SHALL BE PER MANUFACTURE'S RECOMMENDATION. THE TRACER WIRE SHALL RUN TO A TEST STATION OR VALVE BOX LOCATED NEXT TO A FIRE HYDRANT.

6. ALL DOMESTIC WATER MAINS SHALL HAVE 8 FEET MINIMUM FROM FINISHED GRADE TO T.O.P.

7. WHEN IT IS NECESSARY TO LOWER OR RAISE WATER LINES AT UTILITY CROSSINGS, A MINIMUM OF 1.50 FEET SHALL BE MAINTAINED VERTICALLY BETWEEN OUTSIDE OF PIPES.

8. IN ALL CASES MAINTAIN A 10 FOOT HORIZONTAL DISTANCE BETWEEN WATER AND SEWER

9. DUCTILE AND PVC WATER MAINS SHALL MAINTAIN AT LEAST THE MINIMUM RADIUS RECOMMENDED BY THE MANUFACTURER.

10. ALL CONCRETE SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI.

11. ALL REINFORCING STEEL SHALL CONFORM TO THE MINIMUM REQUIREMENTS OF ASTM A-15, INTERMEDIATE GRADE.

12. ALL BACKFILL SHALL BE COMPACTED TO 95 % STANDARD PROCTOR DENSITY

13. THE BEDDING MATERIAL SHALL BE CLEAN WELL GRADED SQUEEGEE SAND AND SHALL CONFORM TO THE FOLLOWING LIMITS WHEN TESTED BY MEANS OF LABORATORY SIEVES:

SIEVE SIZE TOTAL PERCENT PASSING BY WEIGHT 3/8 INCH 100 NO. 200 0 - 5

14. THE WATER SYSTEM SHALL BE TESTED IN ACCORDANCE WITH THE TOWN STANDARDS AND SPECIFICATIONS.

15. CHLORINATION AND FLUSHING: ALL WATER MAINS SHALL BE CHLORINATED IN ACCORDANCE WITH AWWA C651, "DISINFECTING WATER MAINS". THE PREFERRED METHOD IS TO USE SUFFICIENT CHLORINE TABLETS TO PRODUCE A 25 MG/L SOLUTION. THESE TABLETS SHOULD BE ADHERED TO THE TOP OF THE PIPE SECTION WITH PERMATEX CLEAR RTV. CHLORINATION OF 16-INCH AND LARGER PIPE REQUIRES A CHLORINE SLURRY. THE CHLORINATION OF ANY PIPELINE SHALL BE DONE PRIOR TO HYDROSTATIC TESTING.

16. HYDROSTATIC TESTING: ALL WATER MAINS SHALL BE TESTED IN ACCORDANCE WITH AWWA C-600 SECTION 4 "HYDROSTATIC TESTING". ALL PIPE SHALL BE FIELD PRESSURE TESTED TO A MINIMUM OF 150 PSI OR DOUBLE THE OPERATING PRESSURE. WHICHEVER IS GREATER. ALLOWABLE LEAKAGE FOR EACH SECTION OF PIPE RETWEEN LINE VALVES SHALL NOT EXCEED THE LEAKAGE RATE SET FORTH.

| SECTION OF PIPE BETWEEN | LINE VALVES SHALL NOT | EXCEED THE LEA |
|-------------------------|-----------------------|-----------------|
| PIPE SIZE | ALLOWABLE LEAKAG | E PER 1000 FEET |
| INSIDE DIAMETER | GALLONS F | PER HOUR |
| | DIP | PVC |
| 4" | 0.37 | 0.33 |
| 6 " | 0.55 | 0.50 |
| 8" | 0.74 | 0.66 |
| 12" | 1.10 | 1.00 |

17. THE CONTRACTOR SHALL BE RESPONSIBLE FOR REMOVING AND REPLACING ANY EXISTING SIGN, STRUCTURES, FENCES, ETC., ENCOUNTERED ON THE JOB AND RESTORING THEM TO THEIR ORIGINAL CONDITION.

18. ALL SERVICES 2" AND SMALLER.

A. METER PIT AND VAULT LIDS MUST HAVE A 2-INCH DIAMETER HOLE FOR ERT.

19. INDIVIDUAL SERVICE LINE PRV'S ARE REQUIRED. (USE THIS NOTE ONLY WHEN AREA PRESSURE EXCEEDS 80 PSI.)

20. INSTALL 6 INCH WIDE DETECTABLE ALUMINUM FOIL PLASTIC BACKED TAPE INDICATING FOR BURIED WATER LINE BELOW AND INSTALLED, 12 INCHES TO 18 INCHES BELOW SURFACE GRADE. TAPE MUST BE BLUE IN COLOR AND BE MANUFACTURED BY THORTEC OR EQUAL.

21. ALL 90° BENDS SHALL BE RODDED, EXCEPT FOR PVC PIPE.

22. TOWN PERSONNEL ARE NOT RESPONSIBLE FOR WORK SITE SAFETY OR COMPLIANCE/ENFORCEMENT OF SAFETY REGULATIONS AND STANDARDS ESTABLISHED BY OTHER AGENCIES. ALL SAFETY COMPLIANCE/ENFORCEMENT AT THE WORK SITE SHALL BE THE CONTRACTOR'S RESPONSIBILITY.

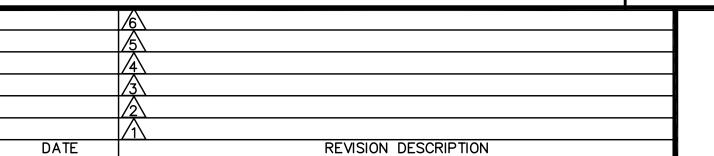
23. THE WATER QUALITY CONTROL DIVISION OF THE COLORADO DEPARTMENT OF PUBLIC HEALTH AND ENVIRONMENT (CDPHE) REQUIRES ALL WATER LINE CONTRACTORS TO POSSESS A CURRENT DISCHARGE PERMIT FOR DISCHARGES OF CHLORINATED AND PROCESS WATERS ASSOCIATED WITH THE INSTALLATION OF NEW MAINS ON CONDUITS. CONTACT CDPHE WATER QUALITY CONTROL DIVISION AT (303) 692-3539 FOR INFORMATION ON OBTAINING THE REQUIRED PERMIT.

24. ENGINEERS SHALL FURNISH THE TOWN WITH A SET OF PLANS MARKED "RECORD DRAWING" WITH LOCATIONS OF MANHOLES, DISTANCES AND GRADES BEFORE FINAL ACCEPTANCE BY THE TOWN.

25. CONTRACTOR SHALL NOTIFY THE TOWN OF MOUNTAIN VILLAGE WATER & SANITATION DISTRICT ENGINEERING INSPECTOR 48 HOURS PRIOR TO THE START OF CONSTRUCTION.

26. ALL WORK, INCLUDING CORRECTION WORK, SHALL BE INSPECTED BY A TOWN REPRESENTATIVE WHO SHALL HAVE THE AUTHORITY TO HALT CONSTRUCTION WHEN STANDARD CONSTRUCTION PRACTICES ARE NOT BEING ADHERED TO.

27. LOCATION OF EXISTING UTILITIES SHALL BE VERIFIED BY CONTRACTOR PRIOR TO START OF CONSTRUCTION.



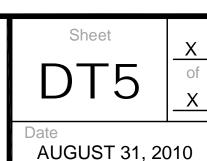
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| Job Number BOKA MVH2 | | | |
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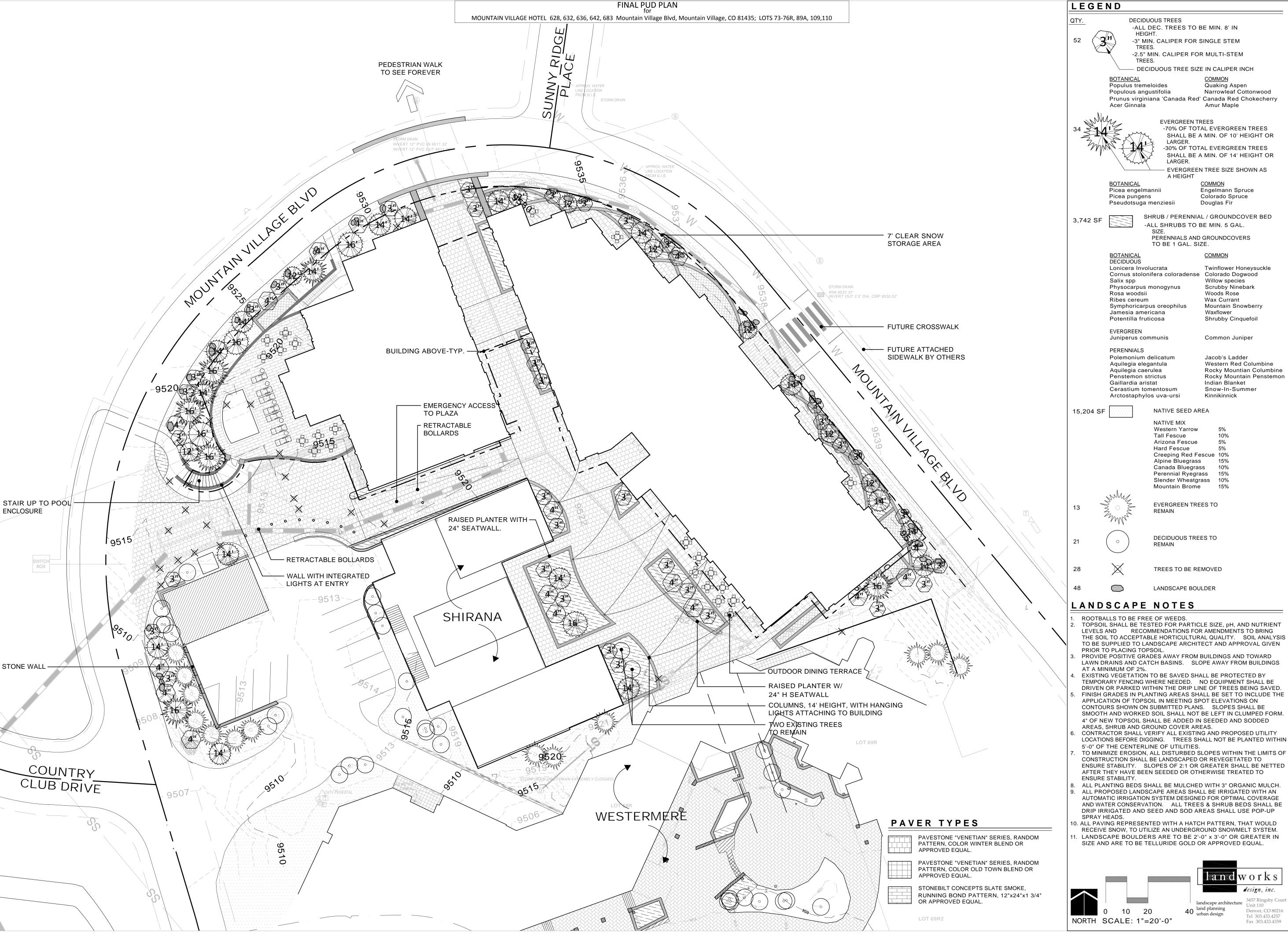


MOUNTAIN VILLAGE HOTEL

SITE IMPROVEMENT PLANS

DETAILS - WATER





BO KA Powell

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FINAL PUD PLAN ISSUE

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project
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628, 632, 636, 642, 683
Mountain Village Blvd,
Mountain Village Blvd,
Mountain Village, CO 81435
LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT 05-3BR-1, TOWN OF MOUNTAIN
VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF
LOTS 73 AND 76, TOWN OF MOUNTAIN VILLAGE AND REPLAT
OF TRACT 05-3BR, TOWN OF MOINTAIN VILLAGE AND REPLAT
COCONDING TO THE PLAT RECORDED JANUARY 25, 2007 IN
PLAT BOOK 1 AT PAGE 3807 AT RECEPTION NO. 389801,
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COUNTY OF SAN MIGUEL, STATE OF COLORADO.

LOT 109, REPLAT NO. 3, TELLURIDE MOUNTAIN VILLAGE,
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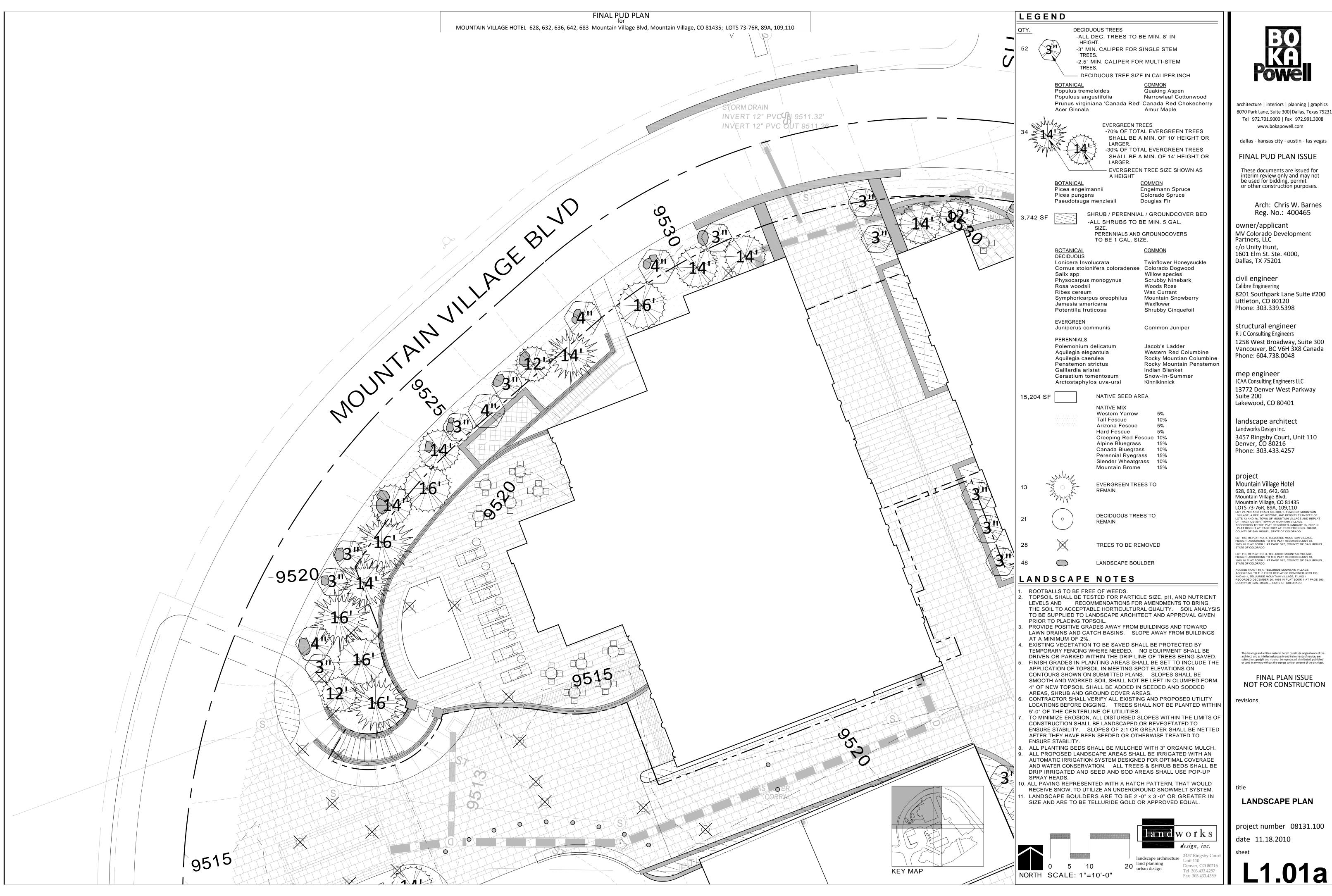
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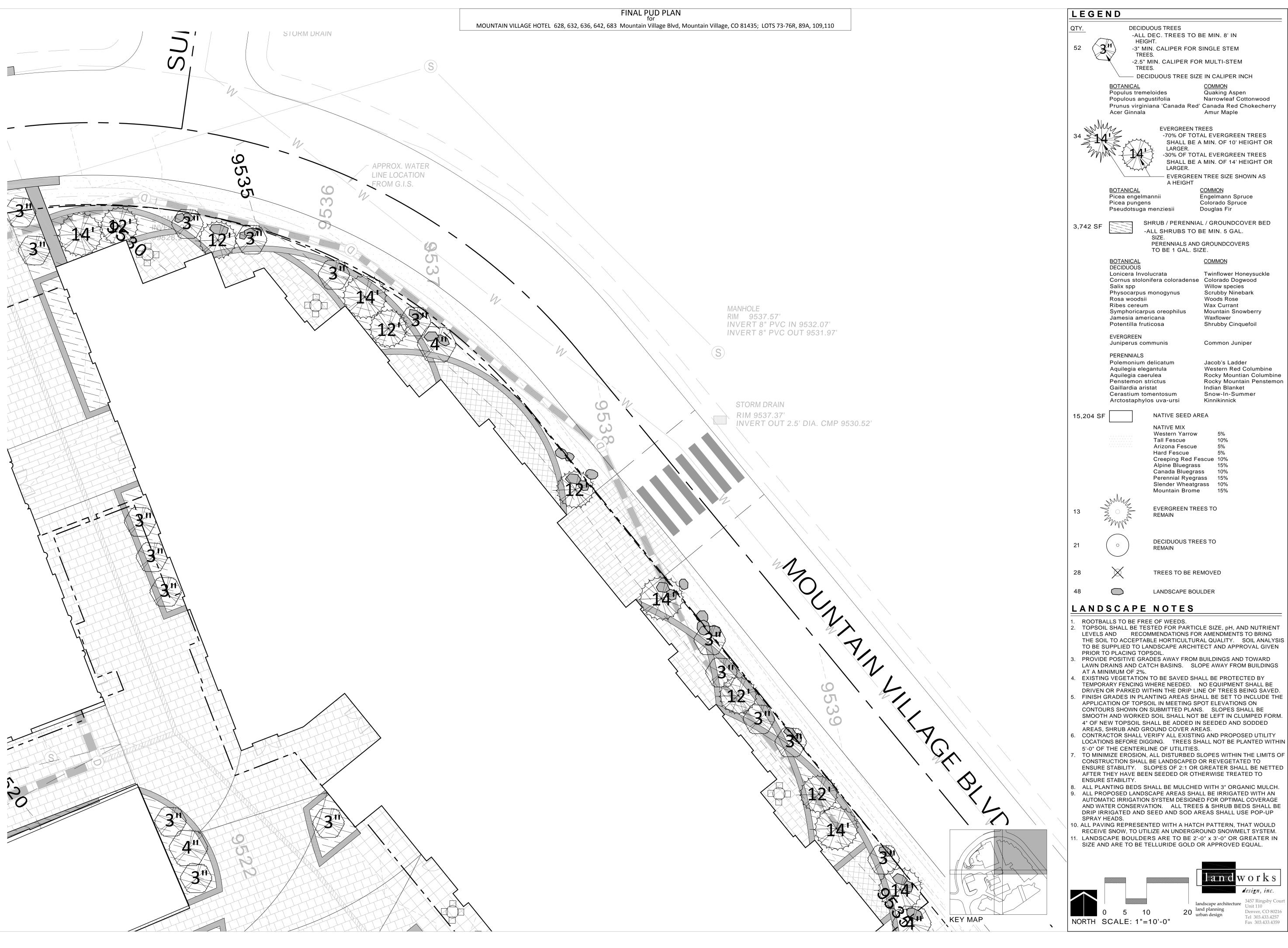
LANDSCAPE PLAN

project number 08131.100

date 11.18.2010

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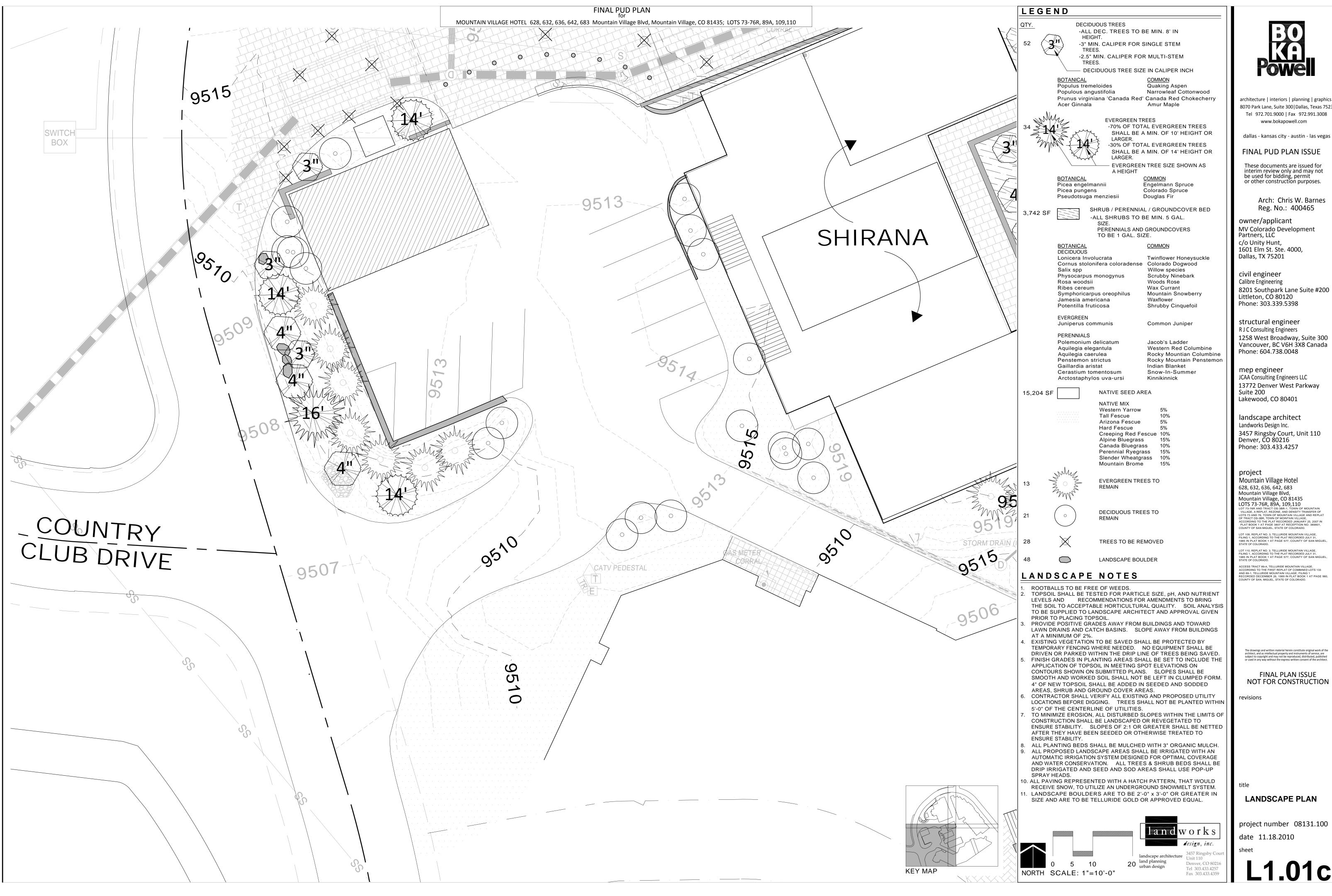
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LANDSCAPE PLAN

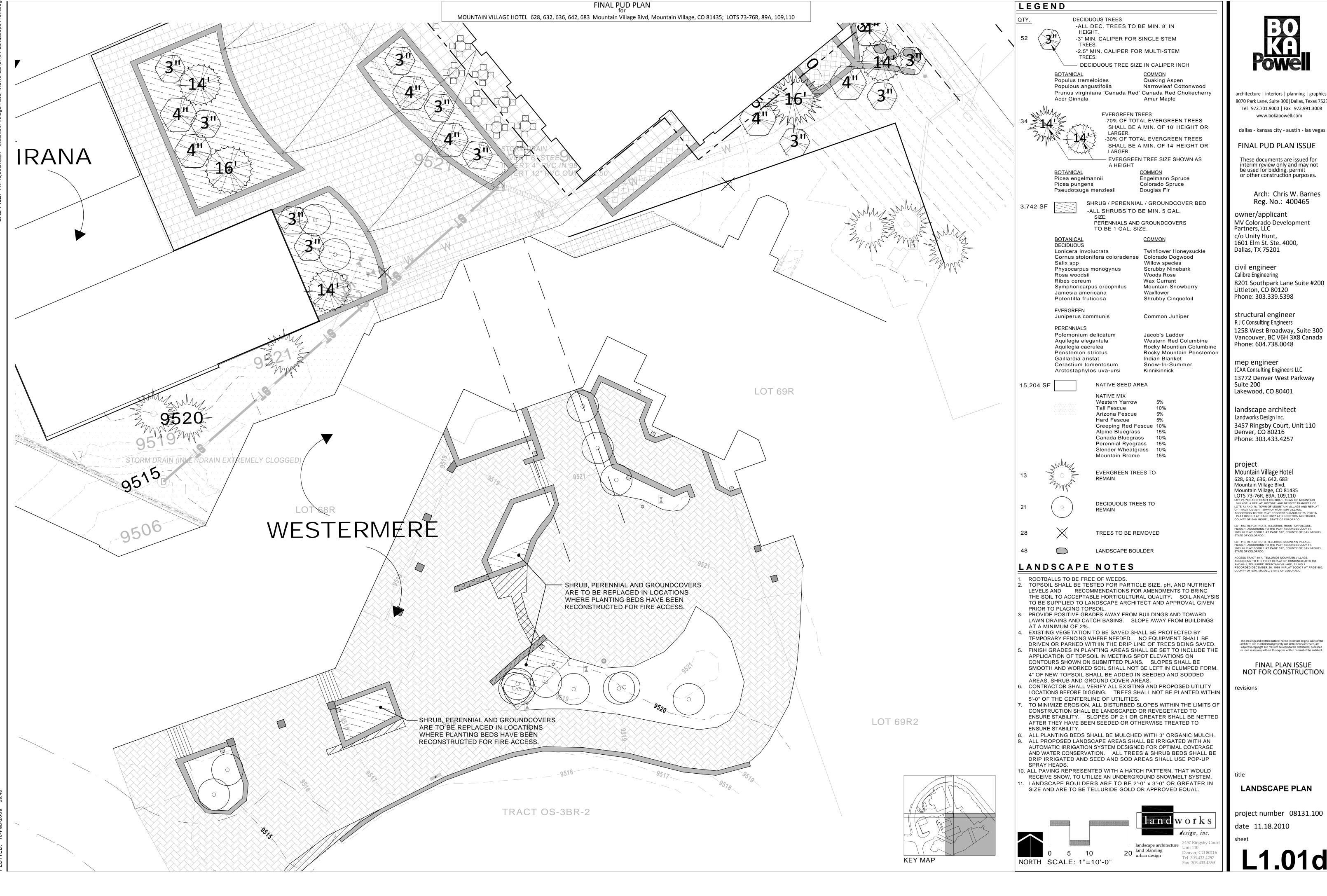
project number 08131.100 date 11.18.2010

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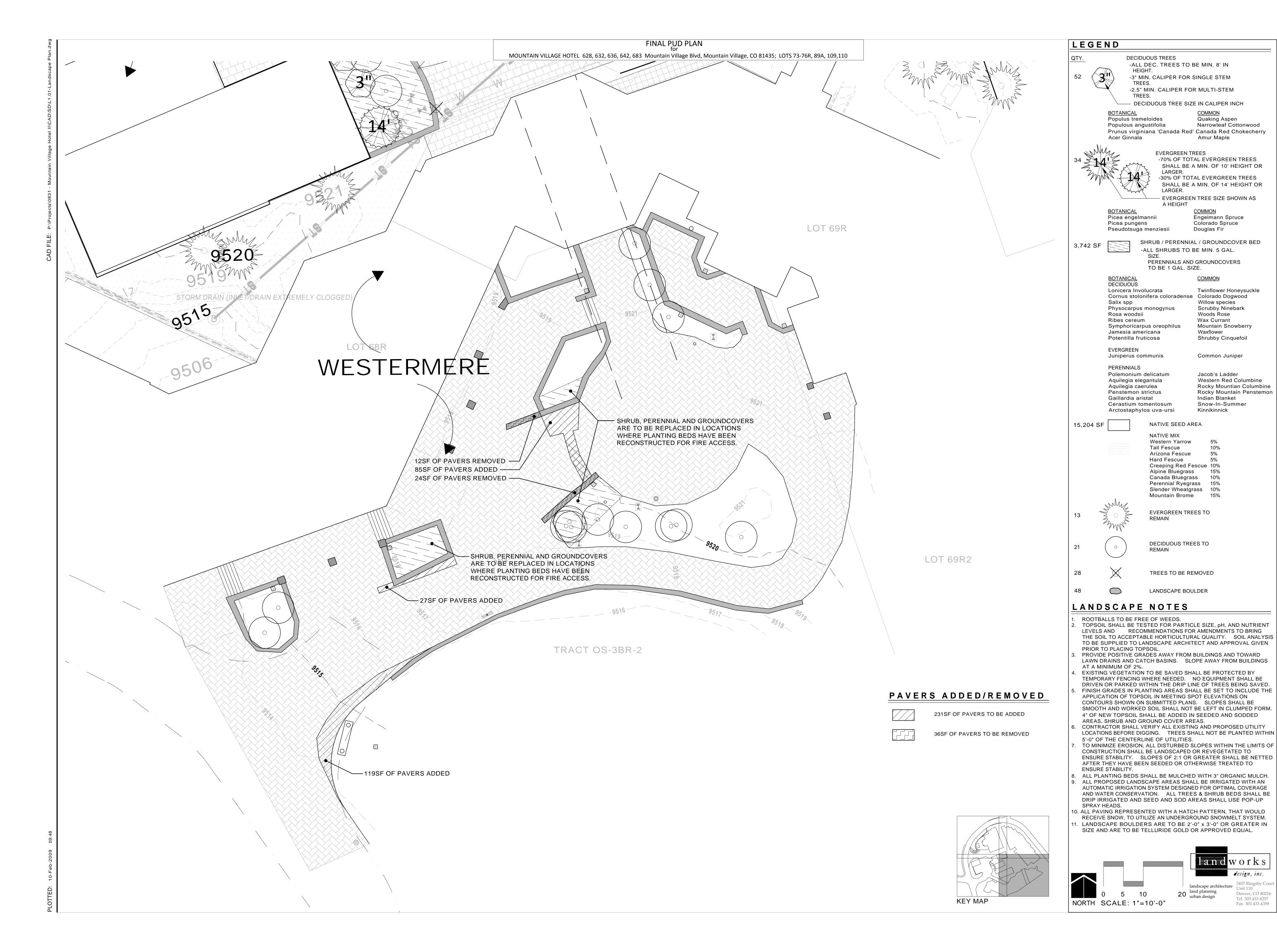
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Arch: Chris W. Barnes

8201 Southpark Lane Suite #200

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owner/applicant MV Colorado Development Partners, LLC

c/o Unity Hunt, 1601 Elm St. Ste. 4000, Dallas, TX 75201 Twinflower Honeysuckle civil engineer

COMMON Quaking Aspen

Amur Maple

Engelmann Spruce

Colorado Spruce

Willow species

Woods Rose

Wax Currant

Waxflower

Scrubby Ninebark

Mountain Snowberry

Shrubby Cinquefoil

Jacob's Ladder

Indian Blanket

Kinnikinnick

Douglas Fir

Narrowleaf Cottonwood

Calibre Engineering 8201 Southpark Lane Suite #200 Littleton, CO 80120 Phone: 303.339.5398

structural engineer Common Juniper R J C Consulting Engineers 1258 West Broadway, Suite 300 Vancouver, BC V6H 3X8 Canada Western Red Columbine Phone: 604.738.0048 Rocky Mountian Columbine Rocky Mountain Penstemon

mep engineer Snow-In-Summer JCAA Consulting Engineers LLC 13772 Denver West Parkway Suite 200 Lakewood, CO 80401

> landscape architect Landworks Design Inc. 3457 Ringsby Court, Unit 110 Denver, ČO 80216 Phone: 303.433.4257

project Mountain Village Hotel 628, 632, 636, 642, 683 Mountain Village Blvd, Mountain Village, CO 81435 LOTS 73-76R, 89A, 109,110 LOT3 75-70R, O5A, LU3, LTU
LOT 33-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
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ACCESS TRACT 89-A. TELLURIDE MOUNTAIN VILLAGE.

FINAL PLAN ISSUE

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revisions

WESTERMERE IMPROVEMENT PLAN

project number 08131.100 date 11.18.2010

landscape architecture Unit 110

Denver, CO 80216 Tel 303.433.4257

Fax 303.433.4359

20 land planning urban design



PAVESTONE "VENETIAN" SERIES, RANDOM PATTERN W/ 6"x6" 6"x9" 6"x12" & 12"x12" PAVERS, COLOR WINTER BLEND, OR APPROVED EQUAL.



PAVESTONE "VENETIAN" SERIES, RANDOM PATTERN W/ 6"x6" 6"x9" 6"x12" & 12"x12" PAVERS, COLOR OLD TOWN BLEND, OR APPROVED EQUAL.

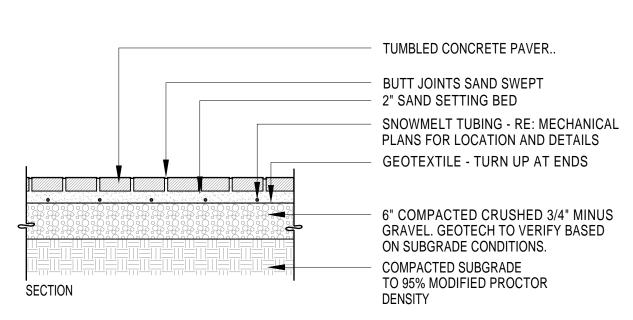
Concrete Unit Pavers Type II

STONEBILT CONCEPTS SLATE, COLOR SMOKE, RUNNING BOND PATTERN, 12"x24"x1 3/4" OR APPROVED EQUAL.

Concrete Unit Pavers Type III

SCALE: 1/2" = 1'-0"

Concrete Unit Pavers Type I



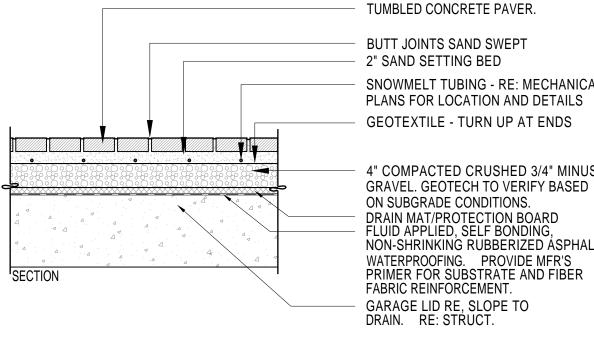
Standard Paver Detail w/ Snowmelt on Grade

SCALE: 3/4" = 1'-0"

BUTT JOINTS SAND SWEPT 2" SAND SETTING BED SNOWMELT TUBING - RE: MECHANICAL PLANS FOR LOCATION AND DETAILS GEOTEXTILE - TURN UP AT ENDS 4" COMPACTED CRUSHED 3/4" MINUS GRAVEL. GEOTECH TO VERIFY BASED ON SUBGRADE CONDITIONS. DRAIN MAT/PROTECTION BOARD FLUID APPLIED, SELF BONDING, NON-SHRINKING RUBBERIZED ASPHALT WATERPROOFING. PROVIDE MFR'S PRIMER FOR SUBSTRATE AND FIBER FABRIC REINFORCEMENT. GARAGE LID RE, SLOPE TO DRAIN. RE: STRUCT.

Standard Paver Detail w/ Snowmelt on Structure

 $^{\prime}$ SCALE: 3/4" = 1'-0"



CAST IRON SILL DRAIN W/ BRONZE DOME PROVIDE WATER PROOF FLASHING BENEATH CLAMPING RINGS. RE. MECHANICAL PLANS AND SPECS FOR CONNECTION & PIPE DETAILS

RE: ARCH AND STRUCTURAL DRAWINGS FOR GARAGE LID DETAILS AND WATERPROOFING

HOT RUBBERIZED ASPHALT W.P. OVER PROTECTIONMAT & DRAINAGE BOARD 3.5" WATERMARK SANDSTONE CAP LENGTH OF CAP- 2'-0" - 3'-0" STONE TO MATCH STONE ON BLDG. #5 @ 10" RE: STRUCTURAL #5 @ 8" RE: STRUCTURAL

DEVELOP YIELD STRENGTH OF BAR

CONDITION VARIES - RE: PLAN 6" LAYER OF DRAIN ROCK AT BASE OF PLANTER HOT RUBBERIZED ASPHALT W.P. OVER PROTECTIONMAT & DRAINAGE BOARD #4 @ 1'=4" DRILL AND EPOXY GROUT USING HILTI HVA SYSTEM. DEPTH AND DIAMETER OF HOLE TO BE ACCORDING TO MANUFACTURES RECOMMENDATIONS TO

Planter on Lid Detail

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LOT 73-76R AND TRACT 0S-3BR-1, TOWN OF MOUNTAIN VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF LOTS 73 AND 76, TOWN OF MOUNTAIN VILLAGE AND REPLAT OF TRACT OS-3BR, TOWN OF MOINTAIN VILLAGE, ACCORDING TO THE PLAT RECORDED JANUARY 25, 2007 IN PLAT BOOK 1 AT PAGE 3807 AT RECEPTION NO. 389801, COUNTY OF SAN MIGUEL, STATE OF COLORADO.

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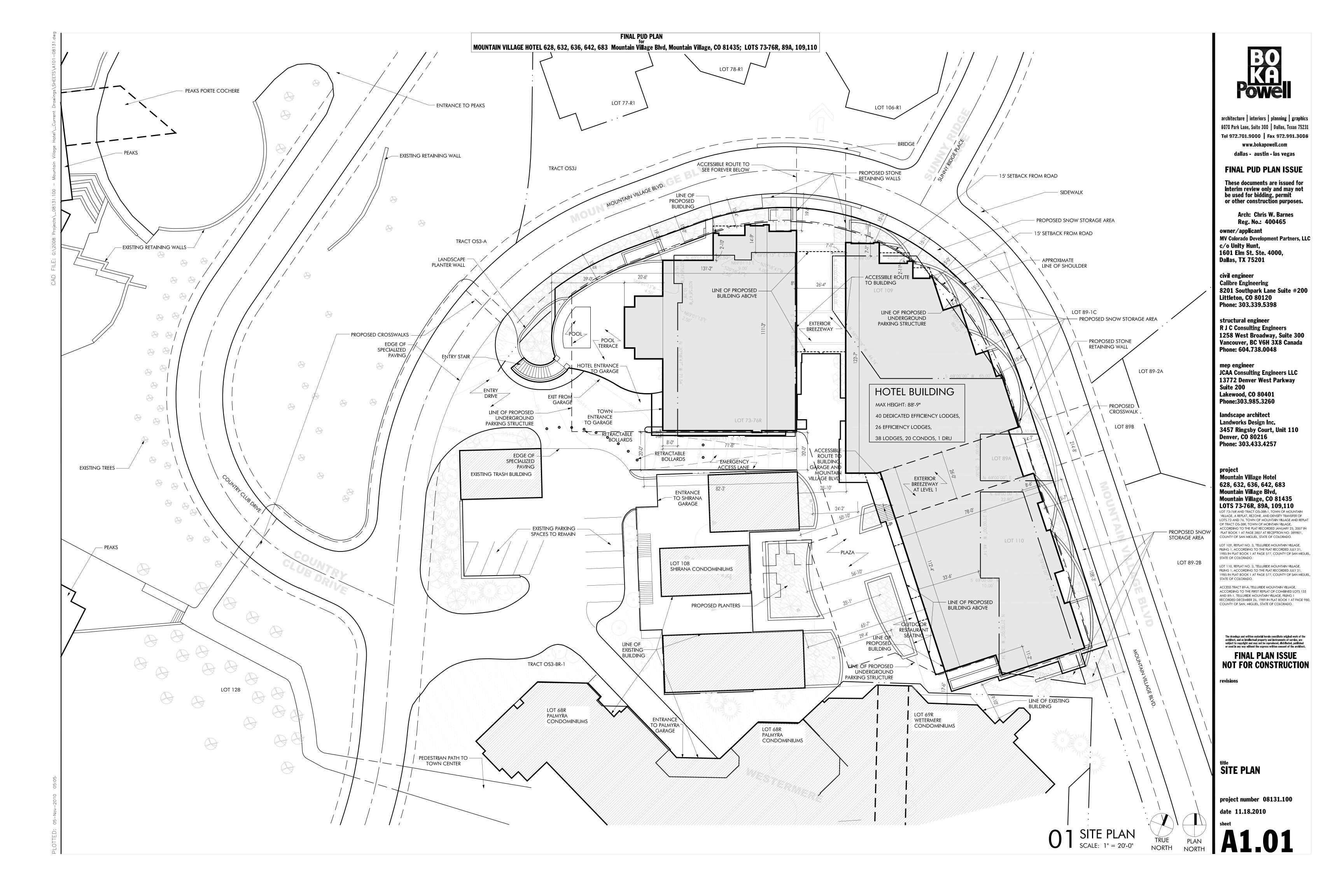
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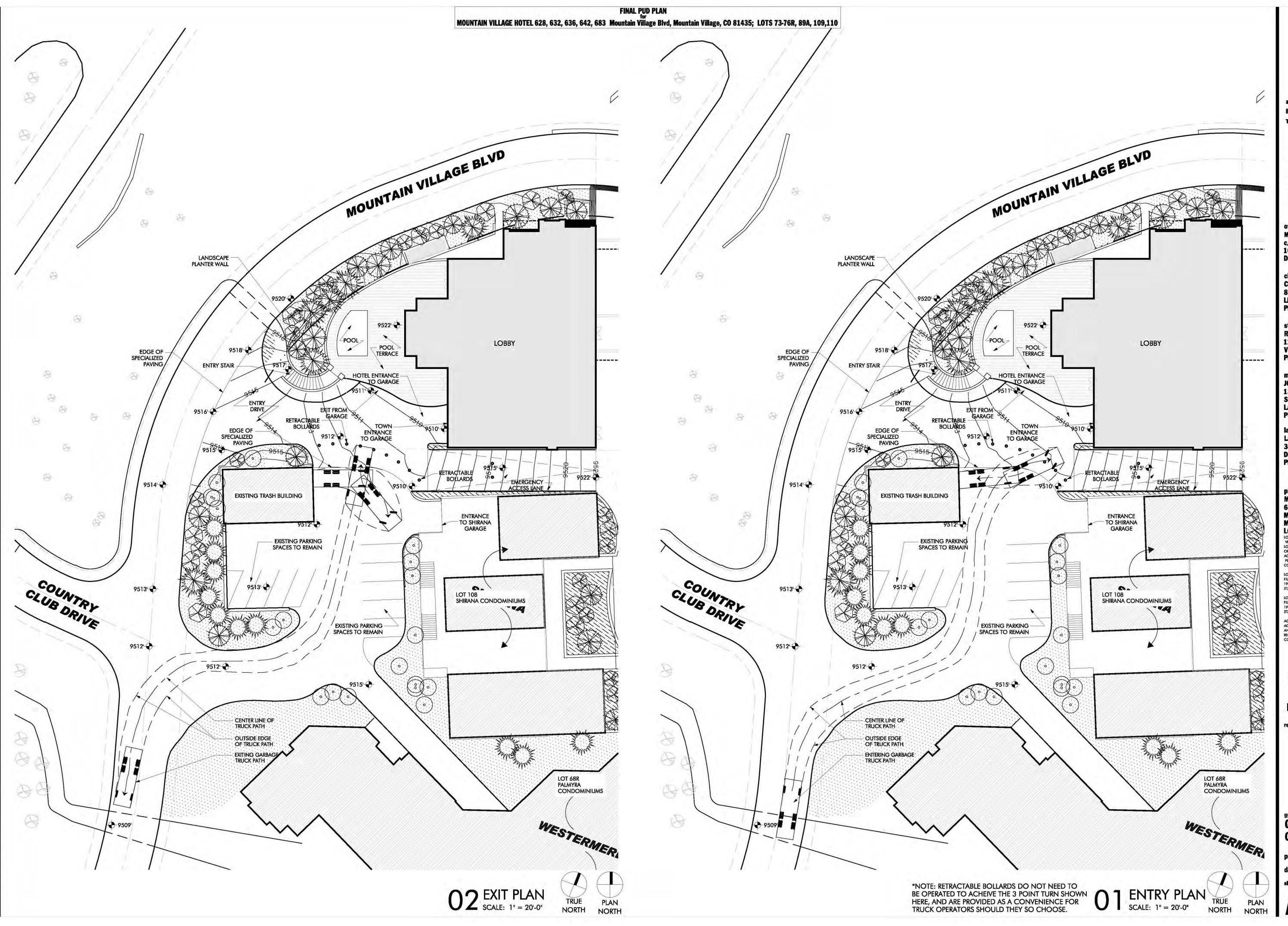
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LANDSCAPE DETAILS

project number 08131.100 date 11.18.2010







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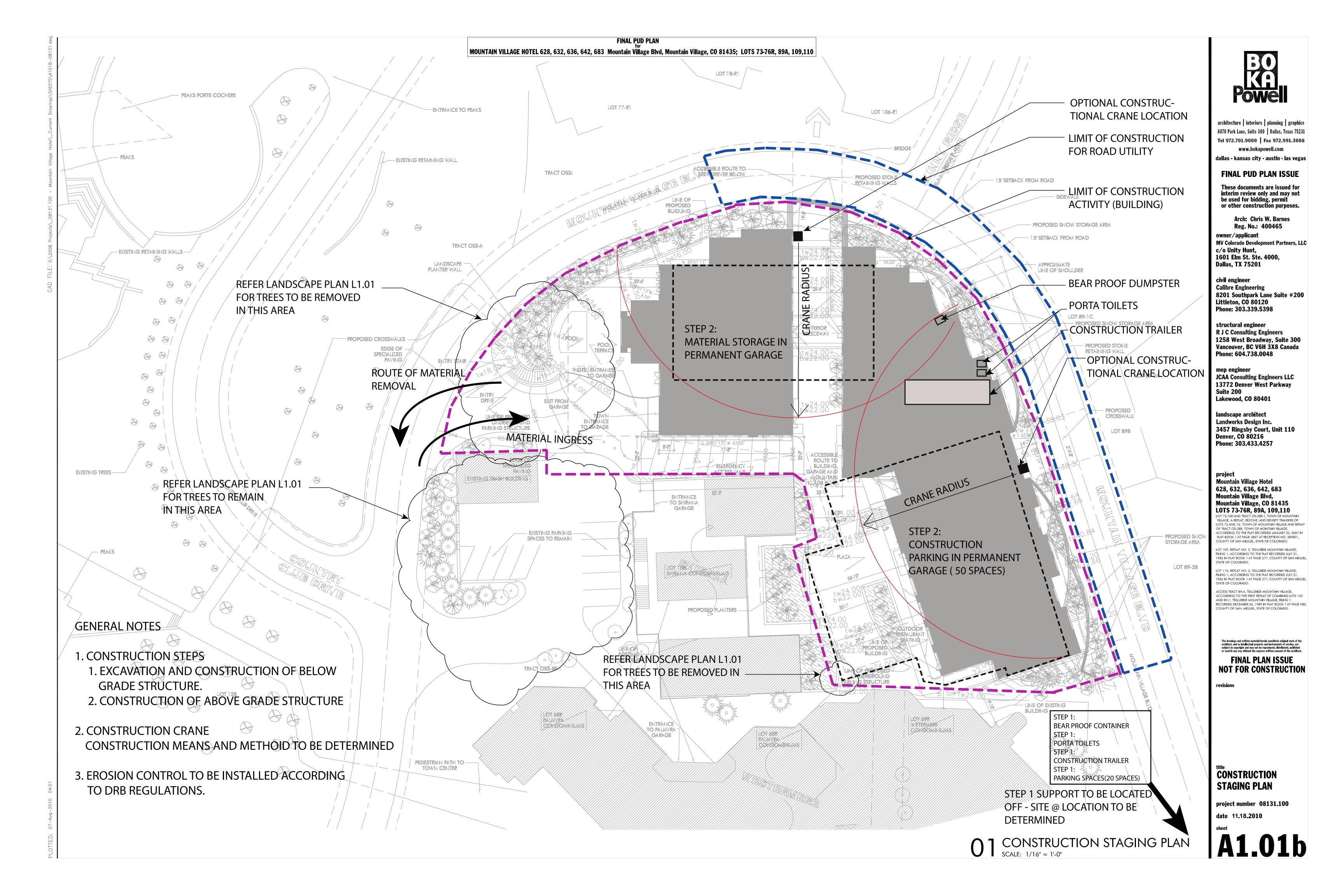
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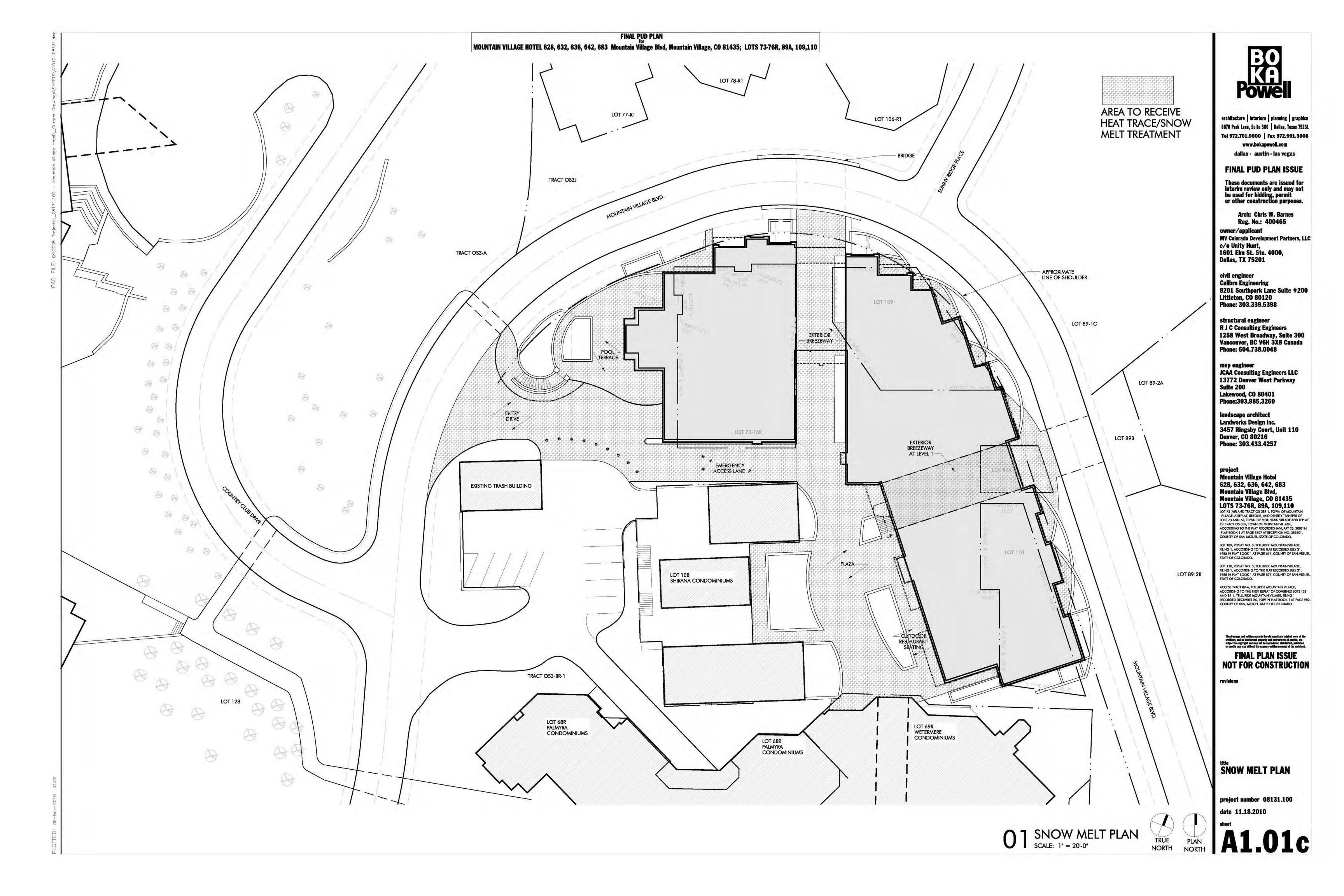
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GARBAGE TRUCK
CIRCULATION PLAN

project number 08131.100 date 11.18.2010

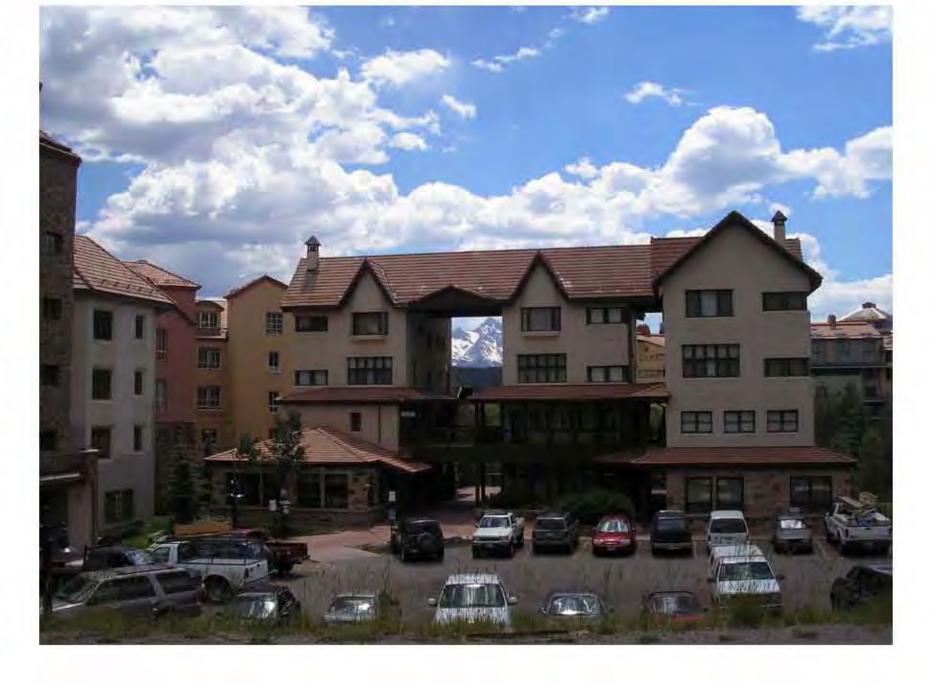
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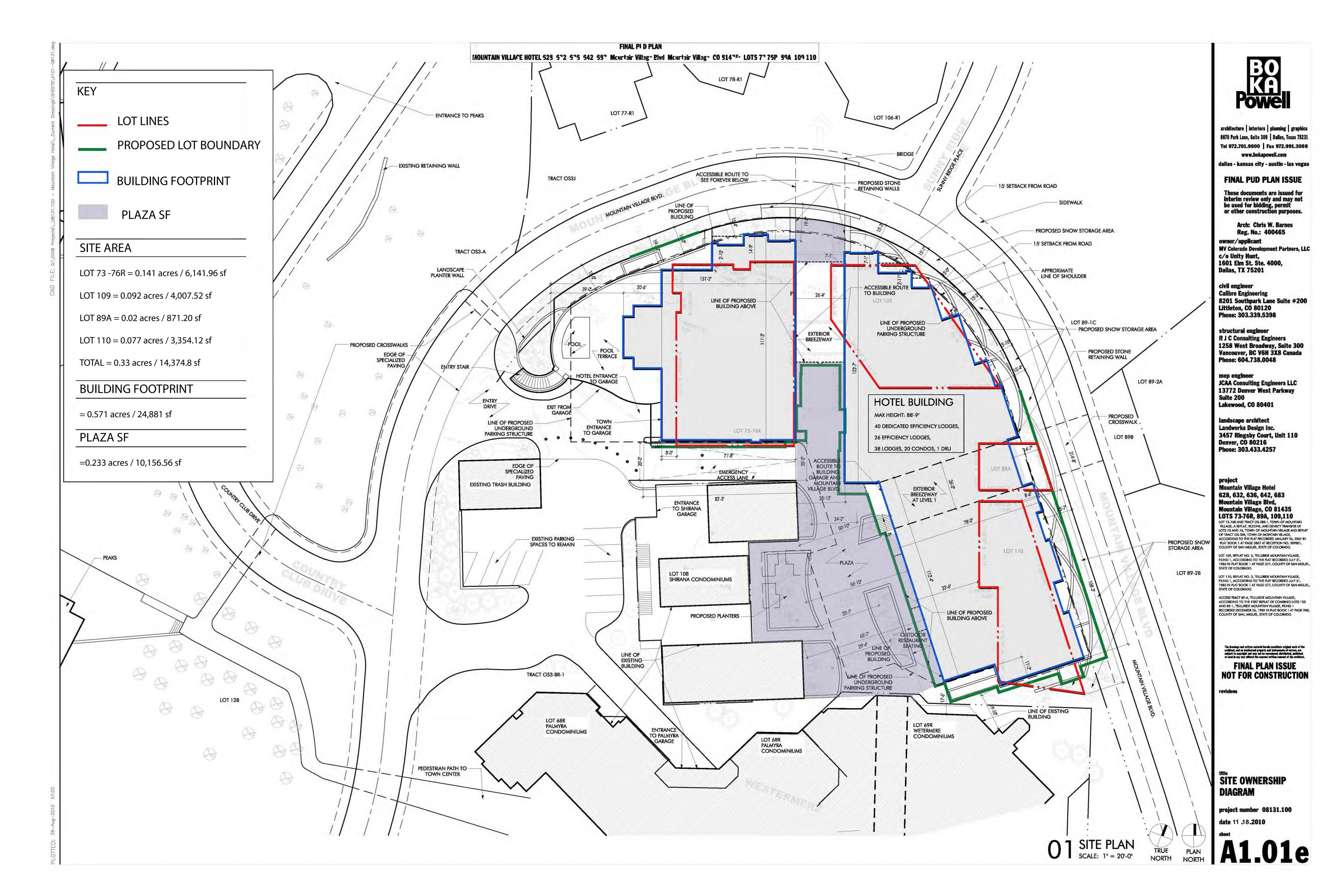
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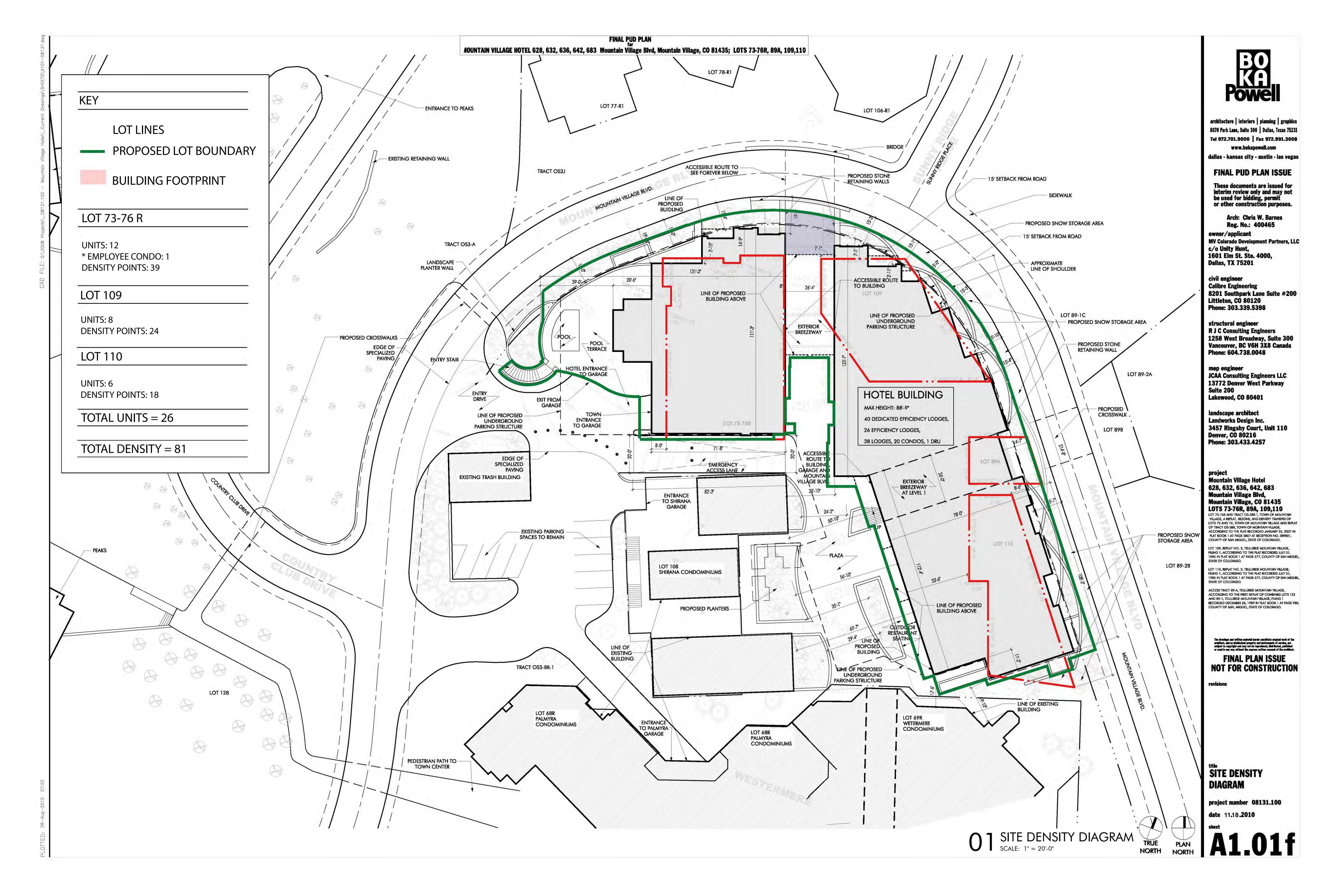
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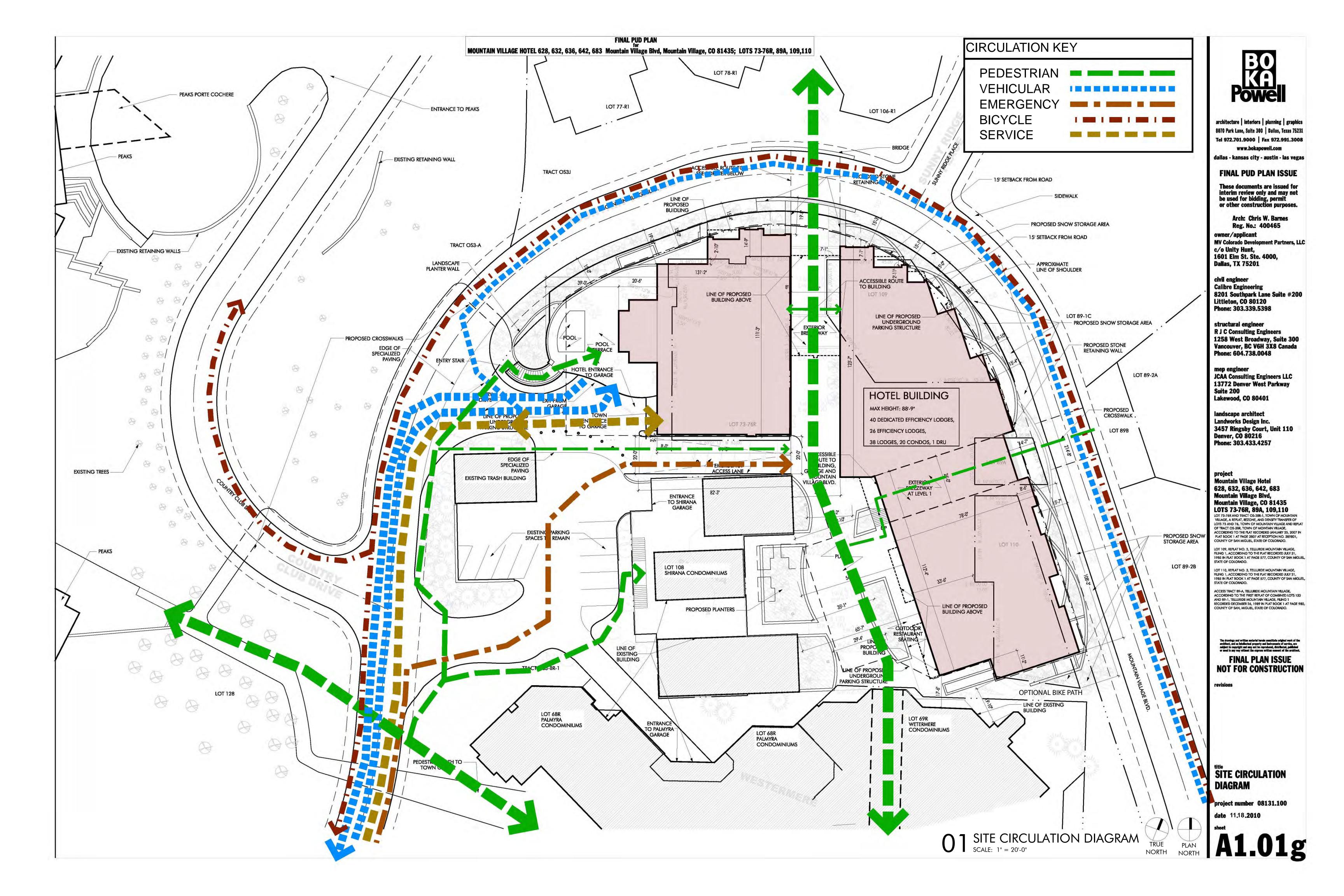
SITE PHOTOS

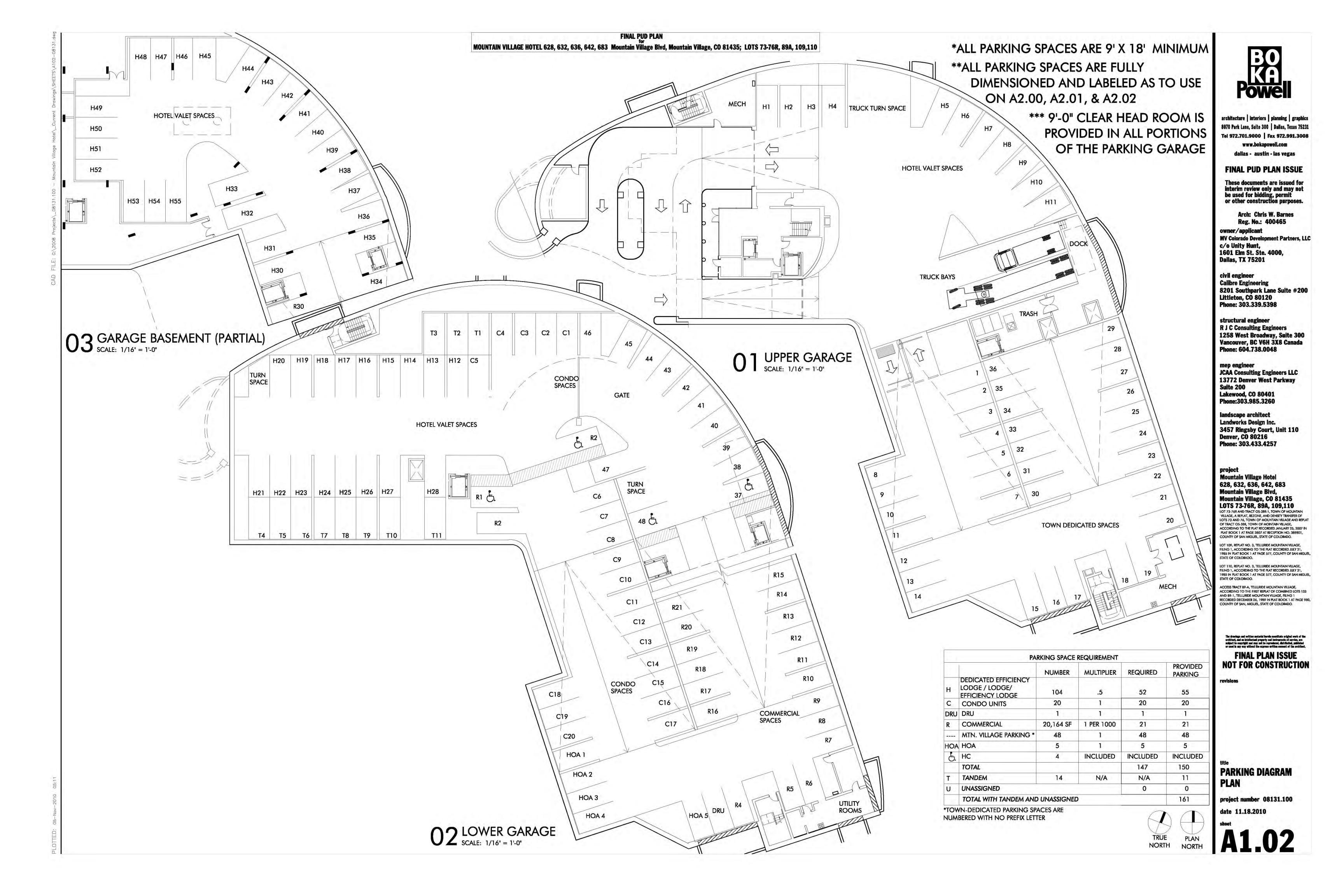
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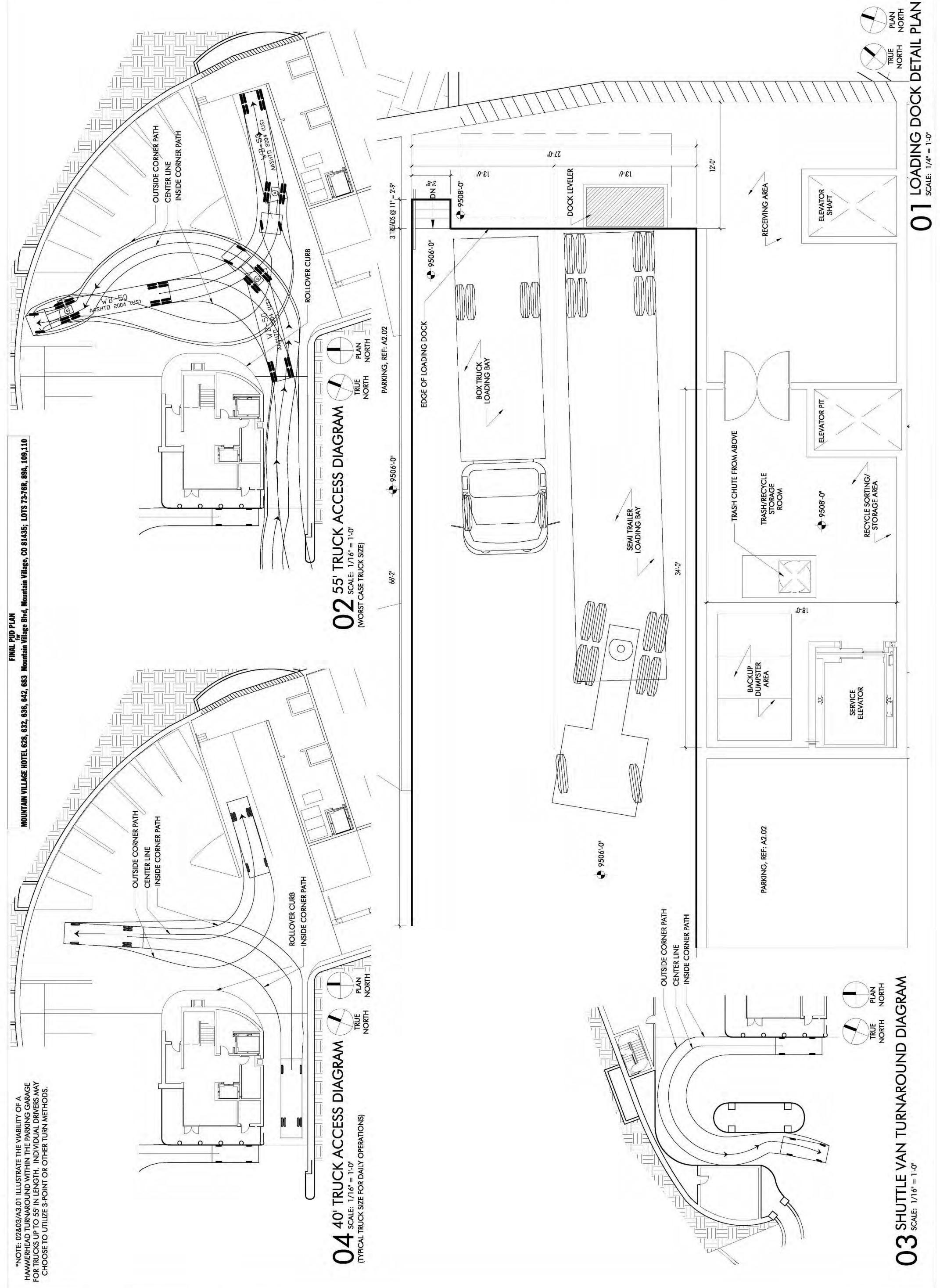








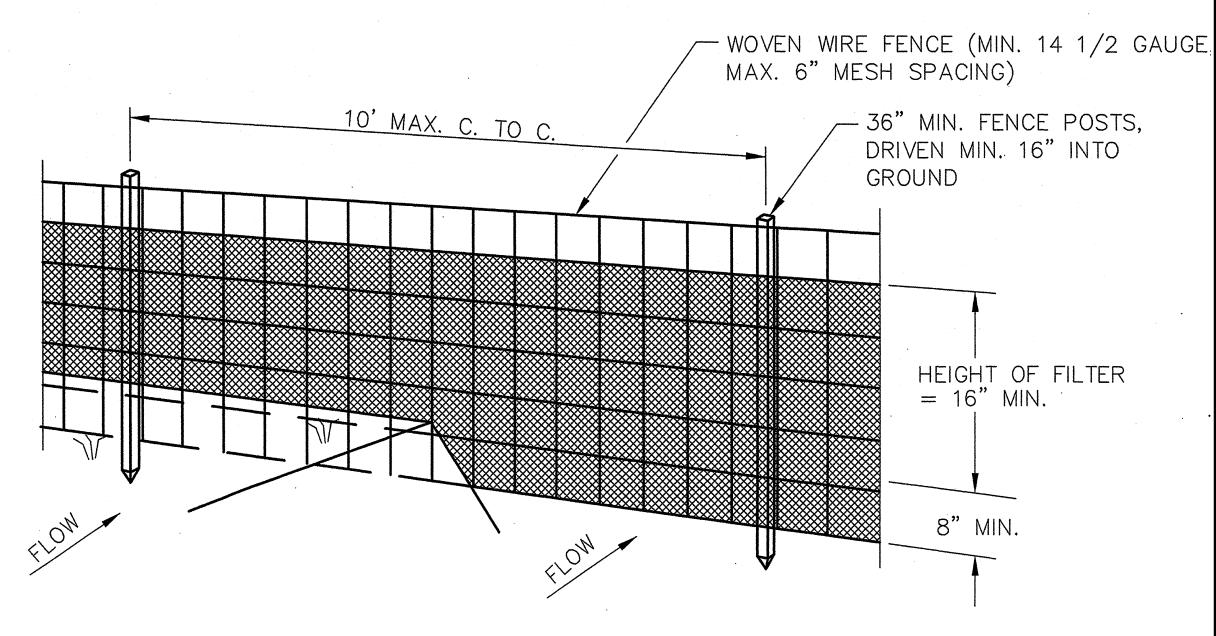




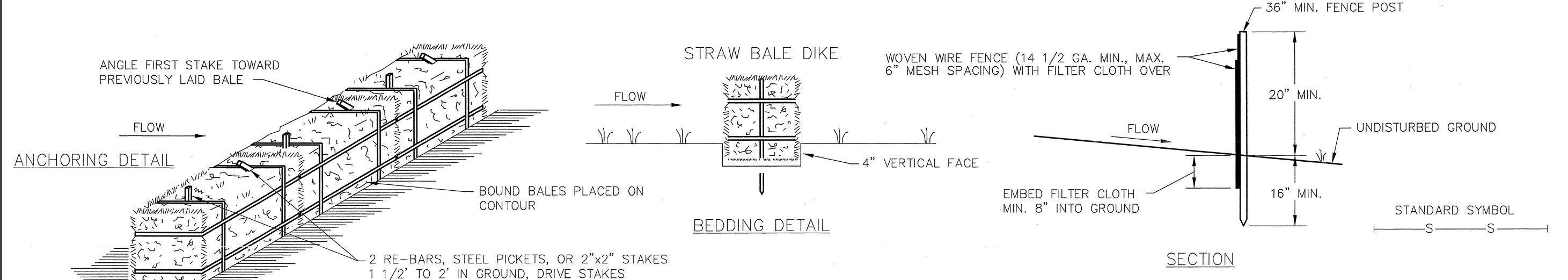
GENERAL NOTES

- Horizontal and vertical base information provided by Architect. Contractor shall verify existing conditions.
- 2. The extent of existing underground utilities is not known. Contractor shall field-verify existing grades, utilities and conditions, and notify the Architect of any discrepancies before starting work, making modifications as directed by Architect. Contractor will be required to complete the work of this project according to the proposed drawings, details and specifications. If conflicts arise in the field, contact Architect for resolution. Contractor shall take responsibility for any costs incurred due to damage of said utilities.
- These drawings do not specify safety materials or equipment, methods or sequencing to protect persons and property. It shall be the contractor's sole responsibility to direct and implement safety operations and procedures to protect the Owner, Architect, subcontractors, the public and others.
- If conflicts arise between size of areas and plans. Contractor is to contact Architect for resolution. Failure to make such conflicts known to the Architect will result in Contractor's liability to relocate the materials.
- Do not willfully proceed with construction as designed when it is obvious that unknown obstructions and/or grade differences exist that may not have been known during design. Such conditions shall be immediately brought to the attention of the Architect. The Contractor shall assume full responsibility for all necessary revisions due to failure to give such notification.
- Contractor shall be responsible for any coordination with subcontractors or other General Contractors as required to accomplish construction operations.
- Verify locations and conditions of existing trees and plant material prior to beginning work. Contractor shall take appropriate measures to protect them and avoid compaction within drip lines. Avoid cutting of roots over 1" diameter Existing plant material damaged by contractor shall be replaced at contractor's expense. Refer to specifications for Tree protection requirements.
- 8. The location of features to be constructed not specifically dimensioned may be determined by scale. If conflicts arise in field, contact the Architect for resolution.

- Dimensions and configurations of construction of existing conditions are based on the best available information. The Contractor shall verify all existing conditions prior to ordering, fabricating, otherwise incurring costs, and/or completion of shop drawing review.
- Finished grades shall provide for natural runoff of water without low spots or pockets. Set flow lines accurately and provide a minimum 2% and maximum 30% gradient unless otherwise noted. Positive drainage shall occur away from buildings and improvements. Notify the Architect of any discrepancies before starting work.
- Hold finished grades (top of mulch) for shrubs and groundcover areas 1" below top of adjacent pavements, curbs, headers unless otherwise noted on the drawings
- Gradually round off tops and toes of slopes to produce a smooth and natural appearing transition between relatively level areas and slopes.
- Rough grade shall be reviewed and accepted by the Architect prior to soil preparation operations.
- All angles are 90 degrees unless noted otherwise
- Finished grades include 8" of topsoil in planting beds.



PERSPECTIVE VIEW



CONSTRUCTION SPECIFICATIONS

FLUSH WITH BALES.

- 1. BALES SHALL BE PLACED AT THE TOE OF A SLOPE OR ON THE CONTOUR AND IN A ROW WITH ENDS TIGHTLY ABUTTING THE ADJACENT BALES.
- 2. EACH BALE SHALL BE EMBEDDED IN THE SOIL A MINIMUM OF (4) INCHES, AND PLACED SO THE BINDINGS ARE HORIZONTAL.
- 3. BALES SHALL BE SECURELY ANCHORED IN PLACE BY EITHER TWO STAKES OR RE-BARS DRIVEN THROUGH THE BALE. THE FIRST STAKE IN EACH BALE SHALL BE DRIVEN TOWARD THE PREVIOUSLY LAID BALE AT AN ANGLE TO FORCE THE BALES TOGETHER. STAKES SHALL BE DRIVEN FLUSH WITH THE BALE.
- 4. INSPECTION SHALL BE FREQUENT AND REPAIR REPLACEMENT SHALL BE MADE PROMPTLY AS NEEDED BY THE CONTRACTOR.
- 5. BALES SHALL BE REMOVED WHEN THEY HAVE SERVED THEIR USEFULNESS SO AS NOT TO BLOCK OR IMPEDE STORM FLOW OR DRAINAGE.

CONSTRUCTION NOTES FOR FABRICATED SILT FENCE

- 1. WOVEN WIRE FENCE TO BE FASTENED SECURELY TO FENCE POSTS WITH WIRE TIES OR STAPLES.
- 2. FILTER CLOTH TO BE FASTENED SECURELY TO WOVEN WIRE FENCE WITH TIES SPACED EVERY 24" AT TOP AND MID SECTION.
- 3. WHEN TWO SECTIONS OF FILTER CLOTH ADJOIN EACH OTHER THEY SHALL BE OVERLAPPED BY SIX INCHES AND FOLDED.
- 4. MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL REMOVED WHEN "BULGES" DEVELOP IN THE SILT FENCE. NTS

POSTS: STEEL EITHER T OR U TYPE OR 2" HARDWOOD.

FENCE: WOVEN WIRE, 14 1/2 GA. 6" MAX. MESH OPENING.

FILTER CLOTH: FILTER X, MIRAFI 100X, STABILINKA T140N OR APPROVED EQUAL.

PREFABRICATED UNIT: GEOFAB, ENVIROFENCE, OR APPROVED EQUAL.

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Mountain Village Hotel 628, 632, 636, 642, 683 Mountain Village Blvd, Mountain Village, CO 81435 LOTS 73-76R, 89A, 109,110

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EROSION CONTROL PLAN





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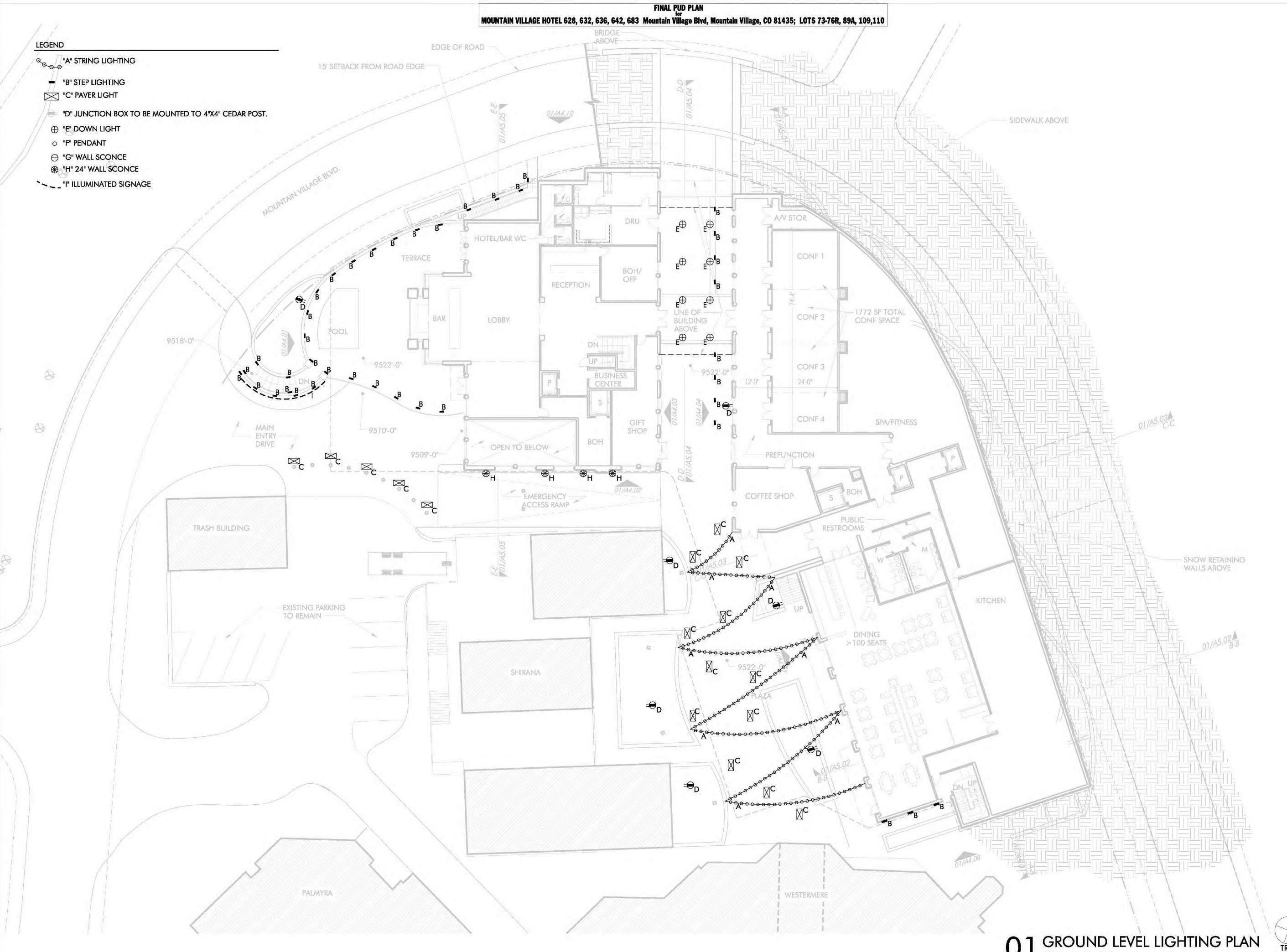
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UPPER GARAGE LIGHTING PLAN

project number 08131.100 date 11.18.2010

A1.05



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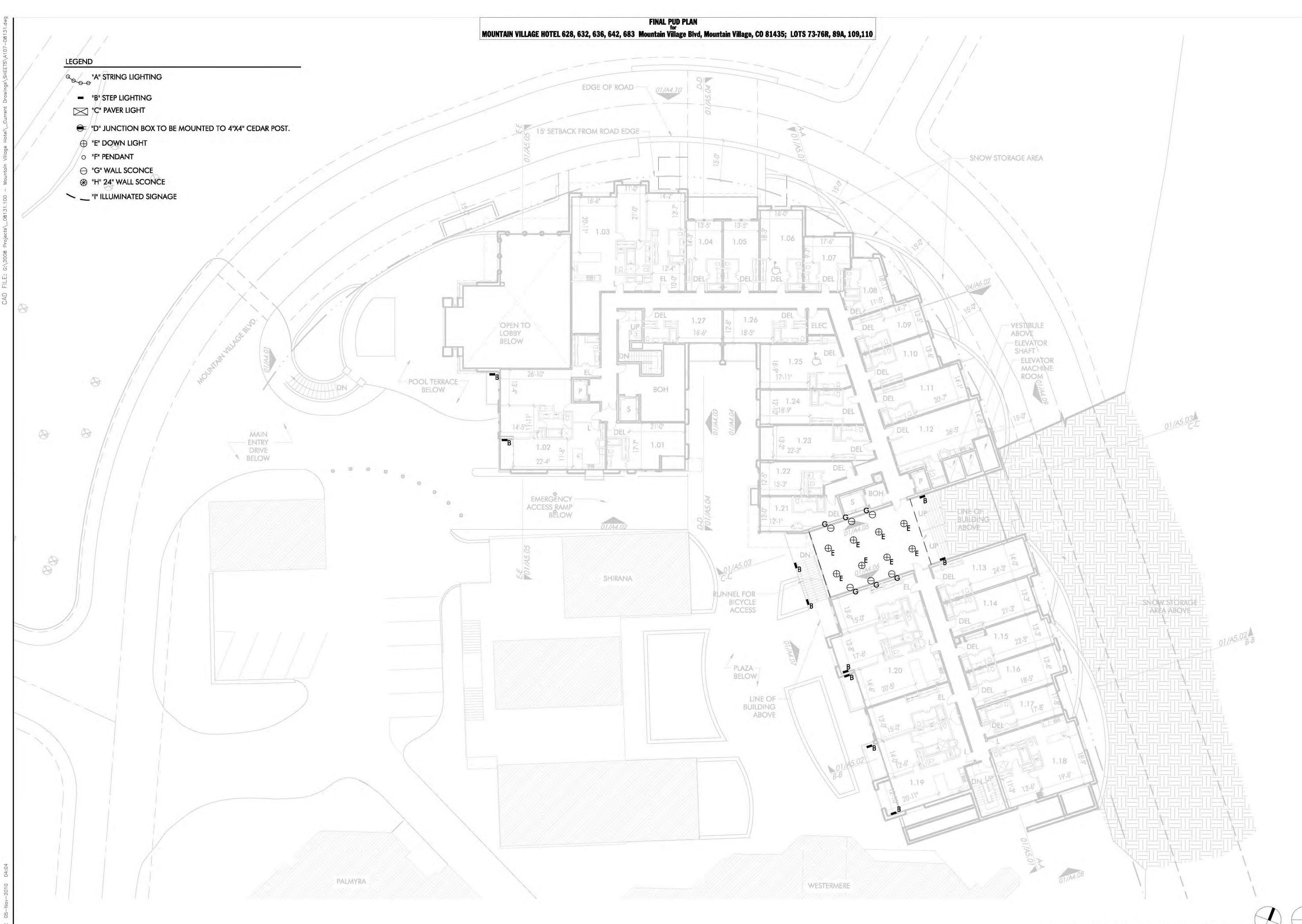
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GROUND FLOOR **LIGHTING PLAN**

project number 08131.100 date 11.18.2010

O 1 GROUND LEVEL LIGHTING PLAN SCALE: 1/16" = 1'-0"





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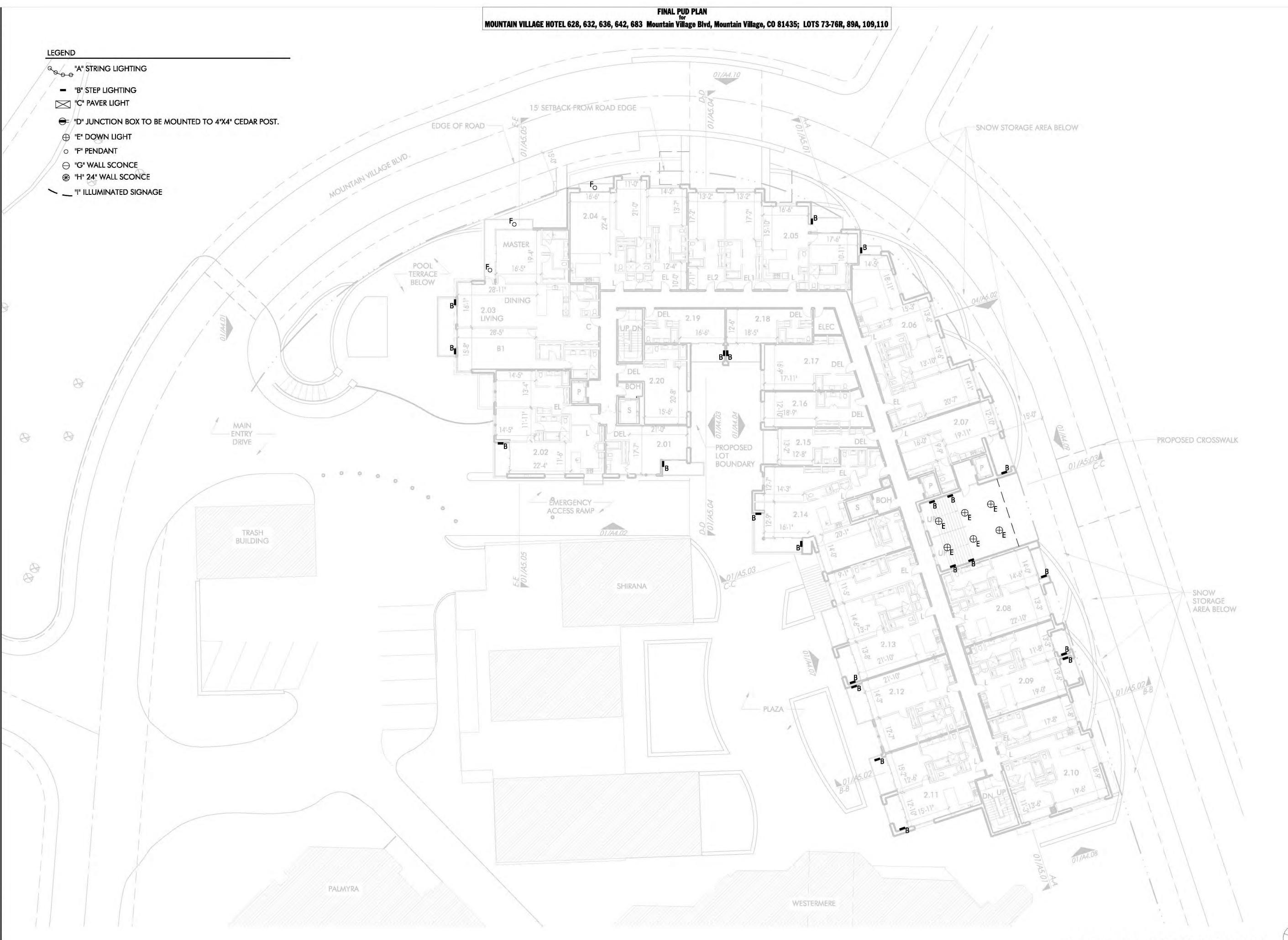
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LEVEL 1 LIGHTING PLAN

project number 08131.100 date 11.18.2010

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O 1 LEVEL 1 LIGHTING PLAN SCALE: 1/16" = 1'-0"





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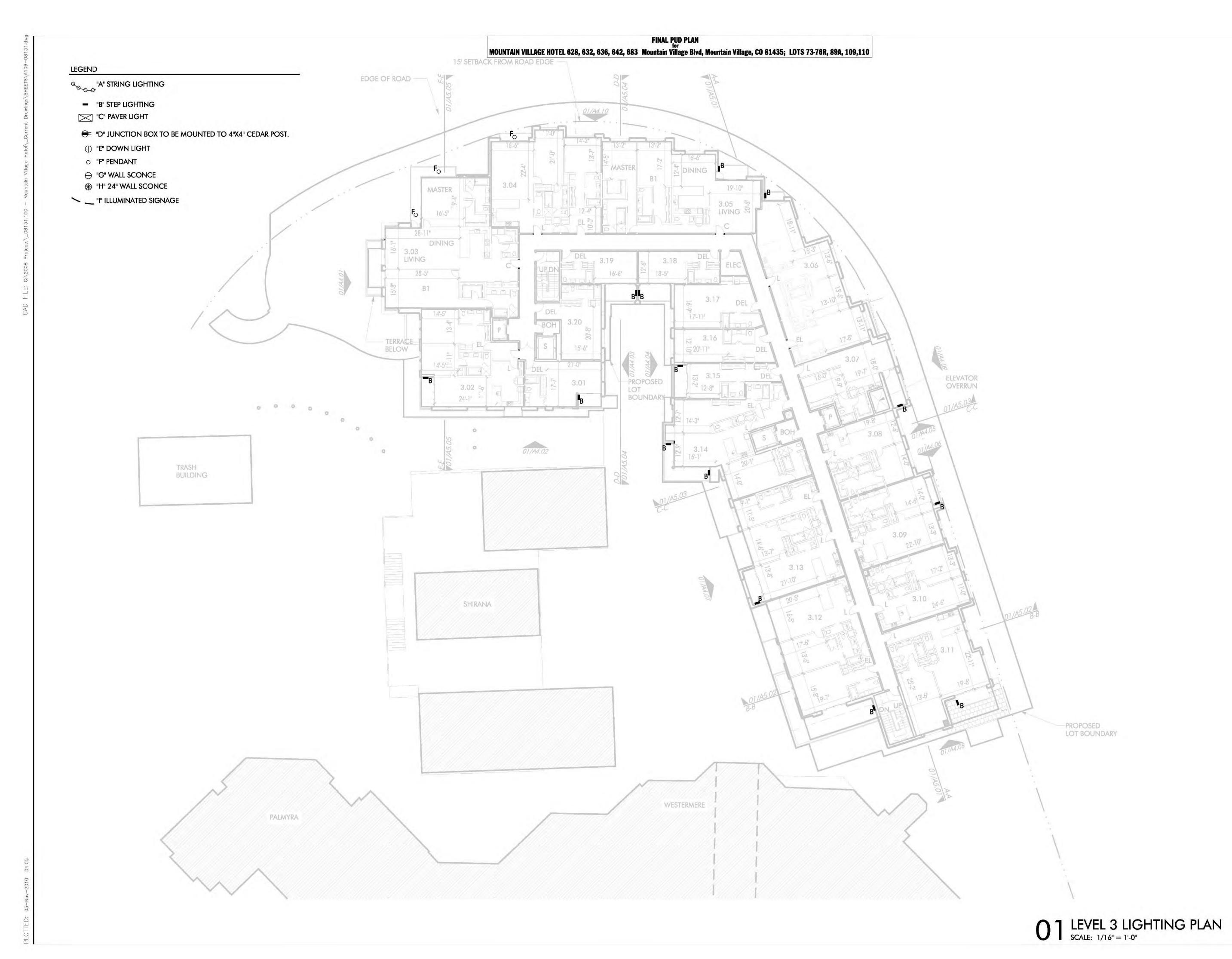
revisions

LEVEL 2 LIGHTING PLAN

project number 08131.100 date 11.18.2010

PLAN ALO

O 1 LEVEL 2 LIGHTING PLAN SCALE: 1/16" = 1'-0"





dallas - austin - las vegas

FINAL PUD PLAN ISSUE

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landscape architect Landworks Design Inc. 3457 Ringsby Court, Unit 110 Denver, CO 80216 Phone: 303.433.4257

project
Mountain Village Hotel
628, 632, 636, 642, 683
Mountain Village Blvd, Mountain Village, CO 81435
LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF LOTS 73 AND 76, TOWN OF MOUNTAIN VILLAGE AND REPLAT OF TRACT OS-3BR, TOWN OF MOINTAIN VILLAGE,
ACCORDING TO THE PLAT RECORDED JANUARY 25, 2007 IN
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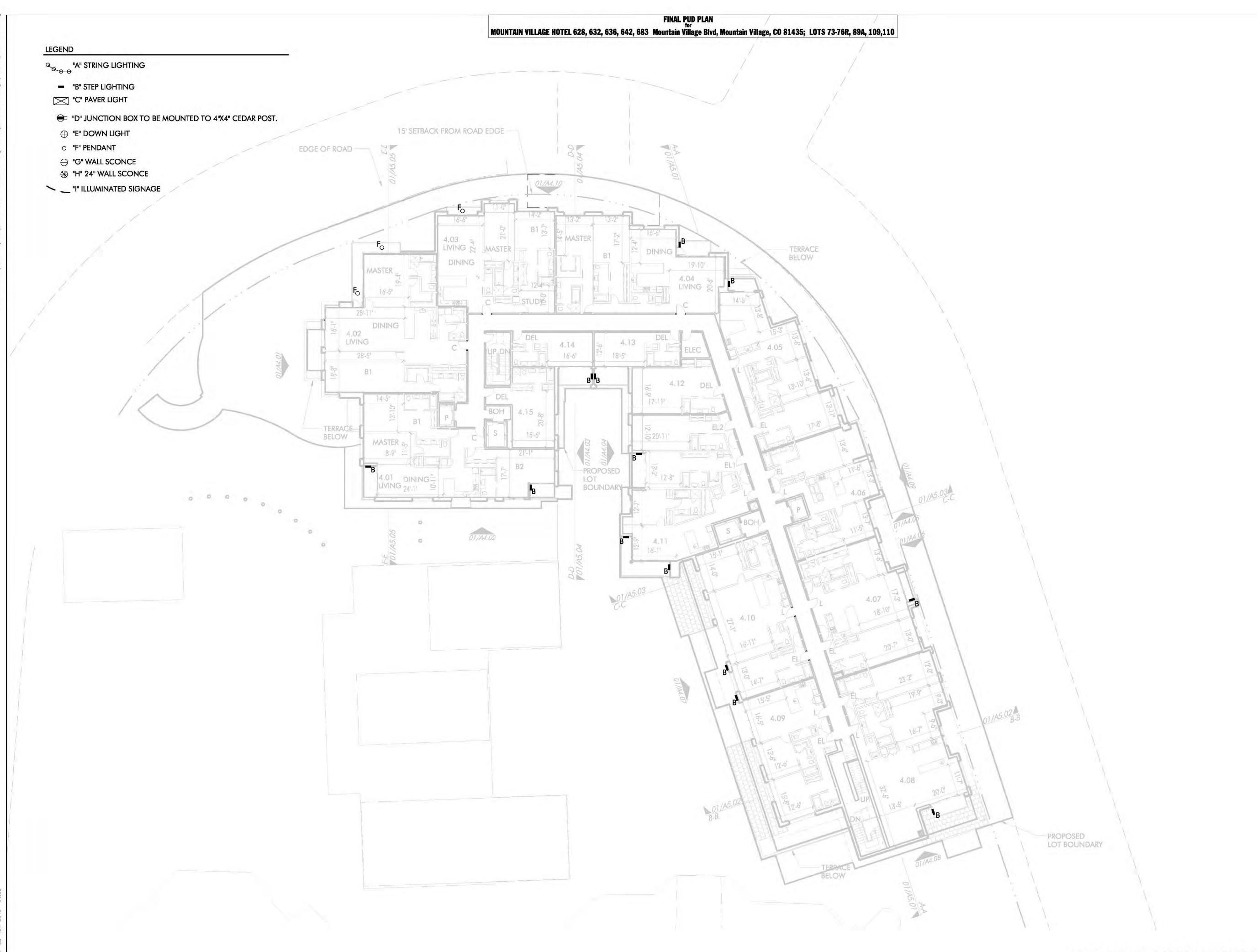
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LEVEL 3 **LIGHTING PLAN**





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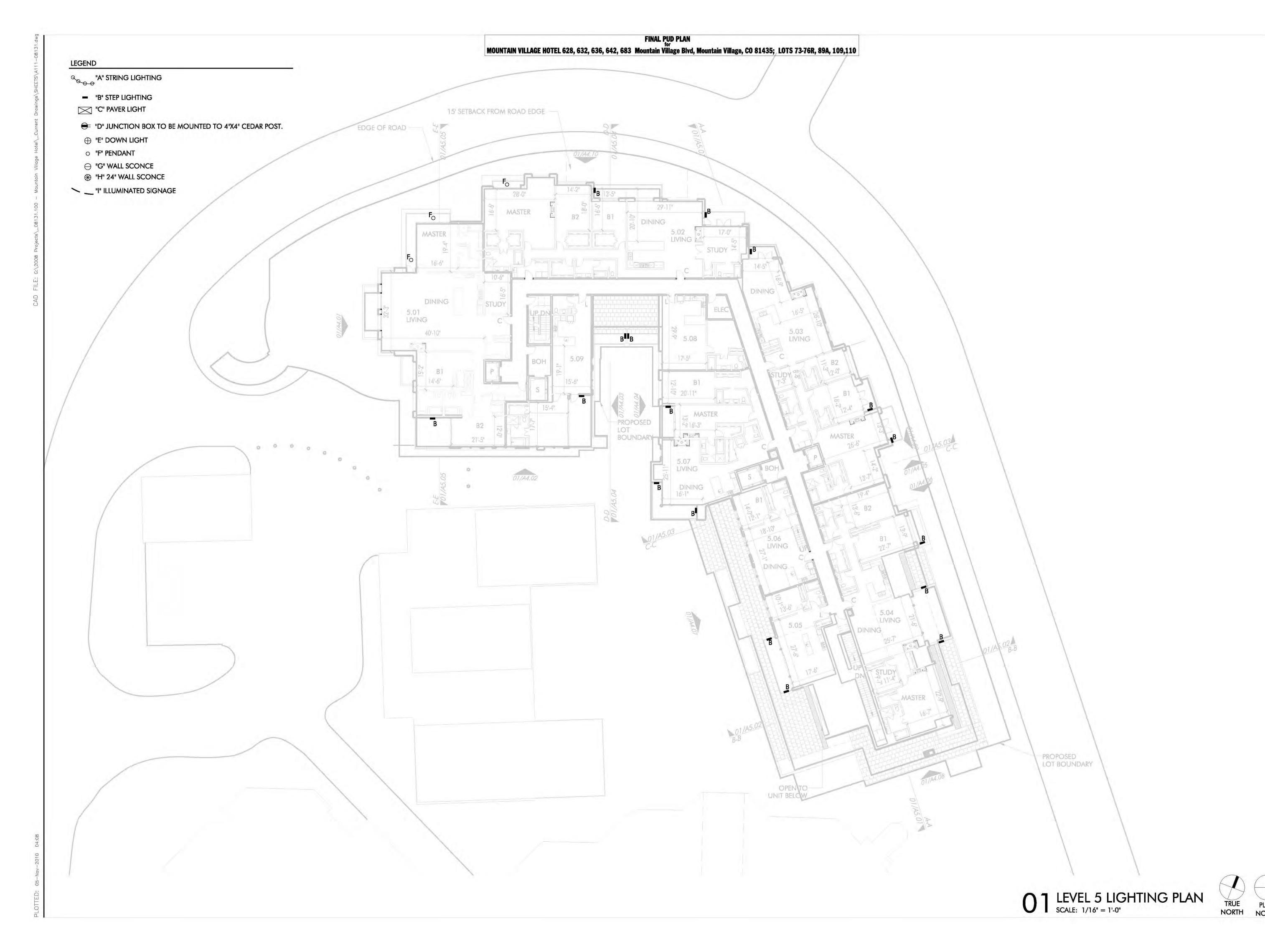
revisio

LEVEL 4 LIGHTING PLAN

project number 08131.100 date 11.18.2010

sheet A 1.10

O 1 LEVEL 4 LIGHTING PLAN SCALE: 1/16" = 1'-0"



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LEVEL 5 LIGHTING PLAN





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LOTS 73-76R, 89A, 109,110
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LEVEL 6 LIGHTING PLAN

project number 08131.100

A1.12





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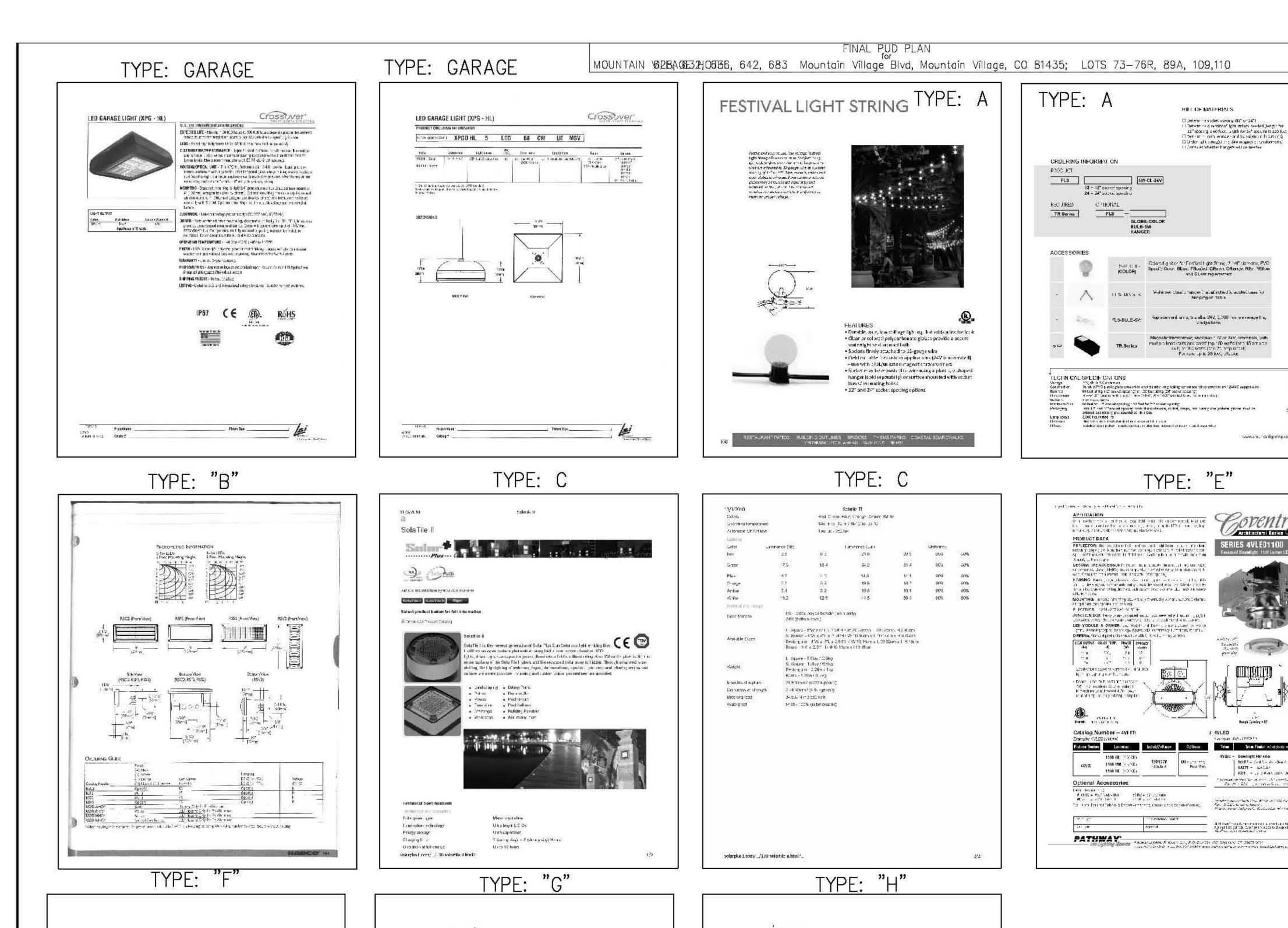
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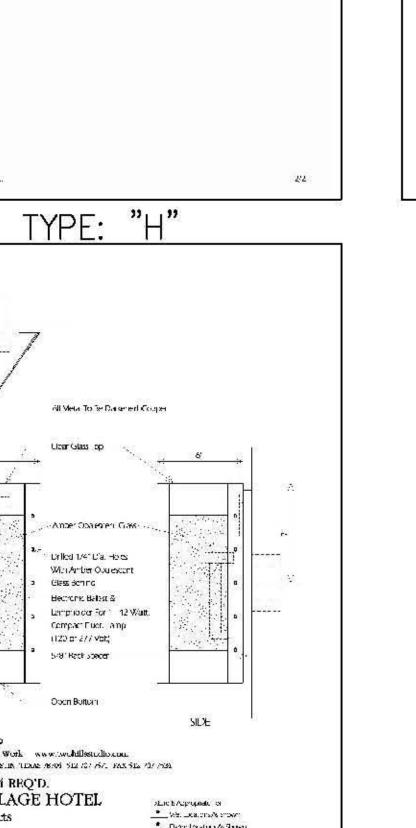
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LEVEL 7 LIGHTING PLAN

project number 08131.100

O 1 LEVEL 7 LIGHTING PLAN SCALE: 1/16" = 1'-0"





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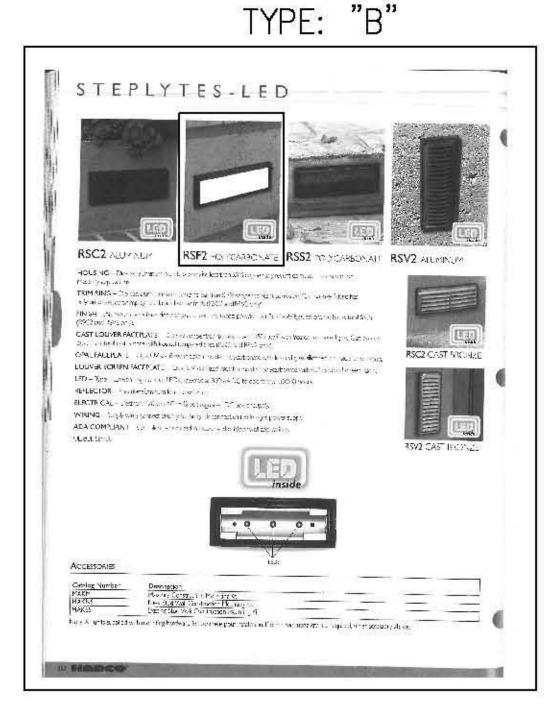
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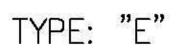
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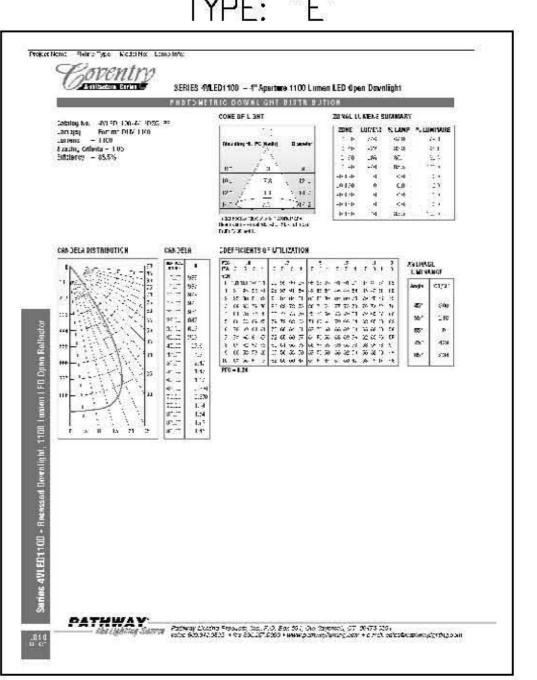
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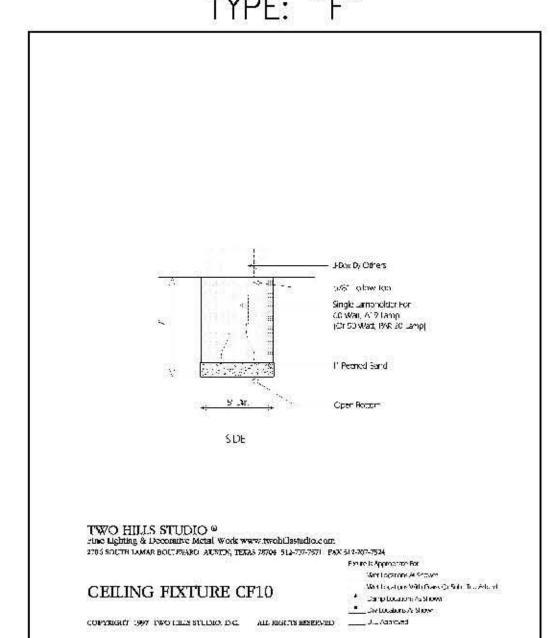
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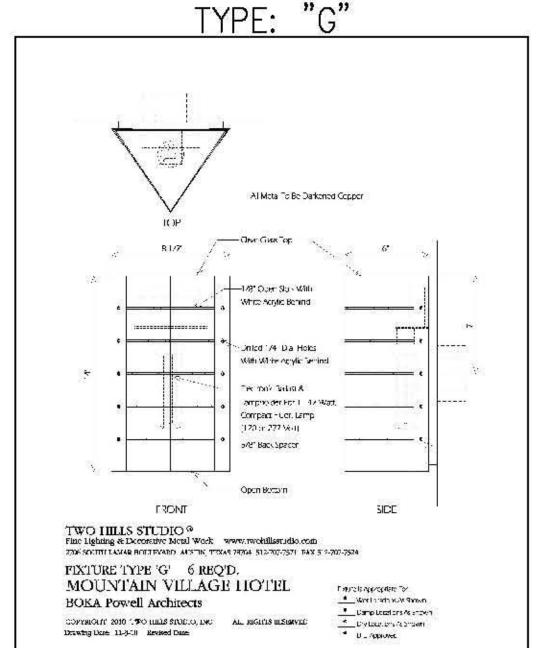
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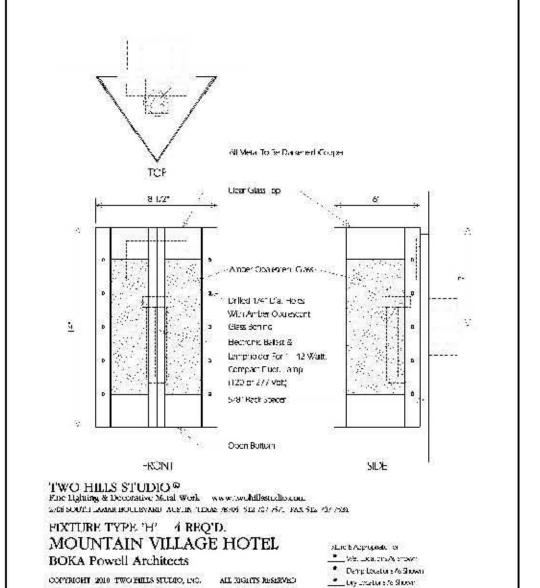
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LIGHTING CUT SHEETS

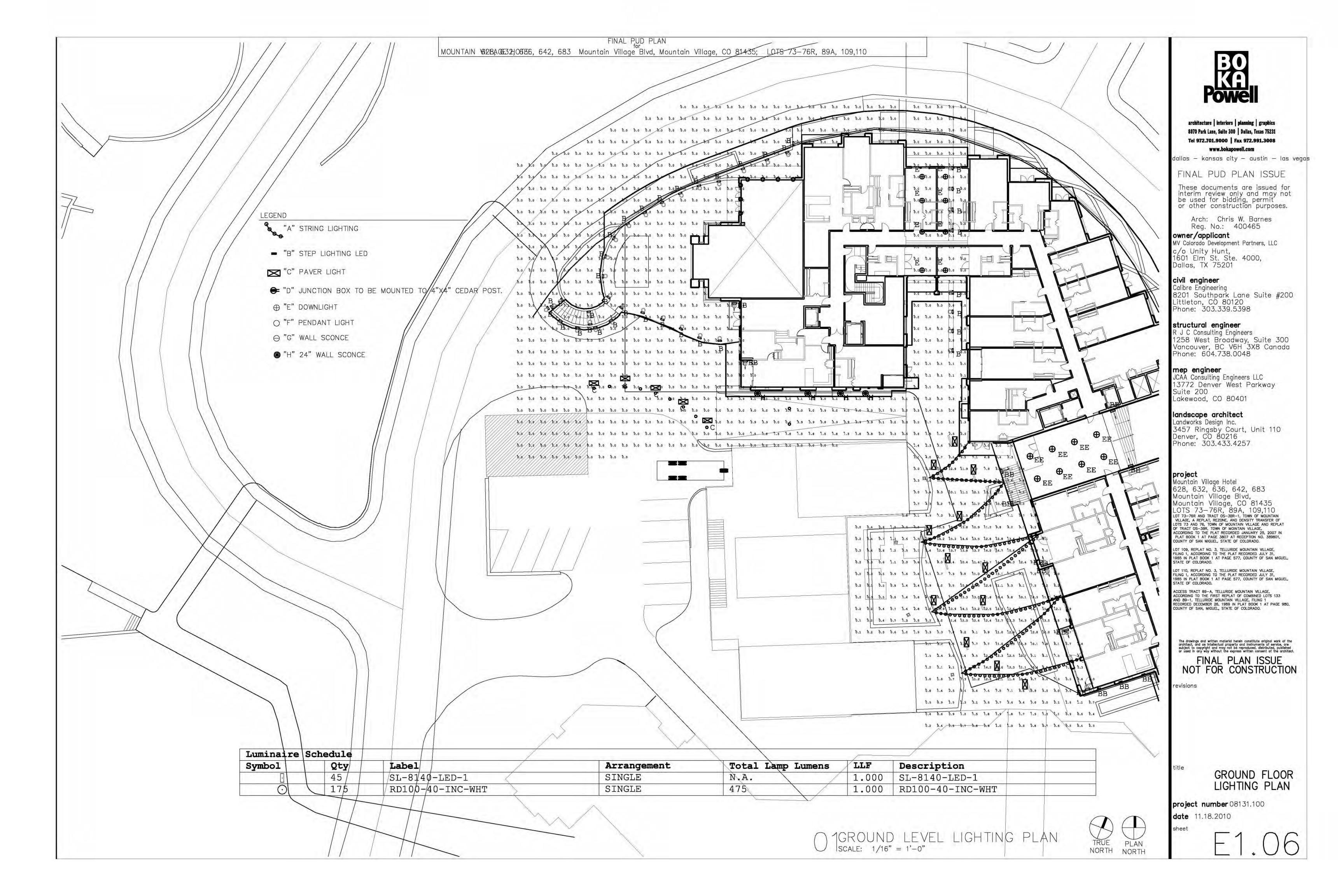
project number 08131.100 date 11.18.2010

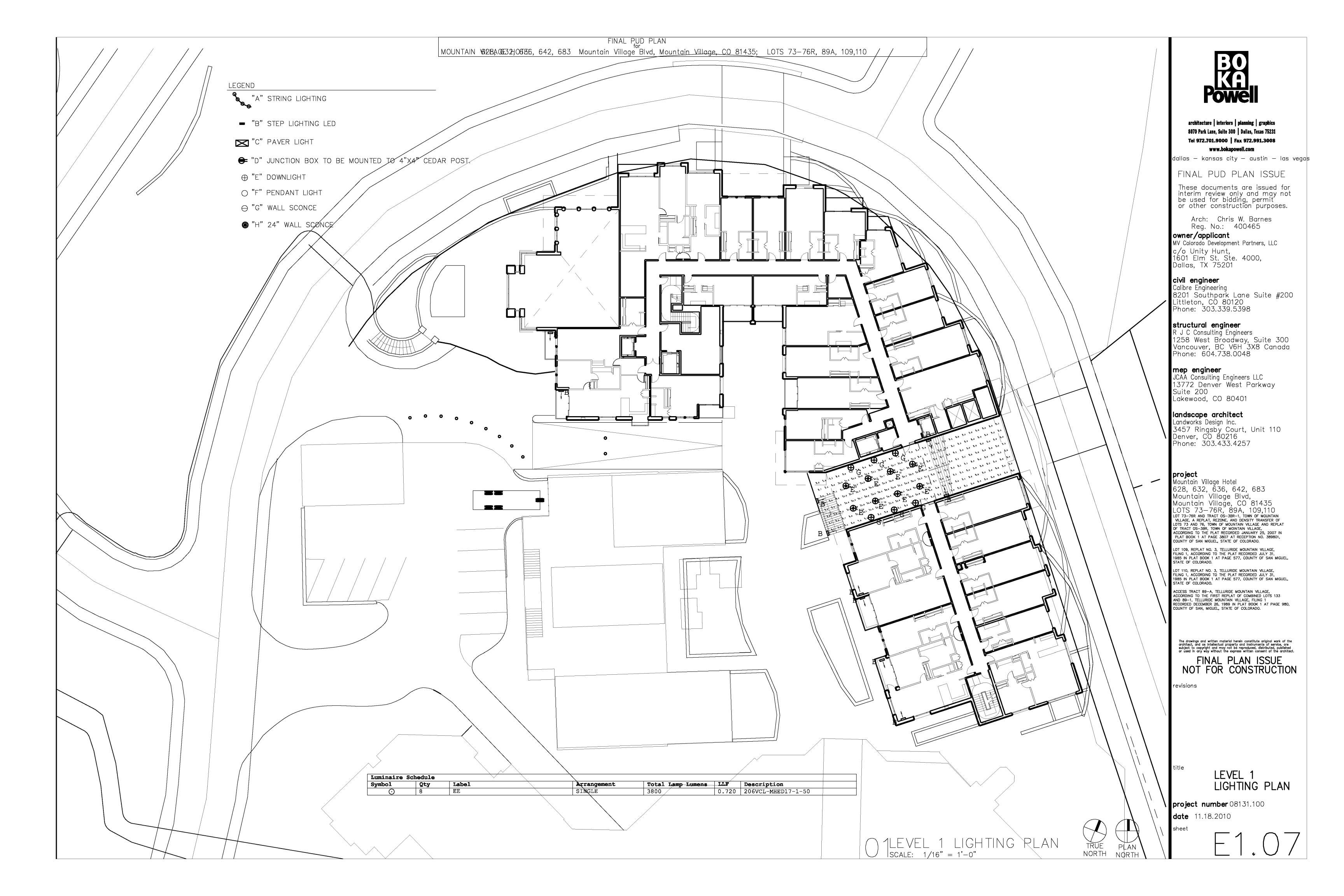


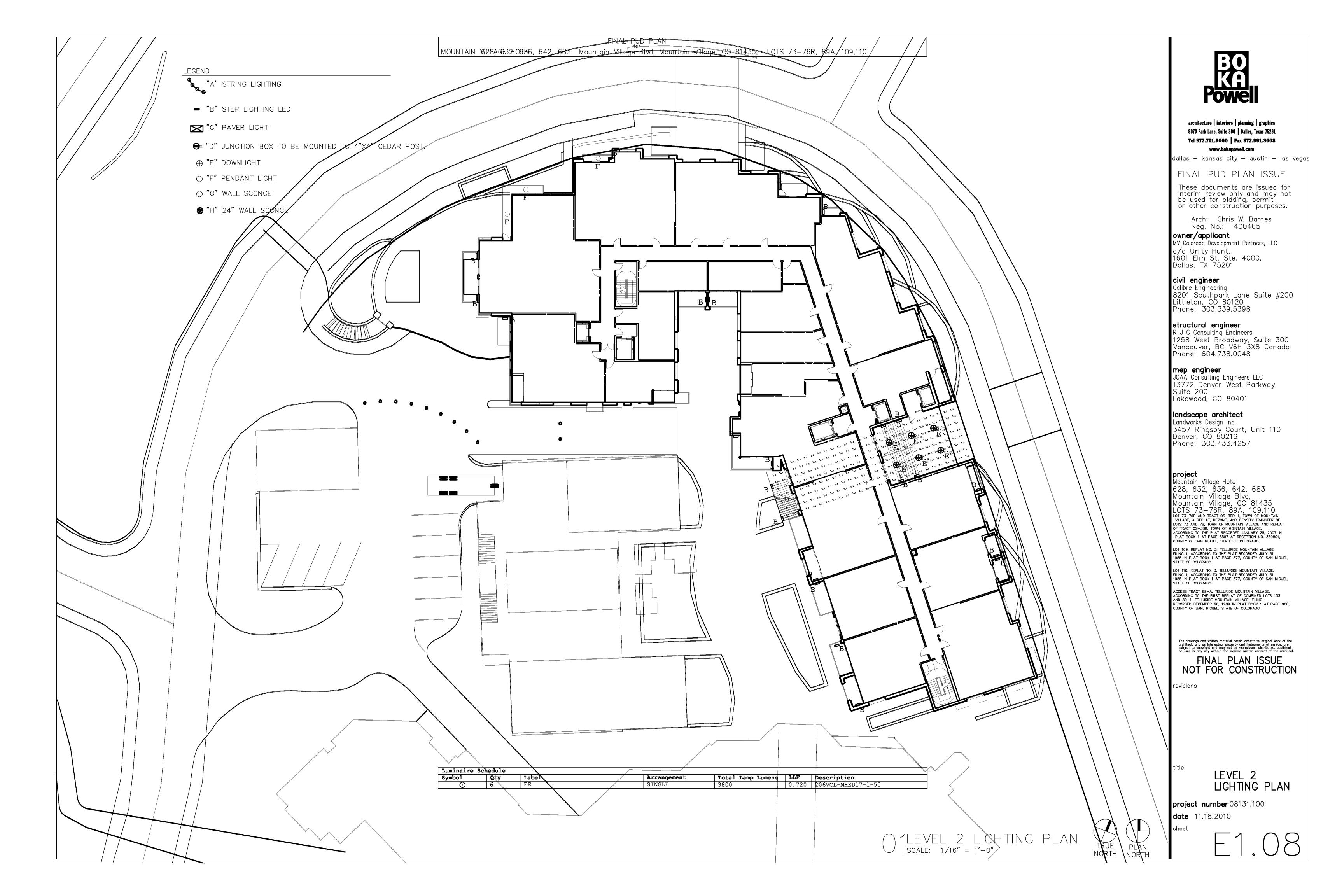


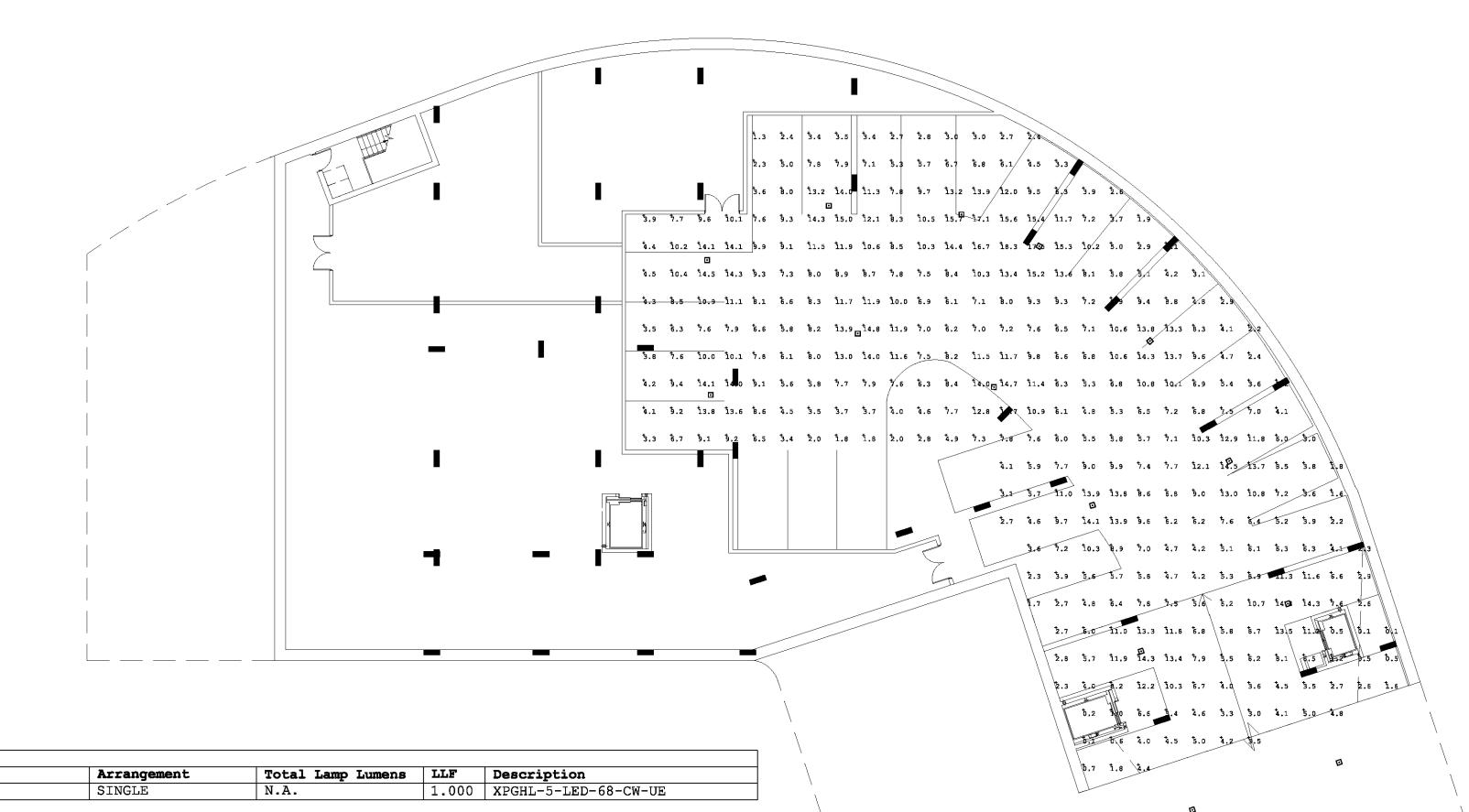


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Calculation Summary
 Avg/Min
 Max/Min

 74.40
 183.00
 CalcType Units Min GARAGE 3 Floor 7.44 18.3 0.1 Illuminance

Luminaire Schedule

Label

XPGHL-5-LED-68-CW-UE



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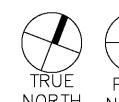
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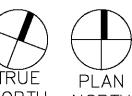
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GARAGE BASEMENT POINT TO POINT

project number 08131.100 **date** 11.18.2010

1GARAGE BASEMENT POINT TO POINT scale: 1/16" = 1'-0"







LLF Description

1.000 XPGHL-5-LED-68-CW-UE

Total Lamp Lumens

6.54 | 14.3 | 0.3 | 21.80 | 47.67

| Calculation Summary | | | | | | | |
|---------------------|-------------|-------|------|------|-----|---------|---------|
| Label | CalcType | Units | Avg | Max | Min | Avg/Min | Max/Min |
| GARAGE 2 Floor | Illuminance | Fc | 7.16 | 17.1 | 0.1 | 71.60 | 171.00 |

Fc

Arrangement

SINGLE

Luminaire Schedule

RAMPS

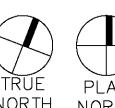
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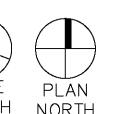
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Illuminance



1LOWER GARAGE POINT TO POINT





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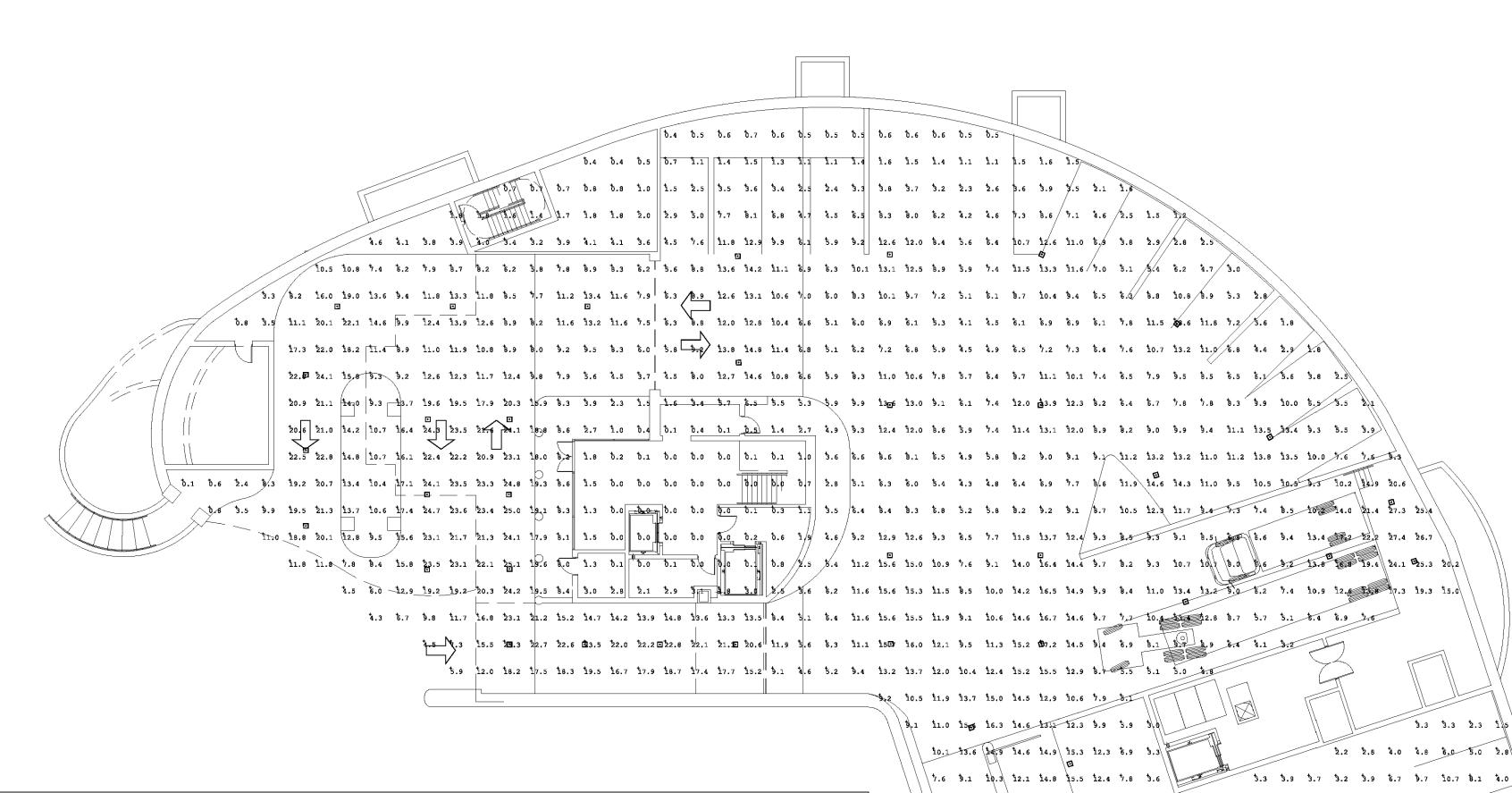
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LOWER GARAGE POINT TO POINT



| Luminaire Sch | edule | | | | | |
|---------------|-------|----------------------|-------------|-------------------|-------|----------------------|
| Symbol | Qty | Label | Arrangement | Total Lamp Lumens | LLF | Description |
| • | 33 | XPGHL-5-LED-68-CW-UE | SINGLE | N.A. | 1.000 | XPGHL-5-LED-68-CW-UE |
| • | 16 | XPG-S-LED-50-CW-UE | SINGLE | N.A. | 1.000 | XPG-S-LED-50-CW-UE |

| Calculation Summary | | | | | | | |
|---------------------|-------------|-------|------|------|-----|---------|---------|
| Label | CalcType | Units | Avg | Маж | Min | Avg/Min | Max/Min |
| GARGE 1_Floor | Illuminance | Fc | 8.21 | 27.4 | 0.0 | N.A. | N.A. |
| RAMPS | Illuminance | Fc | 6.50 | 13.6 | 0.8 | 8.13 | 17.00 |



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> Arch: Chris W. Barnes Reg. No.: 400465

owner/applicant

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revisions

UPPER GARAGE POINT TO POINT

project number 08131.100 **date** 11.18.2010

1UPPER GARAGE POINT TO POINT scale: 1/16" = 1'-0"

\$,1 5.9 5.0 \ \10.7 13.2 13\2 10.1 6.6 3.5 1\6 1.5 2.9 4.8 6.9 7.6 6.1 5.5 5.6 12.5 15.3 10.0 5.2 \$\8\9

\$.9 \$.8 \$3.9 \$4.1 \$3.9 \$3.3 \$3.1 \$5.7 \$5.0 \$\frac{1}{2}\sqrt{3.8} \$7.3 \$10.7 \$10.2 \$7.5 \$\frac{1}{2}\sqrt{4.3} \$5.0 \$5.9 \$6.2 \$5.6 \$\frac{1}{2}\sqrt{3.8}

7.2 5.6 10.1 11.7 13.8 14.3 10.8 5.8 2.7 154 Sanca 158 6.7 5.8 4.8 4.2 5.1 7.4 5.7 5.7 6.5 3.5

4.3 5.1 5.8 8.0 11.1 11.6 8.9 5.4 2.6 New York Ratio -017-004.8 5.2 7.0 6.9 5.4 5.1 9.5 13.1 12.5 8.4 4.0

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1.0 2.4 3.1 3.8 3.8 5.0 7.8 10.3 10.4 7.3 3.5 \2.5 5.0 5.4 11.9 11.0 7.5 4.9 4.9 6.0 5.9 5.8 4.7 2.8

2.3 4.3 8.7 11.2 10.8 7.3 6.1 8.6 12.0 11.9 8.9 4.7 2.2 2.4 4.1 5.2 6.2 6.4 5.5 5.0 6.7 10.0 12.6 11.4 6.6 3.4

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3.8 5.5 1.0 6.9 5.9 4.3 4.1 5.3 7.3 1.1 8.8 5.5 3.5 4.0 6.9 11.4 13.1 10.7 7.1 5.4 6.1 6.6 5.4 3.2 1.6

3.3 5.0 6.3 6.6 6.1 5.1 6.5 10.1 12.9 11.9 1.5 4.2 5.4 5.3 6.1 6.9 1.2 5.3 4.6 5.0 5.8 4.5 1.8

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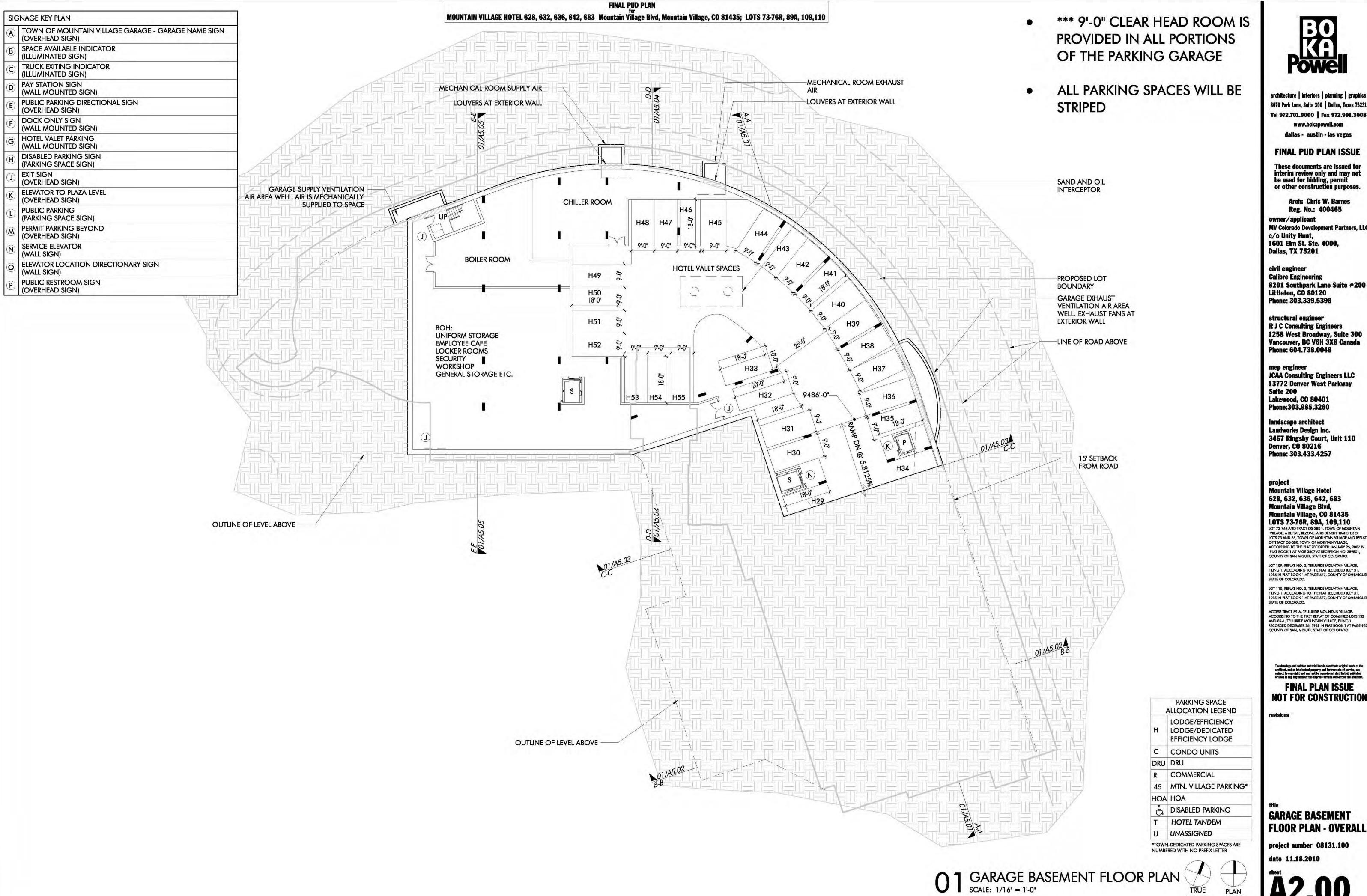
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1.4 2.8 4.8 6.5 7.0 5.7 6.1 5.4 13.2 13.2 15.3 4.5 2.6 3.7 6.4 7.9 7.2 5.8 4.3 4.1 5.5 7.1 1.5 7.8 4.9

2.0 2.0 3.4 4.5 5.9 6.5 6.5 4.5 2.9



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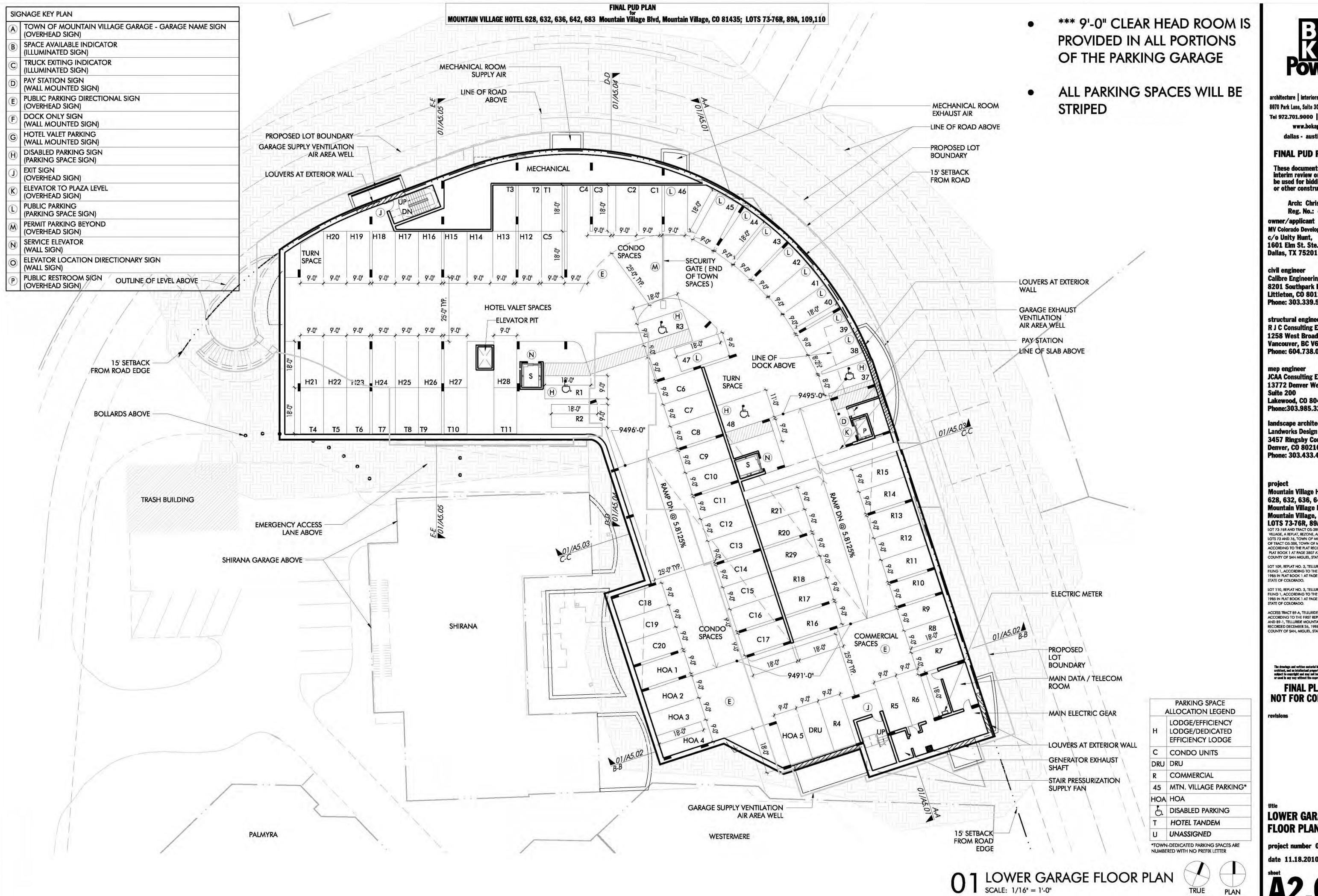
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GARAGE BASEMENT FLOOR PLAN - OVERALL



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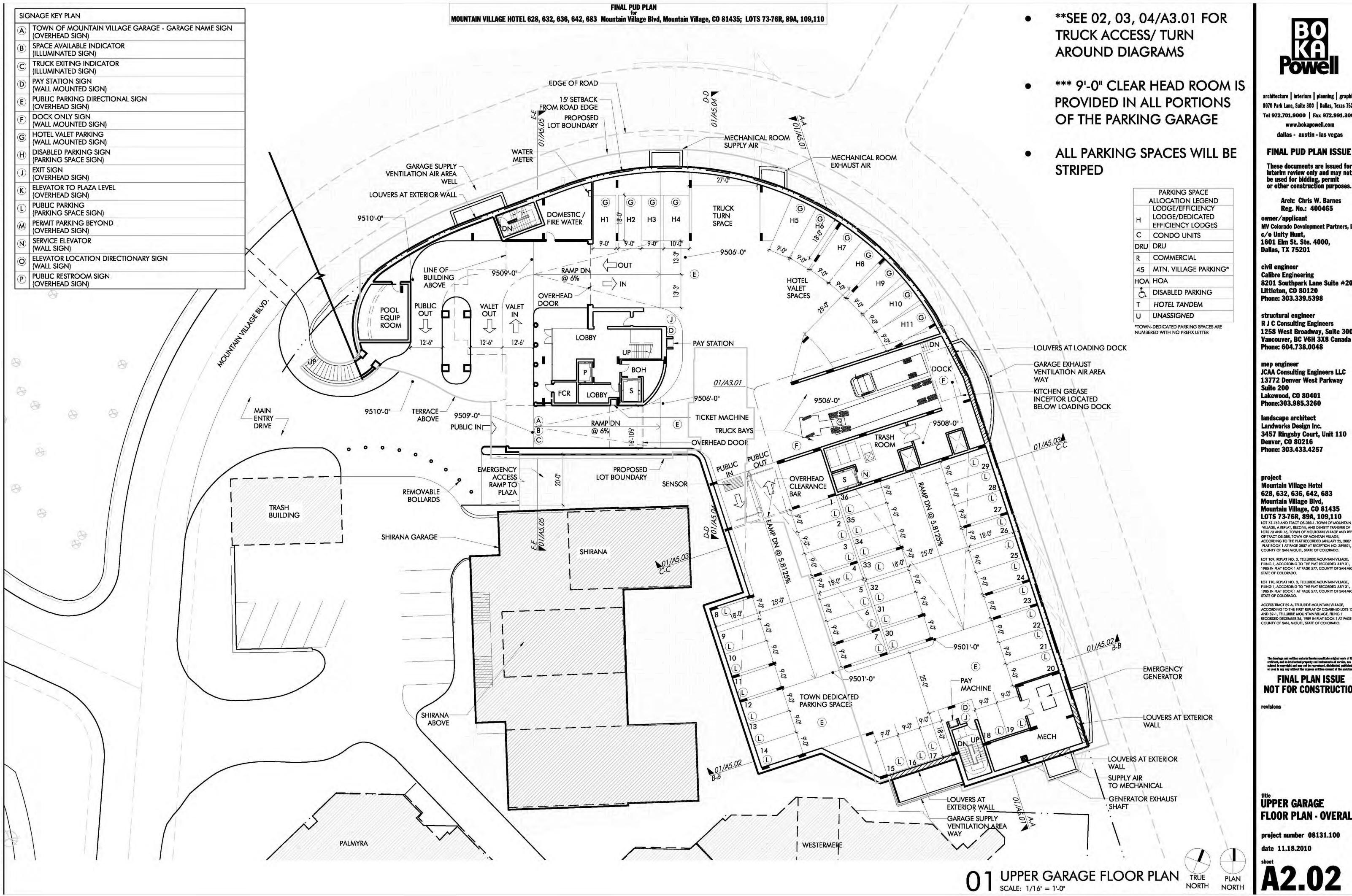
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LOWER GARAGE FLOOR PLAN - OVERALL



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JCAA Consulting Engineers LLC 13772 Denver West Parkway

3457 Ringsby Court, Unit 110

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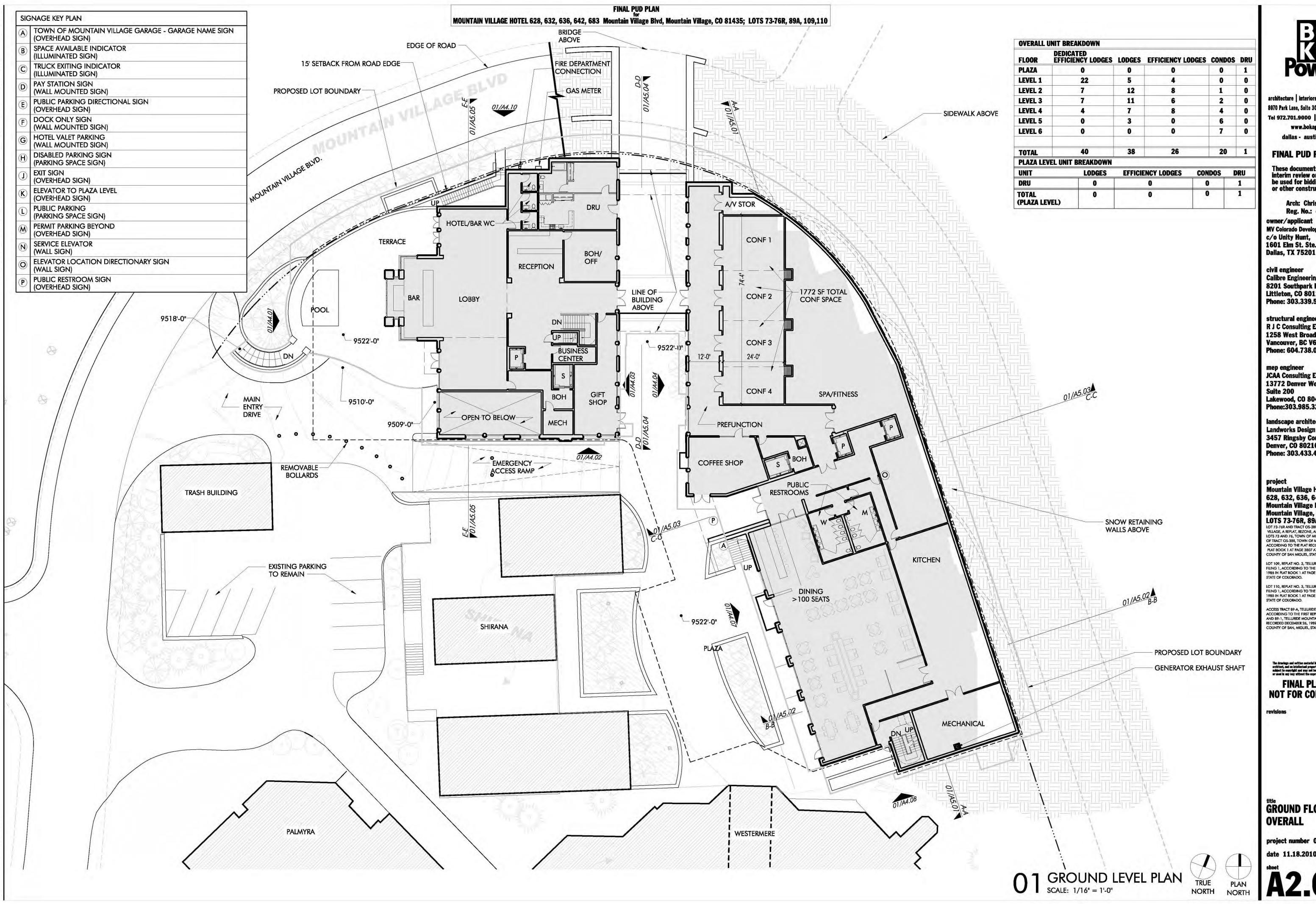
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FLOOR PLAN - OVERALL





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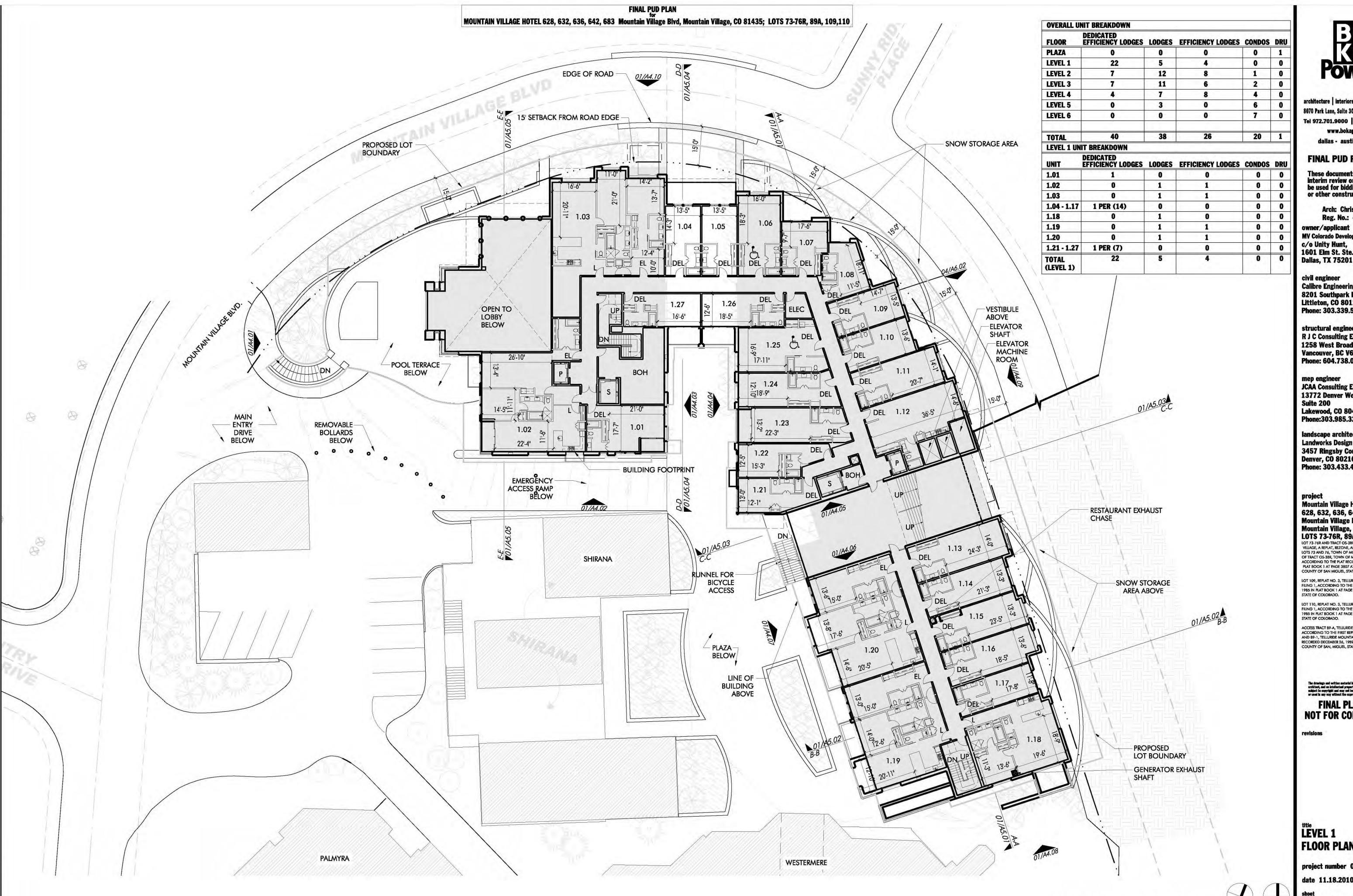
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GROUND FLOOR PLAN -





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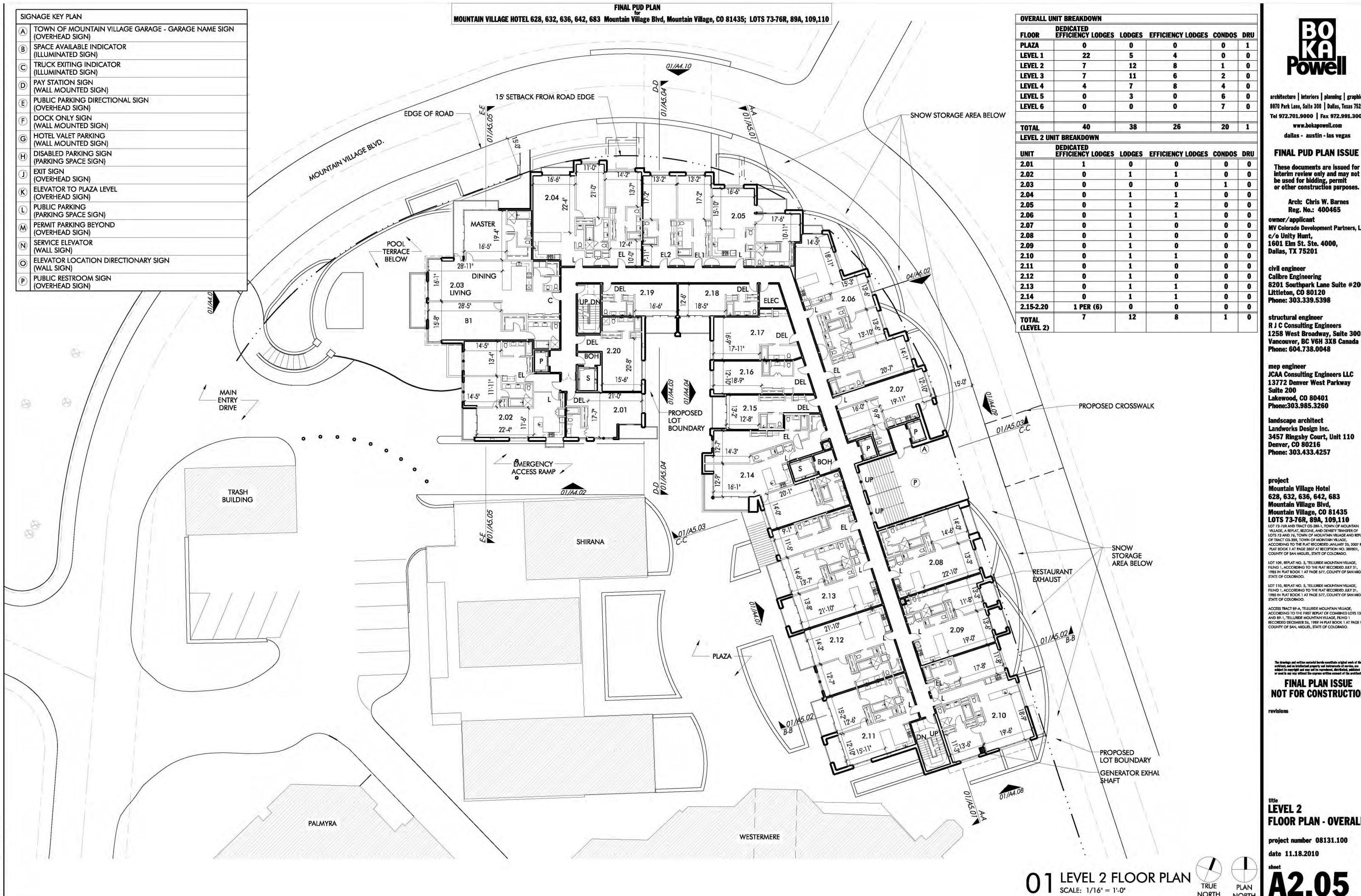
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LEVEL 1 FLOOR PLAN - OVERALL

project number 08131.100 date 11.18.2010

O 1 LEVEL 1 FLOOR PLAN SCALE: 1/16" = 1'-0"



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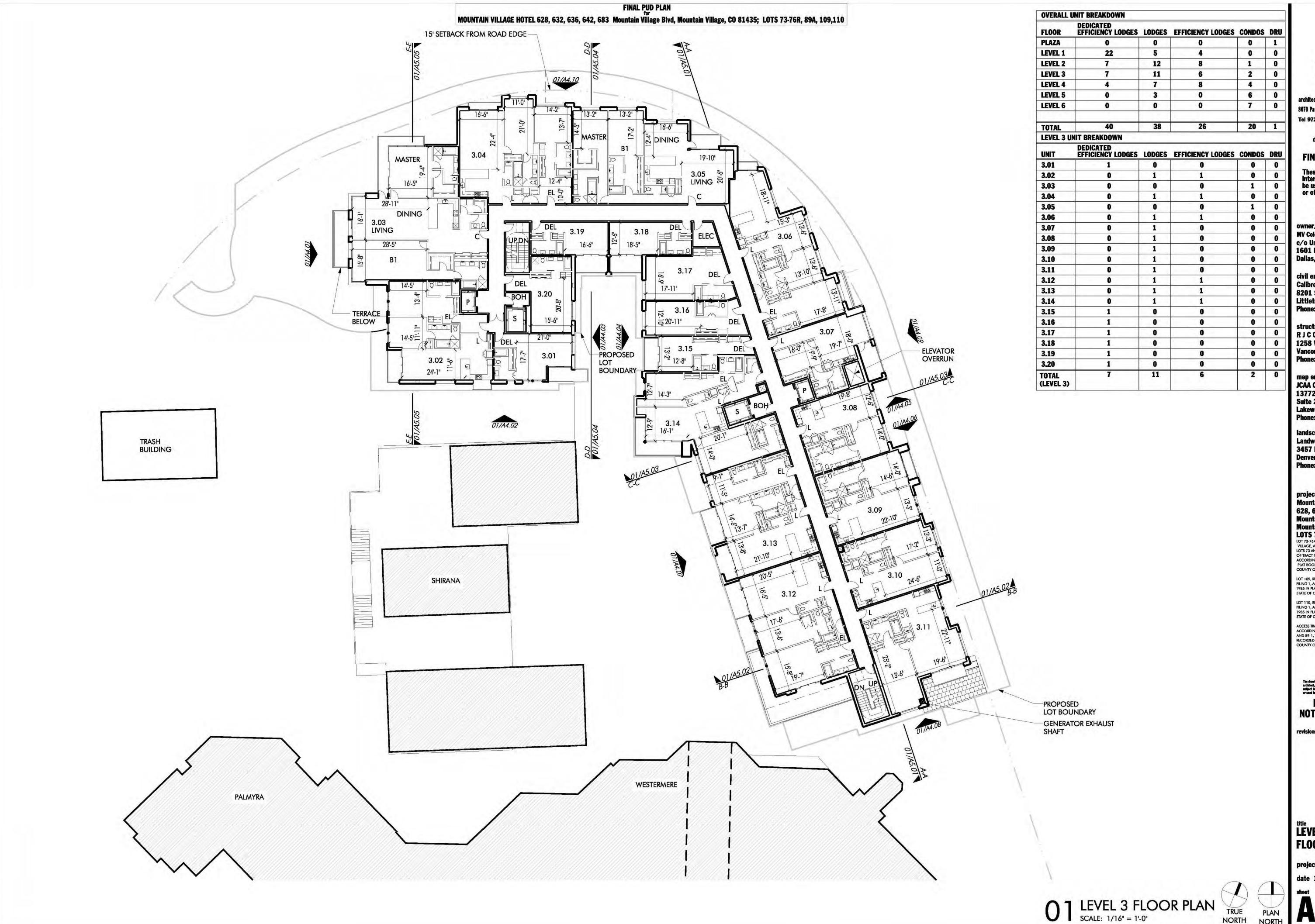
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FLOOR PLAN - OVERALL



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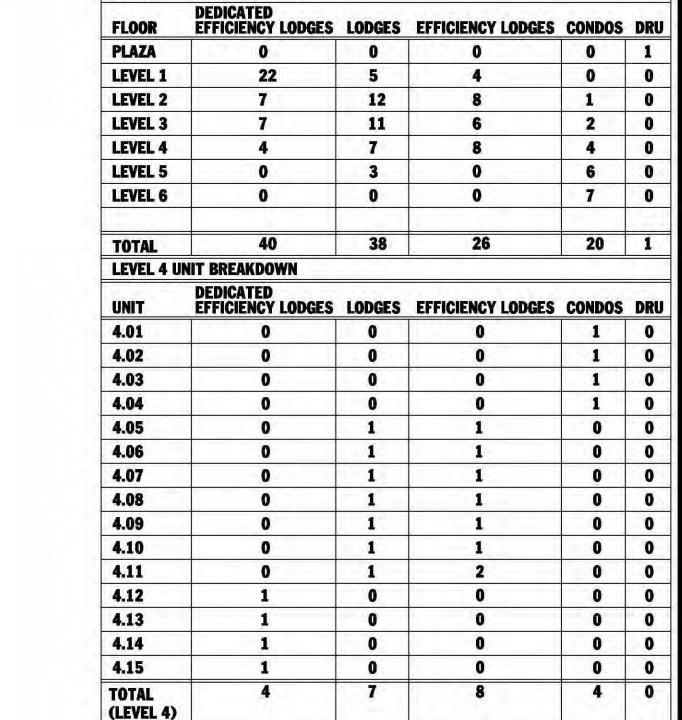
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LEVEL 3 FLOOR PLAN - OVERALL



OVERALL UNIT BREAKDOWN

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Mountain Village Blvd,
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LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
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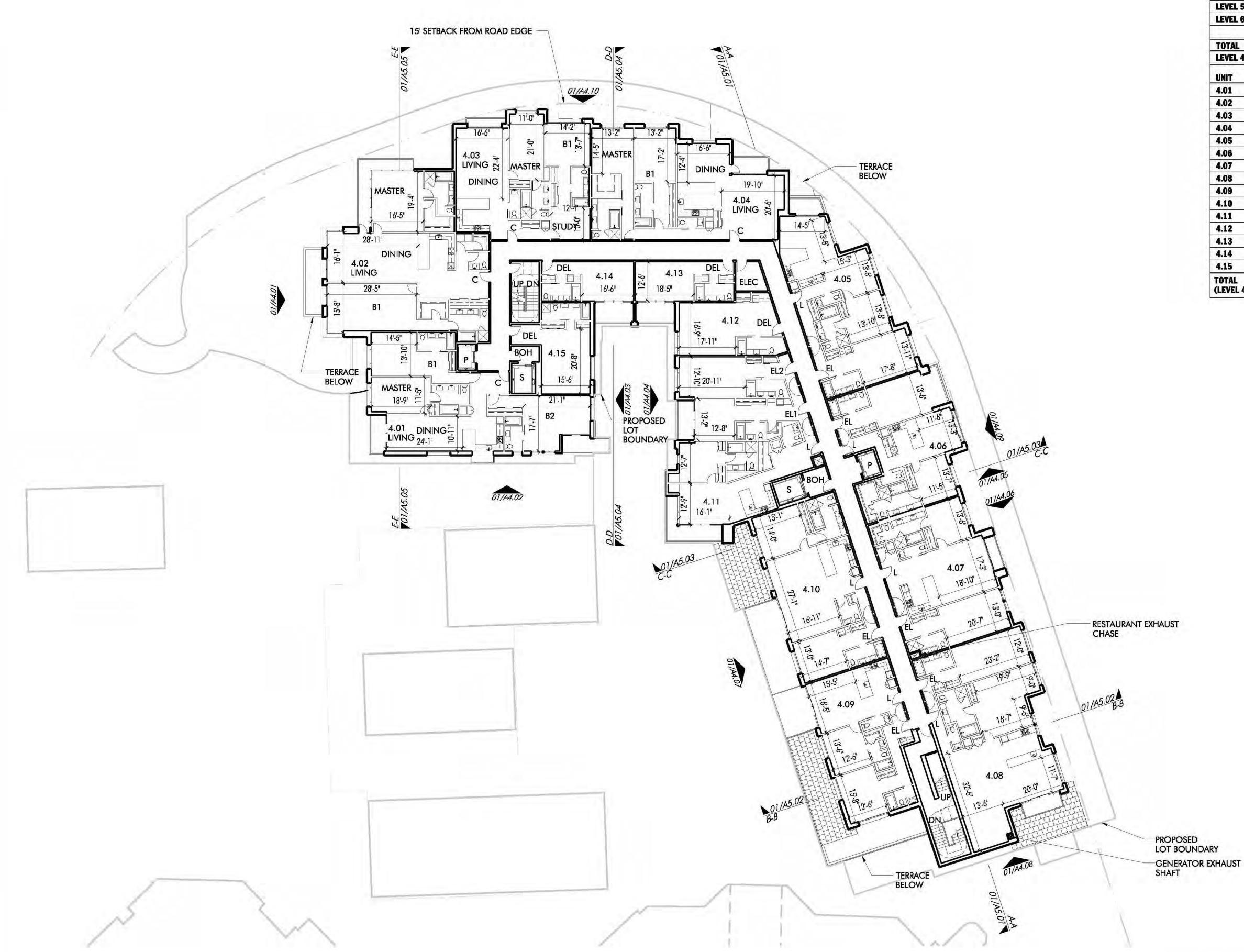
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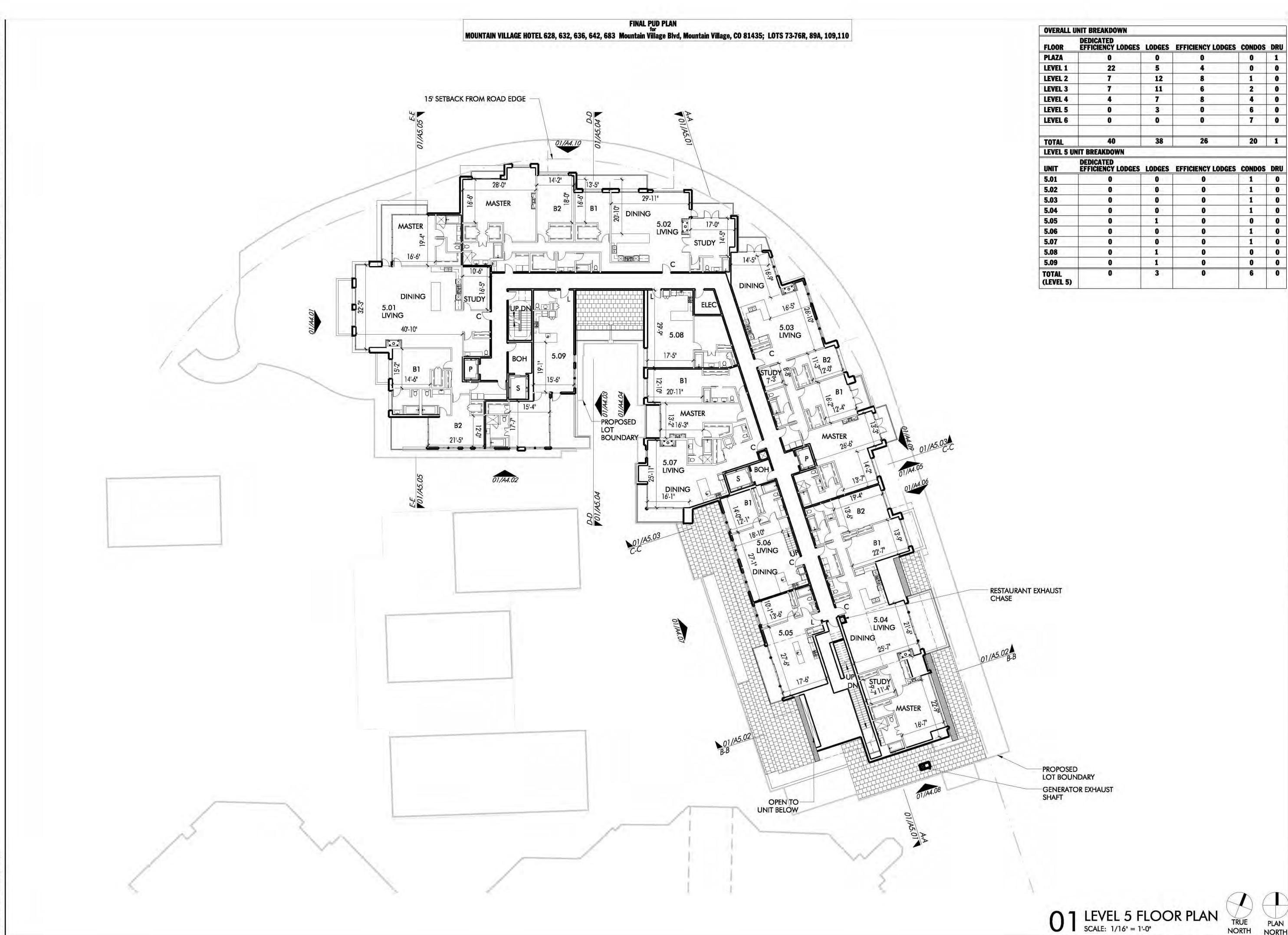
revisions

LEVEL 4 FLOOR PLAN - OVERALL

project number 08131.100 date 11.18.2010

A2.07





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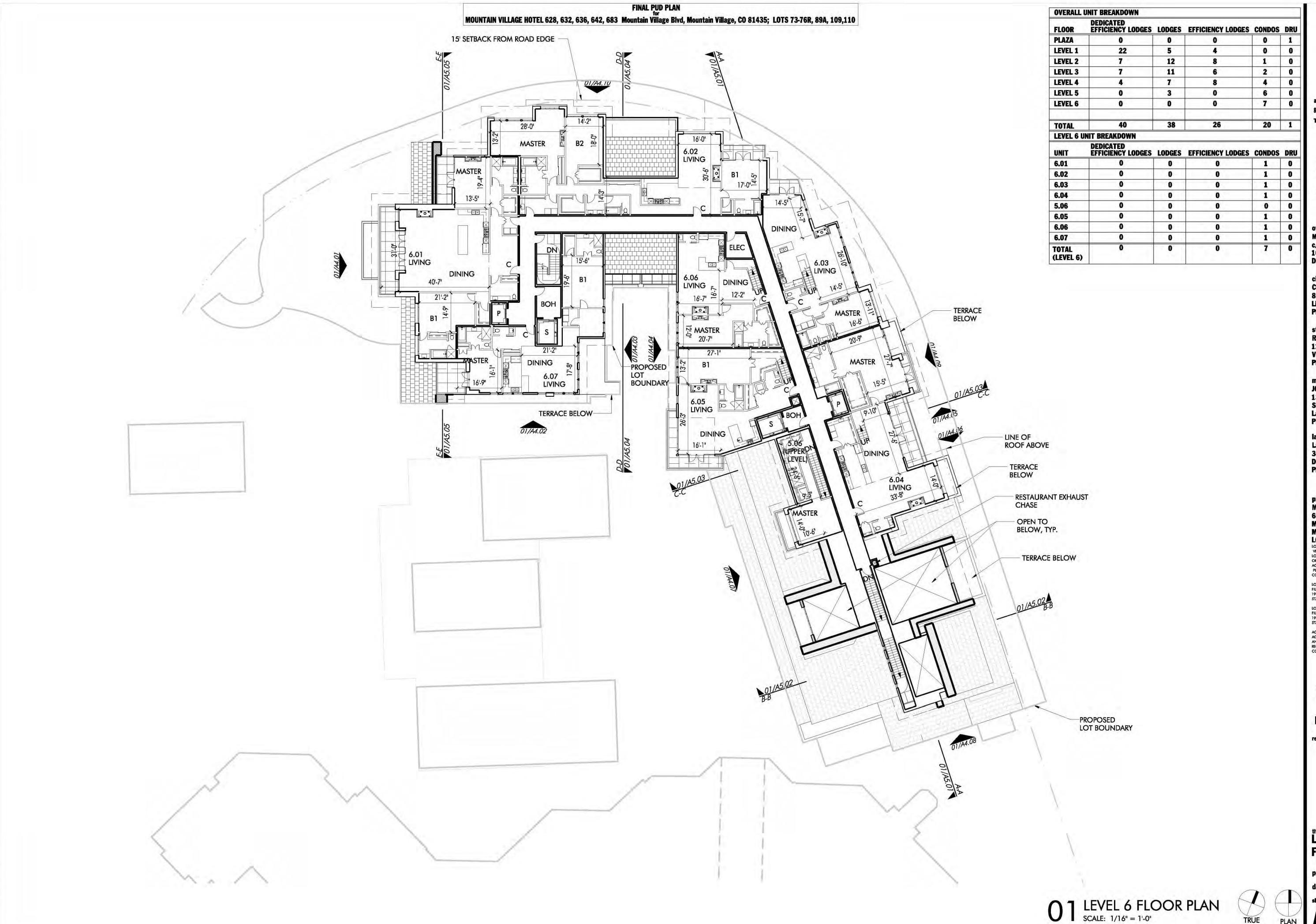
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LEVEL 5 FLOOR PLAN - OVERALL

project number 08131.100 date 11.18.2010

A2.08



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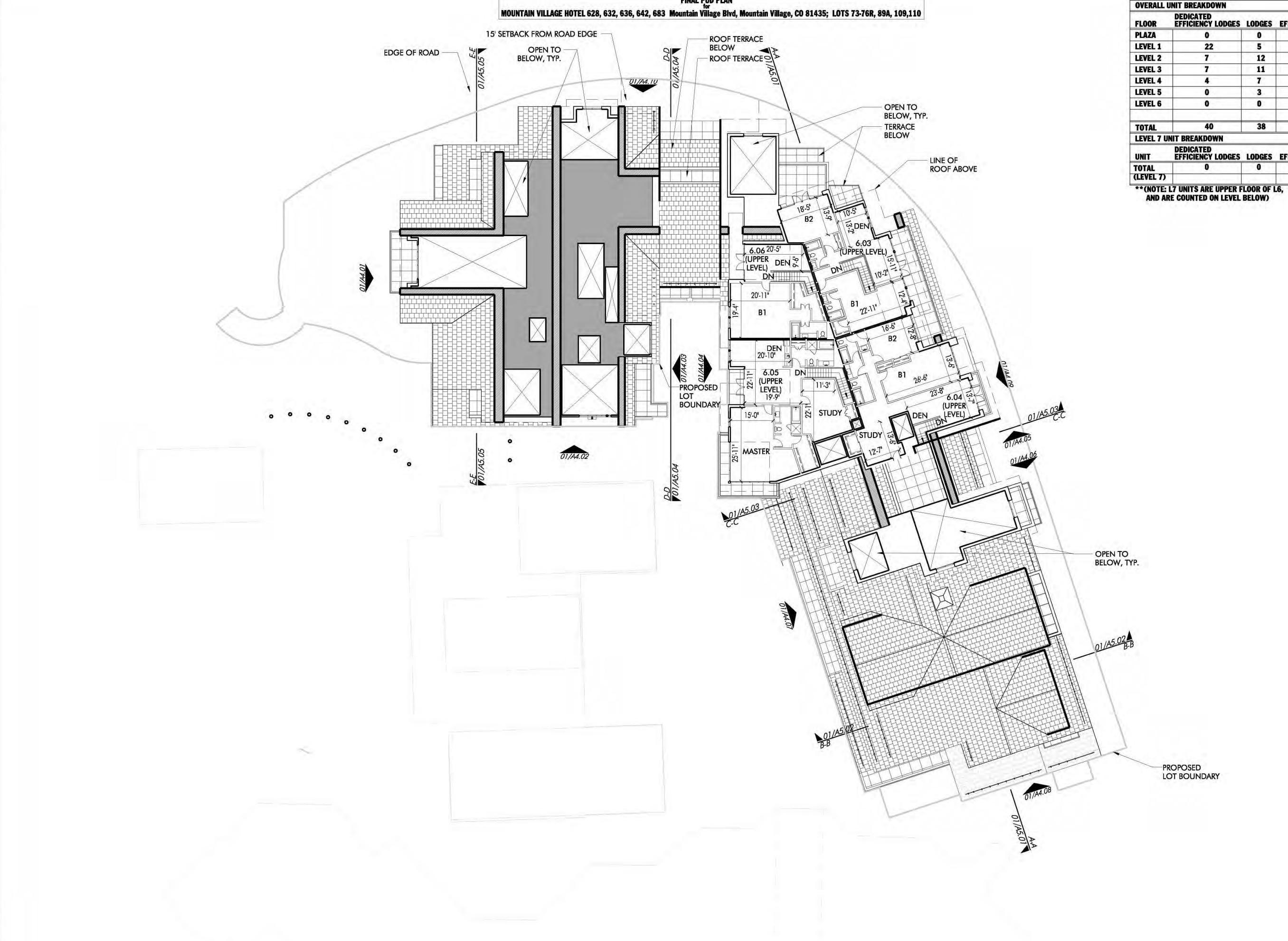
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LEVEL 6 FLOOR PLAN - OVERALL





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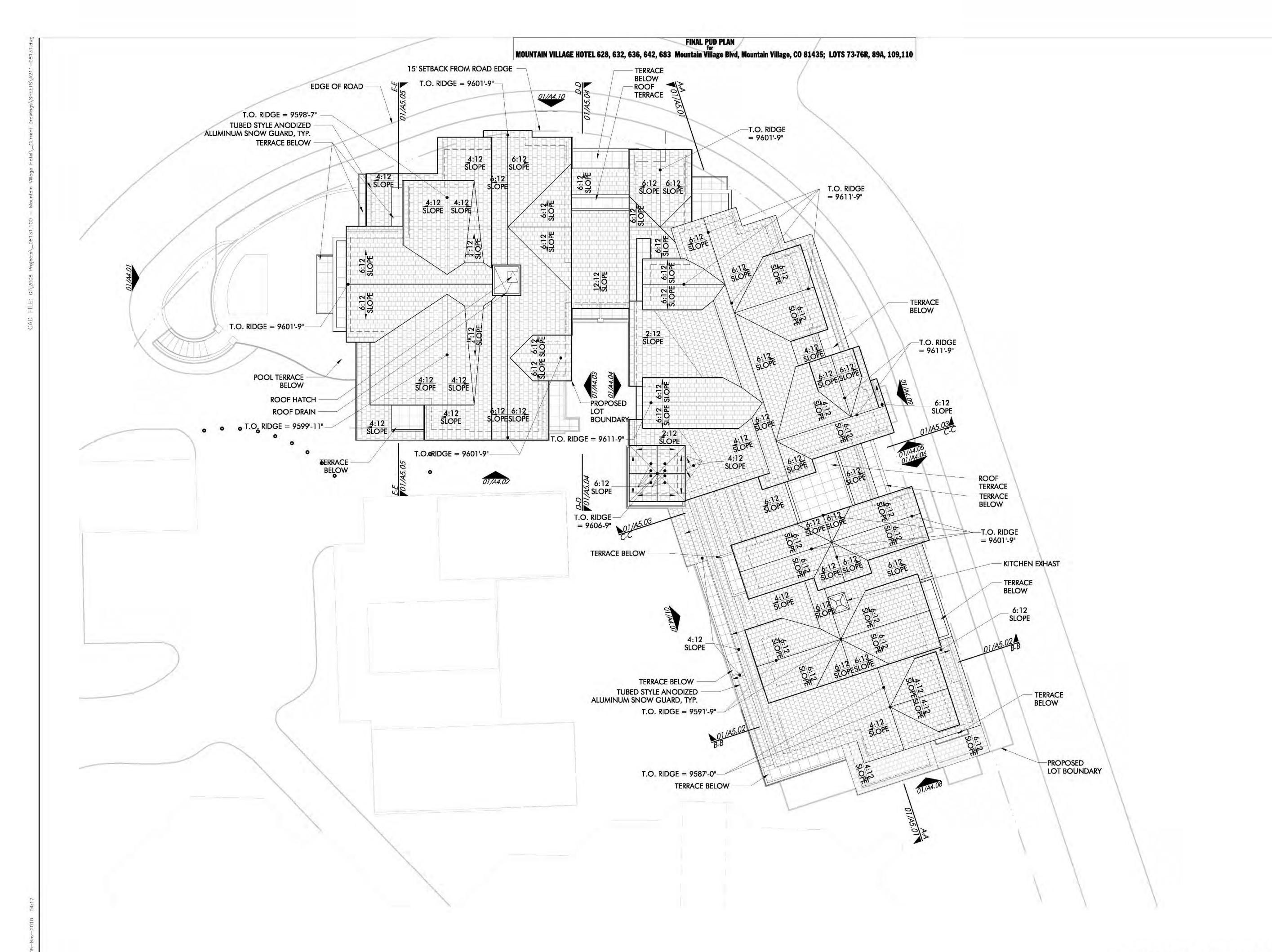
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LEVEL 7 FLOOR PLAN - OVERALL





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ROOF PLAN - OVERALL

project number 08131.100

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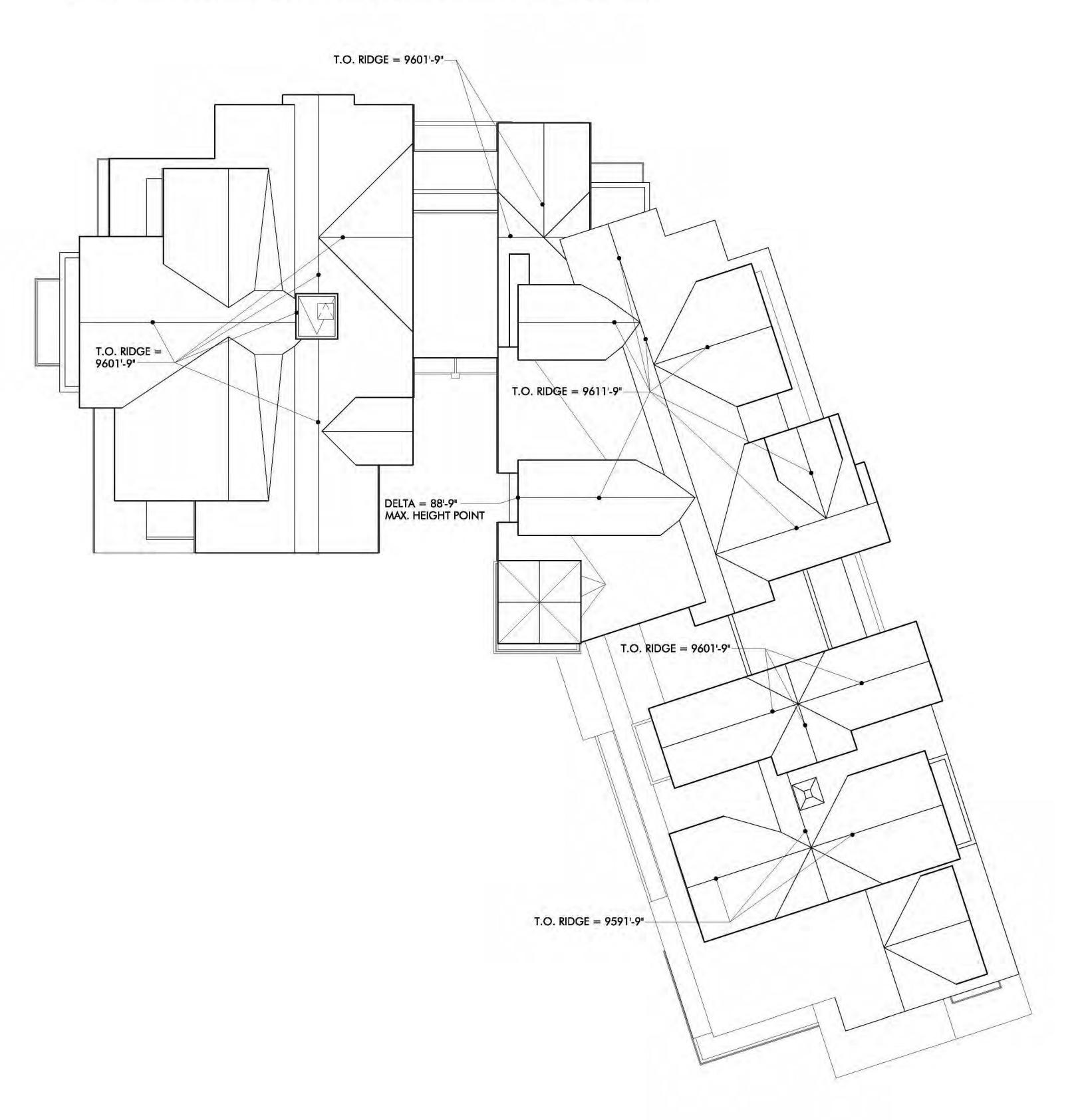
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AVERAGE HEIGHT TARGA PLAN

project number 08131.100 date 11.18.2010

PLAN A2.1





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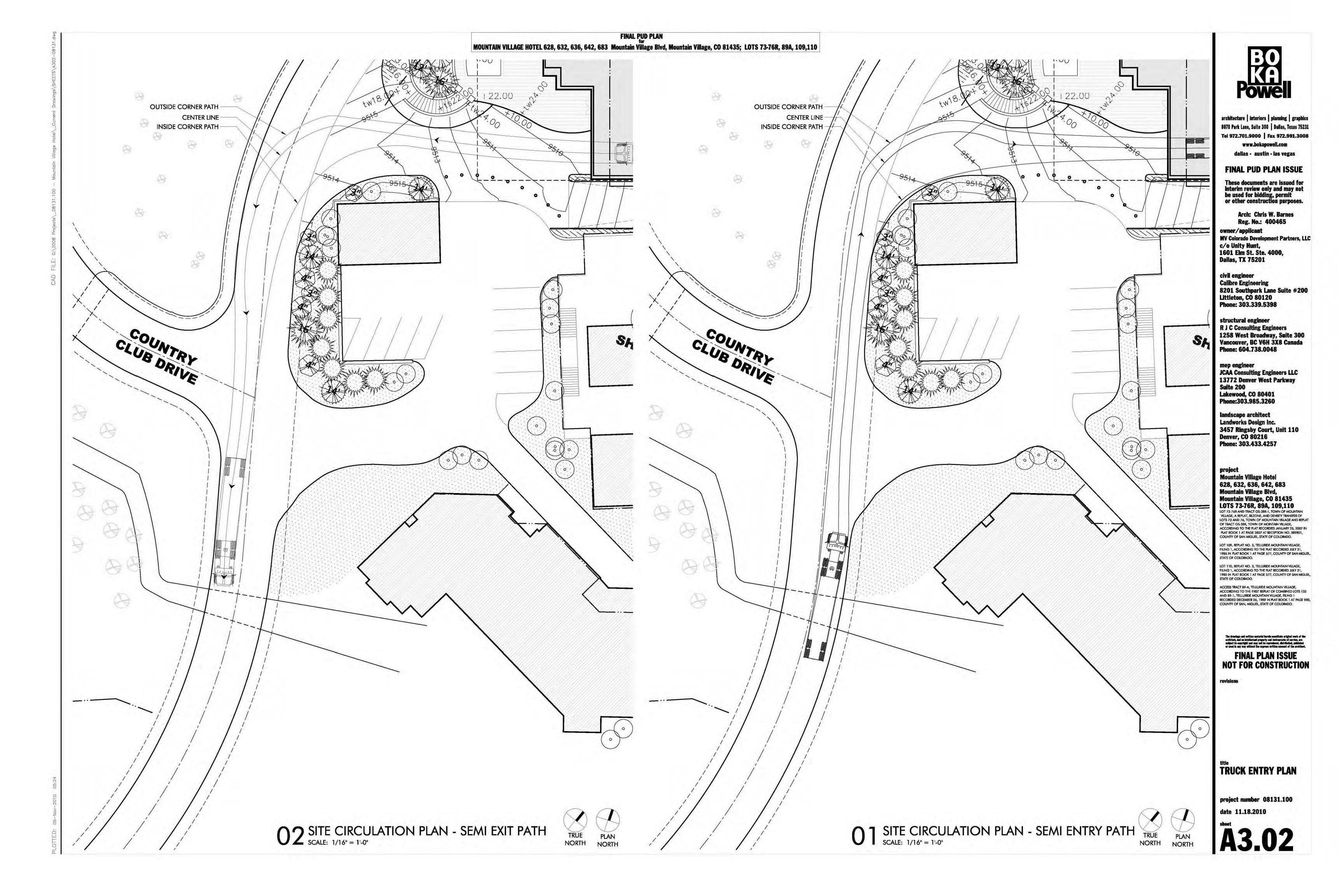
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MAXIMUM HEIGHT PLAN

project number 08131.100



GENERAL NOTES

ALL EXPOSED CONCRETE TO BE CLAD
 WITH ELASTOMERIC COATING

2. ALL GUTTERS & DOWNSPOUTS TO BE

WINDOW MULLION COLOR

KYNAR COATED METAL TO MATCH

| Α | В | С | D | E | F | G | H | - I | 1 | K | L | W | N | 0 | P | Q | R | S | T | U | ٧ | W | X | Υ | Z | AA | BB | CC | DD | EE | FF | GG | HH | 1 | |
|--------|--------|--------|---------|--------|---------|--------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|----------|
| 74'-5" | 74'-5" | 73'-2" | 24'-4" | 71'-6" | 80'-10" | 78'-2" | 77'-7" | 76'-3" | 73'-10" | 62'-5" | 83'-7" | 83'-7" | 83'-7" | 82'-2" | 82'-2" | 66'-5" | 77'-2" | 63'-10" | 66'-4" | 57'-21" | 50'-11" | 51'-8" | 57'-8" | 60'-4" | 48'-0" | 37'-3" | 45'-3" | 55'-6" | 46'-6" | 66'-5" | 54'-9" | 67'-7" | 76'-11" | 71'-2" | |
| IJ | KK | ш | MM | NN | 00 | PP | QQ | RR | SS | T | UU | VV | ww | | | | | | | | | | | | | | | | | | | | | | AVERAGE |
| 71'-2" | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6" | 67'-11" | 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | | | | | | | | | | | | | | | | | 7 | | | 65'-2.9" |

2860

MATERIAL AREA CALCULATION

STUCCO

PORTE COCHERE BEYOND

1579

3405

2098

| ELEVATION | (01/A4.01) | ELEVATION (01/A4.02) | ELEVATION LOOKING EAST (01/A4.03) | ELEVATION LOOKING WEST (01/A4.04) | ELEVATION LOOKING NORTH (01/A4.05) | ELEVATION LOOKING SOUTH (01/A4.06) | ELEVATION (01/A4.07) | (01/A4.08) | (01/A4.09) | (01/A4.10) | TOTALS | % |
|---------------------------|------------|-------------------------|-----------------------------------|-----------------------------------|------------------------------------|------------------------------------|-------------------------|------------|------------|------------|--------|-------|
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
| MATERIAL TYPES | | | | | | | | | | | | |
| STONE ** | 1952 | 2410 | 1179 | 1047 | 1338 | 1148 | 1346 | 1590 | 3057 | 3543 | 18,610 | 27.04 |
| FENESTRATION | 3137 | 2893 | 1229 | 2644 | 202 | 62 | 2711 | 704 | 4344 | 4067 | 21,993 | 31.96 |
| WOOD | 1159 | 1598 | 516 | 806 | 0 | 0 | 506 | 95 | 1271 | 1534 | 7,485 | 10.88 |
| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |

562

1089

** 1616 SF (8.68%) OF TOTAL STONE AREA IS INCORPORATED IN SITE RETAINING WALLS ADJACENT TO AND INTEGRAL OF THE BUILDING

3316

1928

20,710 30.09

2136

1737

01/A4.01-B 01/A4.01-A COPPER CAP FLASHING TUBED STYLE ANODIZED ALUMINUM SNOW GUARD 73.2 74'-5' LEVEL 7 ELEV = 9592'-8" **TILE ROOF** TILE ROOF **TUBED STYLE ANODIZED** WOOD ROOF FASCIA ALUMINUM SNOW GUARD LEVEL 6 ELEV = 9582'-8" WOOD ROOF FASCIA HAND-HEWED MILLED LOG CONTROL JOINT, TYP. STUCCO LEVEL 5 ELEV = 9572'-8" CEDAR SIDING KYNAR COATED ALUMINUM **CEDAR SIDING** CLAD WOOD DOORS LEVEL 4 ELEV = 9562'-8" KYNAR COATED ALUMINUM KYNAR COATED ALUMINUM CLAD WOOD WINDOWS **CLAD WOOD WINDOW** LEVEL 3 ELEV = 9552'-8" KYNAR COATED ALUMINUM HALF ROUND WOOD RAILING **CEDAR SIDING** STUCCO STONE CONTROL JOINT, TYP. LEVEL 2 ELEV = 9542'-8" KYNAR COATED ALUMINUM CLAD WOOD DOOR STONE LINTEL WOOD RAILING HAND-HEWED MILLED LOG LEVEL 1 ELEV = 9532'-8" STONE GRADE STONE LINTEL PLAZA ELEV = 9522'-0" STONE RETAINING WALL HAND-HEWED MILLED LOG WOOD PANEL OVERHEAD GARAGE DOOR POOL DECK BEYOND GRADE GARAGE 1 ELEV = 9510'-0"

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FINAL PUD PLAN ISSUE

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Arch: Chris W. Barnes
Reg. No.: 400465

owner/applicant
MV Colorado Development Partners, LLC
c/o Unity Hunt,
1601 Elm St. Ste. 4000,
Dallas, TX 75201

civil engineer
Calibre Engineering
8201 Southpark Lane Suite #200
Littleton, CO 80120
Phone: 303.339.5398

structural engineer R J C Consulting Engineers 1258 West Broadway, Suite 300 Vancouver, BC V6H 3X8 Canada Phone: 604.738.0048

mep engineer
JCAA Consulting Engineers LLC
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Phone:303.985.3260

landscape architect
Landworks Design Inc.
3457 Ringsby Court, Unit 110
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Phone: 303.433.4257

project
Mountain Village Hotel
628, 632, 636, 642, 683
Mountain Village Blvd,
Mountain Village Blvd,
Mountain Village, CO 81435
LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF
LOTS 73 AND 76, TOWN OF MOUNTAIN VILLAGE AND REPLAT
OF TRACT OS-3BR, TOWN OF MOUNTAIN VILLAGE,
ACCORDING TO THE PLAT RECORDED JANUARY 25, 2007 IN
PLAT BOOK 1 AT PAGE 3807 AT RECEPTION NO. 389801,
COUNTY OF SAN MIGUEL, STATE OF COLORADO.

LOT 109, REPLAT NO. 3, TELLURIDE MOUNTAIN VILLAGE, FILING 1, ACCORDING TO THE PLAT RECORDED JULY 31, 1985 IN PLAT BOOK 1 AT PAGE 577, COUNTY OF SAN MIGUEL, STATE OF COLORADO.

LOT 110, REPLAT NO. 3, TELLURIDE MOUNTAIN VILLAGE, FILING 1, ACCORDING TO THE PLAT RECORDED JULY 31, 1985 IN PLAT BOOK 1 AT PAGE 577, COUNTY OF SAN MIGUEL, STATE OF COLORADO.

ACCESS TRACT 89-A, TELLURIDE MOUNTAIN VILLAGE,
ACCORDING TO THE FIRST REPLAT OF COMBINED LOTS 133
AND 89-1, TELLURIDE MOUNTAIN VILLAGE, FILING 1
RECORDED DECEMBER 26, 1989 IN PLAT BOOK 1 AT PAGE 980,
COUNTY OF SAN, MIGUEL, STATE OF COLORADO.

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FINAL PLAN ISSUE NOT FOR CONSTRUCTION

revisions

EXTERIOR ELEVATION OVERALL

project number 08131.100 date 11.18.2010

A4.01

| Α | В | С | D | E | F | G | Н | | J | K | L | M | N | 0 | P | Q | R | S | T | U | ٧ | W | X | Y | Z | AA | BB | CC | DD | EE | FF | GG | HH | II | |
|--------|--------|--------|---------|--------|---------|--------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|----------|
| 74'-5" | 74'-5" | 73'-2" | 24'-4" | 71'-6" | 80'-10" | 78'-2" | 77'-7" | 76'-3" | 73'-10" | 62'-5" | 83'-7" | 83'-7" | 83'-7" | 82'-2" | 82'-2" | 66'-5" | 77'-2" | 63'-10" | 66'-4" | 57'-21" | 50'-11" | 51'-8" | 57'-8" | 60'-4" | 48'-0" | 37'-3" | 45'-3" | 55'-6" | 46'-6" | 66'-5" | 54'-9" | 67'-7" | 76'-11" | 71'-2" | |
| IJ | KK | LL | MM | NN | 00 | PP | QQ | RR | SS | П | UU | VV | WW | | | | | | | | | | | | | | | | | | | | | | AVERAGE |
| 71'-2" | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6" | 67'-11" | 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | | | | | | | | | | | | | | | | | | | | 65'-2.9" |

MATERIAL AREA CALCULATION

| GEI | NERAL NOTES |
|-----|--------------------------------|
| ĺ. | ALL EXPOSED CONCRETE TO BE CLA |

WITH ELASTOMERIC COATING

2. ALL GUTTERS & DOWNSPOUTS TO BE KYNAR COATED METAL TO MATCH WINDOW MULLION COLOR

| ELEVATION | WEST ELEVATION (01/A4.01) | SOUTH PLAZA ELEVATION (01/A4.02) | NORTH BREEZEWAY ELEVATION LOOKING EAST (01/A4.03) | NORTH BREEZEWAY ELEVATION LOOKING WEST (01/A4.04) | WEST BREEZEWAY ELEVATION LOOKING NORTH (01/A4.05) | WEST BREEZEWAY ELEVATION LOOKING SOUTH (01/A4.06) | WEST PLAZA ELEVATION (01/A4.07) | SOUTH ELEVATION (01/A4.08) | EAST ELEVATION (01/A4.09) | NORTH ELEVATION (01/A4.10) | TOTALS | % |
|---------------------------|---------------------------|--|---|---|---|---|---------------------------------------|----------------------------|---------------------------|----------------------------|--------|-------|
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
| MATERIAL TYPES | | | | | | | | | | | | |
| STONE ** | 1952 | 2410 | 1179 | 1047 | 1338 | 1148 | 1346 | 1590 | 3057 | 3543 | 18,610 | 27.04 |
| FENESTRATION | 3137 | 2893 | 1229 | 2644 | 202 | 62 | 2711 | 704 | 4344 | 4067 | 21,993 | 31.96 |
| WOOD | 1159 | 1598 | 516 | 806 | 0 | 0 | 506 | 95 | 1271 | 1534 | 7,485 | 10.88 |
| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
| STUCCO | 1579 | 3405 | 2098 | 2860 | 1089 | 562 | 1737 | 2136 | 3316 | 1928 | 20,710 | 30.09 |

** 1616 SF (8.68%) OF TOTAL STONE AREA IS INCORPORATED IN SITE RETAINING WALLS ADJACENT TO AND INTEGRAL OF THE BUILDING

O 1 SOUTH PLAZA ELEVATION SCALE: 1/8" = 1'-0"

01/A4.02-B 01/A04.02-A TILE ROOF COPPER CAP FLASHING RIDGE ELEV = 9601'-9" WOOD ROOF FASCIA LEVEL 7 ELEV = 9592'-8" TILE ROOF 62'-5" TUBED STYLE ANODIZED **TUBED STYLE ANODIZED** ALUMINUM SNOW GUARD LEVEL 6 ELEV = 9582'-8" ALUMINUM SNOW GUARD WOOD ROOF FASCIA TIMBER BRACKET STUCCO -STUCCO LEVEL 5 ELEV = 9572'-8" CONTROL JOINT, TYP. CONTROL JOINT, TYP. LEVEL 4 ELEV = 9562'-8" KYNAR COATED ALUMINUM KYNAR COATED ALUMINUM CLAD WOOD DOOR CLAD WOOD WINDOW METAL RAILING LEVEL 3 ELEV = 9552'-8" METAL RAILING 24'-4" CEDAR SIDING LEVEL 2 ELEV = 9542'-8" STONE KYNAR COATED ALUMINUM CLAD WOOD WINDOW KYNAR COATED LEVEL 1 ELEV = 9532'-8" **ALUMINUM WINDOW** STONE METAL RAILING GRADE PLAZA ELEV = 9522'-0" STONE LINTEL GARAGE 1 ELEV = 9510'-0" HAND-HEWED MILLED LOG STONE LINTEL TOWN GARAGE EXIT VALET ENTRY/EXIT HAND-HEWED MILLED LOG

LINE OF RAMP BEYOND

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landscape architect Landworks Design Inc. 3457 Ringsby Court, Unit 110 Denver, CO 80216 Phone: 303.433.4257

project Mountain Village Hotel 628, 632, 636, 642, 683 Mountain Village Blvd, Mountain Village, CO 81435
LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF LOTS 73 AND 76, TOWN OF MOUNTAIN VILLAGE AND REPLAT OF TRACT OS-3BR, TOWN OF MOUNTAIN VILLAGE, ACCORDING TO THE PLAT RECORDED JANUARY 25, 2007 IN PLAT BOOK 1 AT PAGE 3807 AT RECEPTION NO. 389801, COUNTY OF SAN MIGUEL, STATE OF COLORADO.

LOT 109, REPLAT NO. 3, TELLURIDE MOUNTAIN VILLAGE, FILING 1, ACCORDING TO THE PLAT RECORDED JULY 31, 1985 IN PLAT BOOK 1 AT PAGE 577, COUNTY OF SAN MIGUEL, STATE OF COLORADO.

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ACCESS TRACT 89-A, TELLURIDE MOUNTAIN VILLAGE, ACCORDING TO THE FIRST REPLAT OF COMBINED LOTS 133 AND 89-1, TELLURIDE MOUNTAIN VILLAGE, FILING 1 RECORDED DECEMBER 26, 1989 IN PLAT BOOK 1 AT PAGE 980, COUNTY OF SAN, MIGUEL, STATE OF COLORADO.

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FINAL PLAN ISSUE NOT FOR CONSTRUCTION

EXTERIOR ELEVATION OVERALL

GENERAL NOTES

ALL EXPOSED CONCRETE TO BE CLAD
 WITH ELASTOMERIC COATING

2. ALL GUTTERS & DOWNSPOUTS TO BE KYNAR COATED METAL TO MATCH

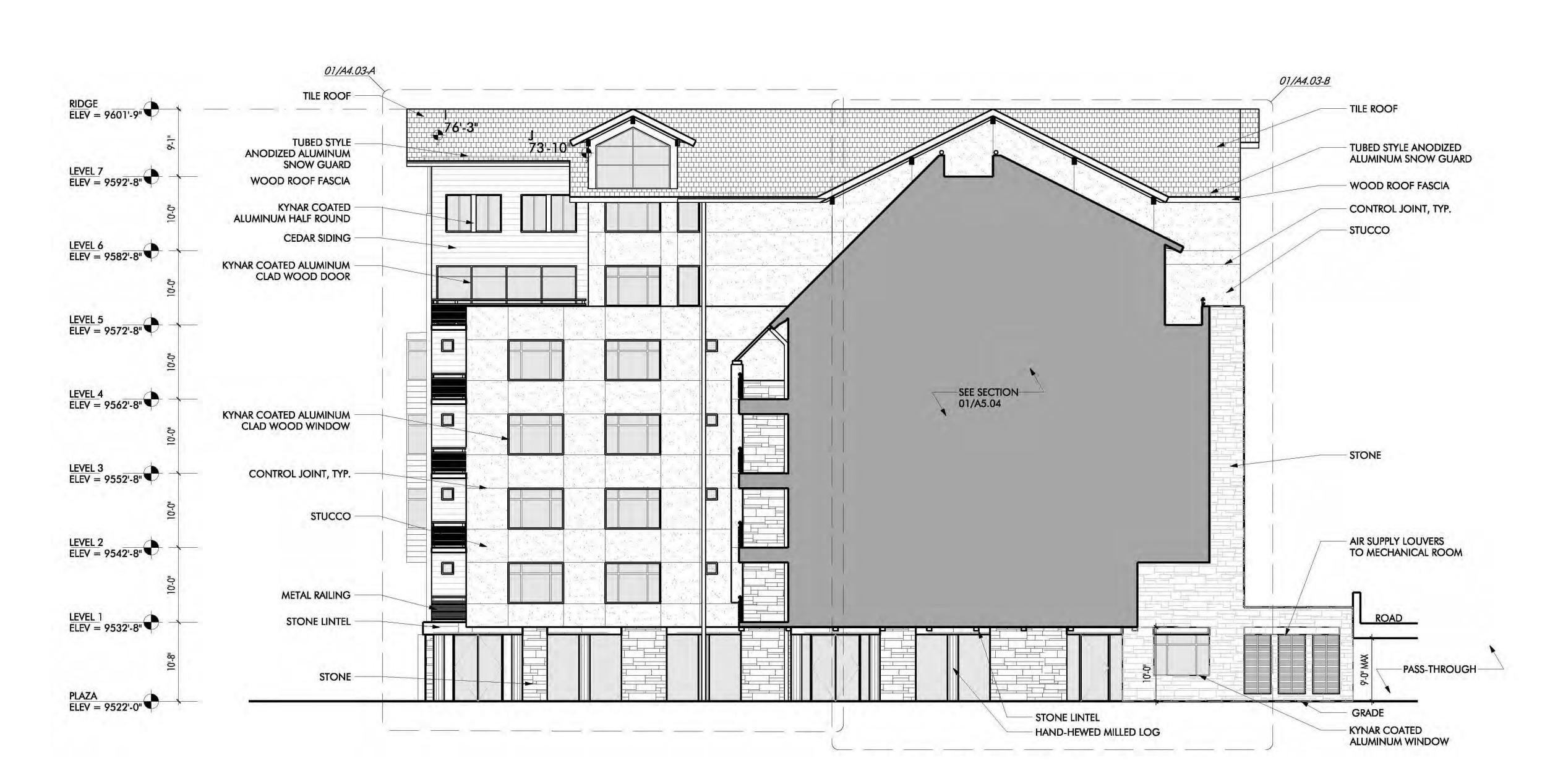
WINDOW MULLION COLOR

| | Α | В | С | D | E | F | G | Н | I | J | K | L | M | N | 0 | P | Q | R | S | T | U | ٧ | W | Х | Y | Z | AA | ВВ | CC | DD | EE | FF | GG | HH | 11 | |
|-----|--------|--------|--------|---------|--------|---------|--------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|----------|
| 7 | 4'-5" | 74'-5" | 73'-2" | 24'-4" | 71'-6" | 80'-10" | 78'-2" | 77'-7" | 76'-3" | 73'-10" | 62'-5" | 83'-7" | 83'-7" | 83'-7" | 82'-2" | 82'-2" | 66'-5" | 77'-2" | 63'-10" | 66'-4" | 57'-21" | 50'-11" | 51'-8" | 57'-8" | 60'-4" | 48'-0" | 37'-3" | 45'-3" | 55'-6" | 46'-6" | 66'-5" | 54'-9" | 67'-7" | 76'-11" | 71'-2" | |
| | IJ | KK | LL | MM | NN | 00 | PP | QQ | RR | SS | TT | UU | VV | ww | | | | | | | | | | | | | | | | | | | | | | AVERAGE |
| -13 | 71'-2" | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6" | 67'-11" | 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | | ī — ī, | | | | | | | | | | | 7 | | | | | | | 65'-2.9" |

MATERIAL AREA CALCULATION

| ELEVATION | WEST ELEVATION | SOUTH PLAZA ELEVATION | NORTH BREEZEWAY ELEVATION LOOKING EAST | NORTH BREEZEWAY ELEVATION LOOKING WEST | WEST BREEZEWAY ELEVATION LOOKING NORTH | WEST BREEZEWAY ELEVATION LOOKING SOUTH | WEST PLAZA ELEVATION | SOUTH ELEVATION | EAST ELEVATION | NORTH ELEVATION | TOTALS | % |
|---------------------------|----------------|--------------------------|--|--|--|--|-------------------------|-----------------|----------------|-----------------|--------|-------|
| | (01/A4.01) | (01/A4.02) | (01/A4.03) | (01/A4.04) | (01/A4.05) | (01/A4.06) | (01/A4.07) | (01/A4.08) | (01/A4.09) | (01/A4.10) | | |
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
| MATERIAL TYPES | | | | | | | | | | | | |
| STONE ** | 1952 | 2410 | 1179 | 1047 | 1338 | 1148 | 1346 | 1590 | 3057 | 3543 | 18,610 | 27.04 |
| FENESTRATION | 3137 | 2893 | 1229 | 2644 | 202 | 62 | 2711 | 704 | 4344 | 4067 | 21,993 | 31.96 |
| WOOD | 1159 | 1598 | 516 | 806 | 0 | 0 | 506 | 95 | 1271 | 1534 | 7,485 | 10.88 |
| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
| STUCCO | 1579 | 3405 | 2098 | 2860 | 1089 | 562 | 1737 | 2136 | 3316 | 1928 | 20,710 | 30.09 |

** 1616 SF (8.68%) OF TOTAL STONE AREA IS INCORPORATED IN SITE RETAINING WALLS ADJACENT TO AND INTEGRAL OF THE BUILDING



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architecture | interiors | planning | graphics 8070 Park Lane, Suite 300 | Dallas, Texas 75231 Tel 972.701.9000 | Fax 972.991.3008 www.bokapowell.com

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LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
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ACCESS TRACT 89-A, TELLURIDE MOUNTAIN VILLAGE,
ACCORDING TO THE FIRST REPLAT OF COMBINED LOTS 133
AND 89-1, TELLURIDE MOUNTAIN VILLAGE, FILING 1
RECORDED DECEMBER 26, 1989 IN PLAT BOOK 1 AT PAGE 980,
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FINAL PLAN ISSUE NOT FOR CONSTRUCTION

revisions

EXTERIOR ELEVATION OVERALL

project number 08131.100 date 11.18.2010

A4.03

GENERAL NOTES

1. ALL EXPOSED CONCRETE TO BE CLAD

2. ALL GUTTERS & DOWNSPOUTS TO BE KYNAR COATED METAL TO MATCH

WITH ELASTOMERIC COATING

WINDOW MULLION COLOR

| A | В | С | D | E | F | G | Н | 1 | J | K | L | M | N | 0 | P | Q | R | S | T | U | ٧ | W | X | Y | Z | AA | BB | CC | DD | EE | FF | GG | HH | II | |
|--------|--------|--------|---------|--------|---------|--------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|----------|
| 74'-5" | 74'-5" | 73'-2" | 24'-4" | 71'-6" | 80'-10" | 78'-2" | 77'-7" | 76'-3" | 73'-10" | 62'-5" | 83'-7" | 83'-7" | 83'-7" | 82'-2" | 82'-2" | 66'-5" | 77'-2" | 63'-10" | 66'-4" | 57'-21" | 50'-11" | 51'-8" | 57'-8" | 60'-4" | 48'-0" | 37'-3" | 45'-3" | 55'-6" | 46'-6" | 66'-5" | 54'-9" | 67'-7" | 76'-11" | 71'-2" | |
| 11 | KK | LL. | MM | NN | 00 | PP | QQ | RR | SS | U | UU | VV | WW | 1 | | | | | | | | | | | | | 1 | | | | | | | | AVERAGE |
| 71'-2" | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6" | 67'-11" | 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | | | | | | | | | | | | | | | | | | | | 65'-2.9" |

MATERIAL AREA CALCULATION

| ELEVATION | WEST ELEVATION (01/A4.01) | SOUTH PLAZA ELEVATION (01/A4.02) | NORTH BREEZEWAY ELEVATION LOOKING EAST (01/A4.03) | NORTH BREEZEWAY ELEVATION LOOKING WEST (01/A4.04) | WEST BREEZEWAY ELEVATION LOOKING NORTH (01/A4.05) | WEST BREEZEWAY ELEVATION LOOKING SOUTH (01/A4.06) | WEST PLAZA ELEVATION (01/A4.07) | SOUTH ELEVATION (01/A4.08) | EAST ELEVATION (01/A4.09) | NORTH ELEVATION (01/A4.10) | TOTALS | % |
|---------------------------|---------------------------|--|---|---|---|---|---------------------------------------|----------------------------|---------------------------|----------------------------|--------|-------|
| | 10000000000 | 21/22/22/24 | | 27720357.00 | (01),,,,,,,,,,, | (01)//1100/ | 2357.557.5 | | | (01)//11/0] | | |
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
| MATERIAL TYPES | | | | | | | | | | | | |
| STONE ** | 1952 | 2410 | 1179 | 1047 | 1338 | 1148 | 1346 | 1590 | 3057 | 3543 | 18,610 | 27.04 |
| FENESTRATION | 3137 | 2893 | 1229 | 2644 | 202 | 62 | 2711 | 704 | 4344 | 4067 | 21,993 | 31.96 |
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| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
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EXTERIOR ELEVATION OVERALL

project number 08131.100 date 11.18.2010

A4.04

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 WITH ELASTOMERIC COATING

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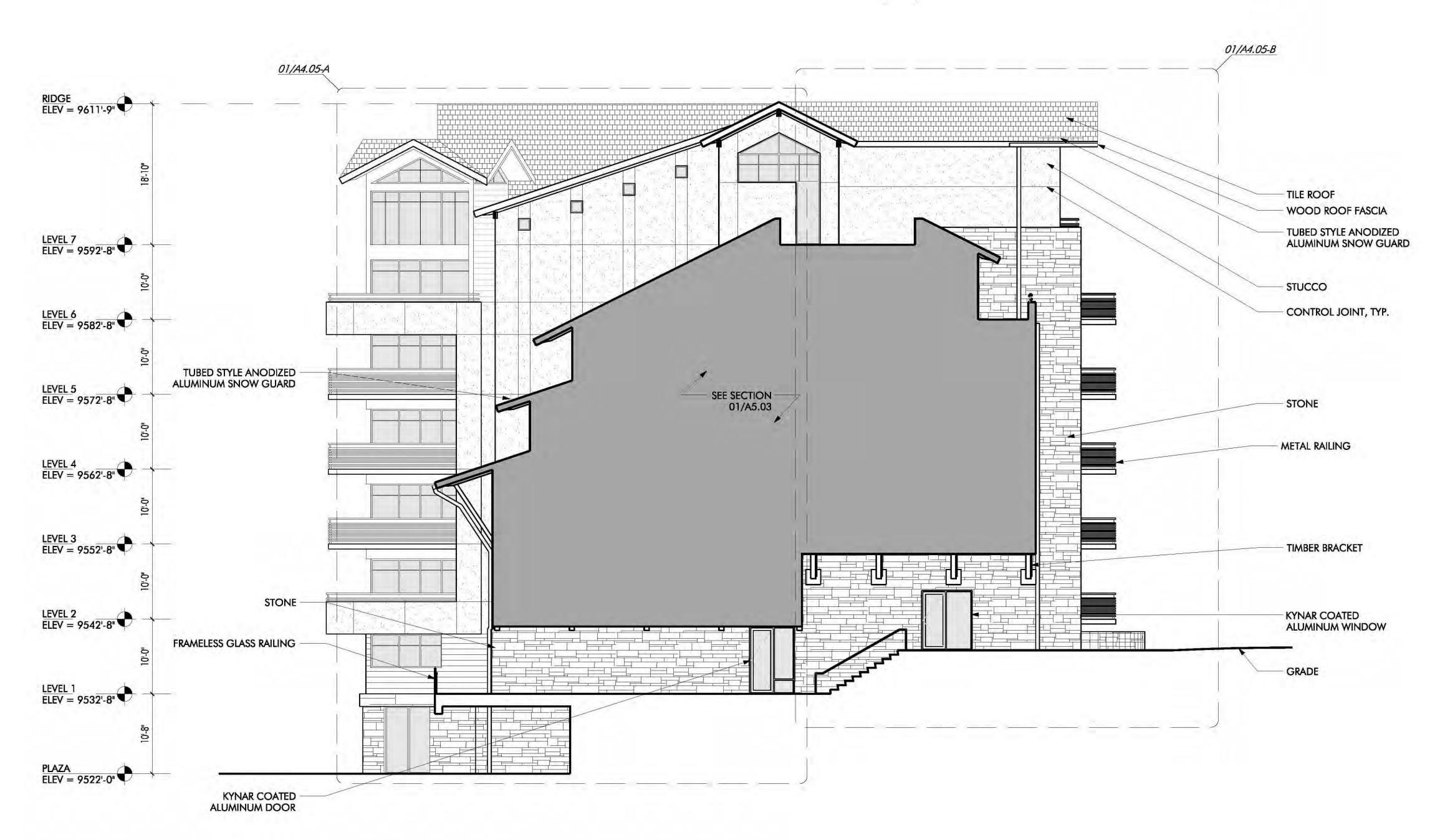
WINDOW MULLION COLOR

| Α | В | С | D | E | F | G | Н | 1 | J | K | L. | M | N | 0 | P | Q | R | S | T | U | V | W | X | Y | Z | AA | BB | CC | DD | EE | FF | GG | HH | 11 | |
|--------|--------|--------|---------|--------|---------|--------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|----------|
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| 11 | KK | LL | MM | NN | 00 | PP | QQ | RR | SS | П | UU | VV | WW | | | | | | | | | | | | | | | | | | | | | | AVERAGE |
| 71'-2 | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6" | 67'-11" | 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | | | | | | | | | | | | | | | | | | | | 65'-2.9" |

MATERIAL AREA CALCULATION

| ELEVATION | WEST ELEVATION | SOUTH PLAZA ELEVATION | NORTH BREEZEWAY ELEVATION LOOKING EAST | NORTH BREEZEWAY ELEVATION LOOKING WEST | WEST BREEZEWAY ELEVATION LOOKING NORTH | WEST BREEZEWAY ELEVATION LOOKING SOUTH | WEST PLAZA ELEVATION | SOUTH ELEVATION | EAST ELEVATION | NORTH ELEVATION | TOTALS | % |
|---------------------------|----------------|--------------------------|--|--|--|--|-------------------------|-----------------|----------------|-----------------|--------|-------|
| | (01/A4.01) | (01/A4.02) | (01/A4.03) | (01/A4.04) | (01/A4.05) | (01/A4.06) | (01/A4.07) | (01/A4.08) | (01/A4.09) | (01/A4.10) | | |
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
| MATERIAL TYPES | | | | | | | | | | | | |
| STONE ** | 1952 | 2410 | 1179 | 1047 | 1338 | 1148 | 1346 | 1590 | 3057 | 3543 | 18,610 | 27.04 |
| FENESTRATION | 3137 | 2893 | 1229 | 2644 | 202 | 62 | 2711 | 704 | 4344 | 4067 | 21,993 | 31.96 |
| WOOD | 1159 | 1598 | 516 | 806 | 0 | 0 | 506 | 95 | 1271 | 1534 | 7,485 | 10.88 |
| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
| STUCCO | 1579 | 3405 | 2098 | 2860 | 1089 | 562 | 1737 | 2136 | 3316 | 1928 | 20,710 | 30.09 |

** 1616 SF (8.68%) OF TOTAL STONE AREA IS INCORPORATED IN SITE RETAINING WALLS ADJACENT TO AND INTEGRAL OF THE BUILDING



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FINAL PUD PLAN ISSUE

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Reg. No.: 400465

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c/o Unity Hunt,
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structural engineer R J C Consulting Engineers 1258 West Broadway, Suite 300 Vancouver, BC V6H 3X8 Canada Phone: 604.738.0048

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project
Mountain Village Hotel
628, 632, 636, 642, 683
Mountain Village Blvd,
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LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF
LOTS 73 AND 76, TOWN OF MOUNTAIN VILLAGE AND REPLAT
OF TRACT OS-3BR, TOWN OF MOUNTAIN VILLAGE,
ACCORDING TO THE PLAT RECORDED JANUARY 25, 2007 IN
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ACCESS TRACT 89-A, TELLURIDE MOUNTAIN VILLAGE,
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RECORDED DECEMBER 26, 1989 IN PLAT BOOK 1 AT PAGE 980,
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FINAL PLAN ISSUE NOT FOR CONSTRUCTION

revision

EXTERIOR ELEVATION OVERALL

project number 08131.100 date 11.18.2010

GENERAL NOTES

1. ALL EXPOSED CONCRETE TO BE CLAD

2. ALL GUTTERS & DOWNSPOUTS TO BE

KYNAR COATED METAL TO MATCH

WITH ELASTOMERIC COATING

WINDOW MULLION COLOR

| Α | В | С | D | E | F | G | H | 1 | J | K | L | M | N | 0 | P | Q | R | S | T | U | ٧ | W | X | Υ | Z | AA | BB | CC | DD | EE | FF | GG | HH | II | |
|--------|--------|--------|---------|--------|---------|--------|--------|--------|----------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|
| 74'-5" | 74'-5" | 73'-2" | 24'-4" | 71'-6" | 80'-10' | 78'-2" | 77'-7" | 76'-3" | 73'-10" | 62'-5" | 83'-7" | 83'-7" | 83'-7" | 82'-2" | 82'-2" | 66'-5" | 77'-2" | 63'-10" | 66'-4" | 57'-21" | 50'-11" | 51'-8" | 57'-8" | 60'-4" | 48'-0" | 37'-3" | 45'-3" | 55'-6" | 46'-6" | 66'-5" | 54'-9" | 67'-7" | 76'-11" | 71'-2" | |
| IJ | KK | LL | MM | NN | 00 | PP | QC | RR | SS | П | UU | VV | ww | | | | | | | | | | | | | | | | | | | | | | AVERA |
| 71'-2" | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6 | 67'-11 | " 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | | | | | | | | | | | | | | | | | | | | 65'-2.' |

MATERIAL AREA CALCULATION

| ELEVATION | WEST ELEVATION (01/A4.01) | SOUTH PLAZA ELEVATION (01/A4.02) | NORTH BREEZEWAY ELEVATION LOOKING EAST (01/A4.03) | NORTH BREEZEWAY ELEVATION LOOKING WEST (01/A4.04) | WEST BREEZEWAY ELEVATION LOOKING NORTH (01/A4.05) | WEST BREEZEWAY ELEVATION LOOKING SOUTH (01/A4.06) | WEST PLAZA ELEVATION (01/A4.07) | SOUTH ELEVATION (01/A4.08) | EAST ELEVATION (01/A4.09) | NORTH ELEVATION (01/A4.10) | TOTALS | % |
|---------------------------|---------------------------|--|---|---|---|---|---------------------------------------|----------------------------|---------------------------|----------------------------|--------|-------|
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
| MATERIAL TYPES | | | | | | | | | | | | |
| STONE ** | 1952 | 2410 | 1179 | 1047 | 1338 | 1148 | 1346 | 1590 | 3057 | 3543 | 18,610 | 27.04 |
| FENESTRATION | 3137 | 2893 | 1229 | 2644 | 202 | 62 | 2711 | 704 | 4344 | 4067 | 21,993 | 31.96 |
| WOOD | 1159 | 1598 | 516 | 806 | 0 | 0 | 506 | 95 | 1271 | 1534 | 7,485 | 10.88 |
| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
| STUCCO | 1579 | 3405 | 2098 | 2860 | 1089 | 562 | 1737 | 2136 | 3316 | 1928 | 20,710 | 30.09 |

** 1616 SF (8.68%) OF TOTAL STONE AREA IS INCORPORATED IN SITE RETAINING WALLS ADJACENT TO AND INTEGRAL OF THE BUILDING

01/A4.06-B 01/A04.06-A TILE ROOF WOOD ROOF FASCIA LEVEL 7 ELEV = 9592'-8" TILE ROOF TUBED STYLE ANODIZED ALUMINUM WOOD ROOF FASCIA LEVEL 6 ELEV = 9582'-8" **SNOW GUARD** CONTROL JOINT, TYP. LEVEL 5 ELEV = 9572'-8" STUCCO SEE SECTION -01/A5.03 STONE TIMBER BRACKET METAL RAILING CONTROL JOINT, TYP. LEVEL 3 ELEV = 9552'-8" STUCCO TIMBER BRACKET METAL RAILING KYNAR COATED ALUMINUM CLAD **WOOD WINDOW** LEVEL 2 ELEV = 9542'-8" STONE GRADE FRAMELESS GLASS RAILING KYNAR COATED **ALUMINUM WINDOW** LEVEL 1 ELEV = 9532'-8" PLAZA ELEV = 9522'-0"

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landscape architect Landworks Design Inc. 3457 Ringsby Court, Unit 110 Denver, CO 80216 Phone: 303.433.4257

project
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Mountain Village Blvd,
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LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
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FINAL PLAN ISSUE NOT FOR CONSTRUCTION

revisions

EXTERIOR ELEVATION OVERALL

project number 08131.100 date 11.18.2010

GENERAL NOTES

1. ALL EXPOSED CONCRETE TO BE CLAD

2. ALL GUTTERS & DOWNSPOUTS TO BE

KYNAR COATED METAL TO MATCH

WITH ELASTOMERIC COATING

WINDOW MULLION COLOR

| | Α | В | С | D | E | F | G | Н | | 1 | K | L | M | N | 0 | P | Q | R | S | T | U | ٧ | W | Х | Y | Z | AA | BB | CC | DD | EE | FF | GG | HH | ĮI. | |
|-----|--------|--------|--------|---------|--------|---------|--------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|----------|
| | 74'-5" | 74'-5" | 73'-2" | 24'-4" | 71'-6" | 80'-10" | 78'-2" | 77'-7" | 76'-3" | 73'-10" | 62'-5" | 83'-7" | 83'-7" | 83'-7" | 82'-2" | 82'-2" | 66'-5" | 77'-2" | 63'-10" | 66'-4" | 57'-21" | 50'-11" | 51'-8" | 57'-8" | 60'-4" | 48'-0" | 37'-3" | 45'-3" | 55'-6" | 46'-6" | 66'-5" | 54'-9" | 67'-7" | 76'-11" | 71'-2" | |
| | IJ | KK | LL | MM | NN | 00 | PP | QQ | RR | SS | П | UU | VV | ww | | | | | | | | | | | | | | | | | | | | | | AVERAGE |
| - 1 | 71'-2" | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6" | 67'-11" | 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | 13 | | | | | | | | | | | | | | | | | 11 1 | | 65'-2.9" |

MATERIAL AREA CALCULATION

| ELEVATION | WEST ELEVATION (01/A4.01) | SOUTH PLAZA ELEVATION (01/A4.02) | NORTH BREEZEWAY ELEVATION LOOKING EAST (01/A4.03) | NORTH BREEZEWAY ELEVATION LOOKING WEST (01/A4.04) | WEST BREEZEWAY ELEVATION LOOKING NORTH (01/A4.05) | WEST BREEZEWAY ELEVATION LOOKING SOUTH (01/A4.06) | WEST PLAZA ELEVATION (01/A4.07) | SOUTH ELEVATION (01/A4.08) | EAST ELEVATION (01/A4.09) | NORTH ELEVATION (01/A4.10) | TOTALS | % |
|---------------------------|---------------------------|--|---|---|---|---|---------------------------------------|----------------------------|------------------------------|----------------------------|--------|-------|
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
| MATERIAL TYPES | | | | | | | | | | | | |
| STONE ** | 1952 | 2410 | 1179 | 1047 | 1338 | 1148 | 1346 | 1590 | 3057 | 3543 | 18,610 | 27.04 |
| FENESTRATION | 3137 | 2893 | 1229 | 2644 | 202 | 62 | 2711 | 704 | 4344 | 4067 | 21,993 | 31.96 |
| WOOD | 1159 | 1598 | 516 | 806 | 0 | 0 | 506 | 95 | 1271 | 1534 | 7,485 | 10.88 |
| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
| STUCCO | 1579 | 3405 | 2098 | 2860 | 1089 | 562 | 1737 | 2136 | 3316 | 1928 | 20,710 | 30.09 |

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FINAL PLAN ISSUE NOT FOR CONSTRUCTION

revision

EXTERIOR ELEVATION - OVERALL

project number 08131.100 date 11.18.2010

GENERAL NOTES

1. ALL EXPOSED CONCRETE TO BE CLAD

2. ALL GUTTERS & DOWNSPOUTS TO BE KYNAR COATED METAL TO MATCH

WITH ELASTOMERIC COATING

WINDOW MULLION COLOR

| Α | В | С | D | E | F | G | Н | I | J | K | L | M | N | 0 | P | Q | R | S | T | U | ٧ | W | Х | Y | Z | AA | BB | CC | DD | EE | FF | GG | HH | 11 | |
|--------|--------|--------|---------|--------|---------|--------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|--------|
| 74'-5" | 74'-5" | 73'-2" | 24'-4" | 71'-6" | 80'-10" | 78'-2" | 77'-7" | 76'-3" | 73'-10" | 62'-5" | 83'-7" | 83'-7" | 83'-7" | 82'-2" | 82'-2" | 66'-5" | 77'-2" | 63'-10" | 66'-4" | 57'-21" | 50'-11" | 51'-8" | 57'-8" | 60'-4" | 48'-0" | 37'-3" | 45'-3" | 55'-6" | 46'-6" | 66'-5" | 54'-9" | 67'-7" | 76'-11" | 71'-2" | |
| IJ | KK | LL - | MM | NN | 00 | PP | QQ | RR | SS | П | UU | VV | WW | | | | | | | | | | | | | | | | | | | | | | AVER.4 |
| 71'-2" | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6" | 67'-11" | 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | | | | | | | | | | | | | | | | | | | | 65'-2. |

MATERIAL AREA CALCULATION

| ELEVATION | WEST ELEVATION | SOUTH PLAZA ELEVATION | NORTH BREEZEWAY ELEVATION LOOKING EAST | NORTH BREEZEWAY ELEVATION LOOKING WEST | WEST BREEZEWAY ELEVATION LOOKING NORTH | WEST BREEZEWAY ELEVATION LOOKING SOUTH | WEST PLAZA ELEVATION | SOUTH ELEVATION | EAST ELEVATION | NORTH ELEVATION | TOTALS | % |
|---------------------------|----------------|--------------------------|--|--|--|--|-------------------------|-----------------|----------------|-----------------|--------|-------|
| | (01/A4.01) | (01/A4.02) | (01/A4.03) | (01/A4.04) | (01/A4.05) | (01/A4.06) | (01/A4.07) | (01/A4.08) | (01/A4.09) | (01/A4.10) | | |
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
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| WOOD | 1159 | 1598 | 516 | 806 | 0 | 0 | 506 | 95 | 1271 | 1534 | 7,485 | 10.88 |
| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
| STUCCO | 1579 | 3405 | 2098 | 2860 | 1089 | 562 | 1737 | 2136 | 3316 | 1928 | 20,710 | 30.09 |

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01/A4.08-B 01/A4.08-A **CEDAR SIDING** LOUVER TILE ROOF TILE ROOF WOOD ROOF FASCIA LEVEL 6 ELEV = 9582'-8" WOOD ROOF FASCIA **GENERATOR EXHAUST** CONTROL JOINT, TYP. INSIDE ENCLOSURE 51'-8" LEVEL 5 ELEV = 9572'-8" CONTROL JOINT, TYP. STUCCO STUCCO KYNAR COATED ALUMINUM CLAD WOOD DOOR METAL RAILING LEVEL 4 ELEV = 9562'-8" HAND-HEWED MILLED LOG **CEDAR SIDING** TUBED STYLE ANODIZED ALUMINUM SNOW GUARD LEVEL 3 ELEV = 9552'-8" METAL RAILING KYNAR COATED ALUMINUM CLAD WOOD WINDOW STONE LEVEL 2 ELEV = 9542'-8" GRADE KYNAR COATED **ALUMINUM CLAD WOOD** WINDOW CEDAR SIDING LEVEL 1 ELEV = 9532'-8" FRAMELESS GLASS - RAILING GRATE LOCATION GRADE PLAZA ELEV = 9522'-0" STONE PRE-RUSTED METAL - DOOR **GRATE LOCATION**

BO KA Powell

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LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
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EXTERIOR ELEVATION OVERALL

project number 08131.100 date 11.18.2010

A4.08

O 1 SOUTH ELEVATION SCALE: 1/8" = 1'-0"

FINAL PUD PLAN for MOUNTAIN VILLAGE HOTEL 628, 632, 636, 642, 683 Mountain Village Blvd, Mountain Village, CO 81435; LOTS 73-76R, 89A, 109,110

AVERAGE HEIGHT CALCULATION

GENERAL NOTES

1. ALL EXPOSED CONCRETE TO BE CLAD

2. ALL GUTTERS & DOWNSPOUTS TO BE KYNAR COATED METAL TO MATCH

WITH ELASTOMERIC COATING

WINDOW MULLION COLOR

| Α | В | С | D | E | F | G | Н | 1 | J | K | L | M | N | 0 | P | Q | R | S | T | U | V | W | Х | Y | Z | AA | BB | CC | DD | EE | FF | GG | НН | 11 | |
|--------|--------|--------|---------|--------|---------|--------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|---------|---------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|--------|---------|--------|----------|
| 74'-5" | 74'-5" | 73'-2 | 24'-4" | 71'-6" | 80'-10" | 78'-2" | 77'-7" | 76'-3" | 73'-10" | 62'-5" | 83'-7" | 83'-7" | 83'-7" | 82'-2" | 82'-2" | 66'-5" | 77'-2" | 63'-10" | 66'-4" | 57'-21" | 50'-11" | 51'-8" | 57'-8" | 60'-4" | 48'-0" | 37'-3" | 45'-3" | 55'-6" | 46'-6" | 66'-5" | 54'-9" | 67'-7" | 76'-11" | 71'-2" | |
| IJ | KK | LL | MM | NN | 00 | PP | QQ | RR | SS | Π | UU | VV | ww | | | | | | | | | | | | | | | | | | | | | | AVERAGE |
| 71'-2" | 70'-8" | 62'-5" | 73'-11" | 61'-6" | 64'-1" | 67'-8" | 64'-6" | 67'-11" | 63'-3" | 60'-6" | 24'-4" | 66'-0" | 74'-5" | | | 10 | | | | | | | | | | | | | | | | | | | 65'-2.9" |

MATERIAL AREA CALCULATION

| ELEVATION | WEST ELEVATION (01/A4.01) | SOUTH PLAZA ELEVATION (01/A4.02) | NORTH BREEZEWAY ELEVATION LOOKING EAST (01/A4.03) | NORTH BREEZEWAY ELEVATION LOOKING WEST (01/A4.04) | WEST BREEZEWAY ELEVATION LOOKING NORTH (01/A4.05) | WEST BREEZEWAY ELEVATION LOOKING SOUTH (01/A4.06) | WEST PLAZA ELEVATION (01/A4.07) | SOUTH ELEVATION (01/A4.08) | EAST ELEVATION (01/A4.09) | NORTH ELEVATION (01/A4.10) | TOTALS | % |
|---------------------------|---------------------------|--|---|---|---|---|---------------------------------------|----------------------------|---------------------------|----------------------------|--------|-------|
| TOTAL SF OF EXTERIOR WALL | 7827 | 10306 | 5022 | 7357 | 2629 | 1772 | 6300 | 4548 | 11988 | 11072 | 68,821 | 100 |
| MATERIAL TYPES | | | | | | | | | | | | |
| STONE ** | 1952 | 2410 | 1179 | 1047 | 1338 | 1148 | 1346 | 1590 | 3057 | 3543 | 18,610 | 27.04 |
| FENESTRATION | 3137 | 2893 | 1229 | 2644 | 202 | 62 | 2711 | 704 | 4344 | 4067 | 21,993 | 31.96 |
| WOOD | 1159 | 1598 | 516 | 806 | 0 | 0 | 506 | 95 | 1271 | 1534 | 7,485 | 10.88 |
| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
| STUCCO | 1579 | 3405 | 2098 | 2860 | 1089 | 562 | 1737 | 2136 | 3316 | 1928 | 20,710 | 30.09 |

** 1616 SF (8.68%) OF TOTAL STONE AREA IS INCORPORATED IN SITE RETAINING WALLS ADJACENT TO AND INTEGRAL OF THE BUILDING



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LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
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ACCESS TRACT 89-A, TELLURIDE MOUNTAIN VILLAGE, ACCORDING TO THE FIRST REPLAT OF COMBINED LOTS 133 AND 89-1, TELLURIDE MOUNTAIN VILLAGE, FILING 1 RECORDED DECEMBER 26, 1989 IN PLAT BOOK 1 AT PAGE 980, COUNTY OF SAN, MIGUEL, STATE OF COLORADO.

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sions

EXTERIOR ELEVATION - OVERALL

project number 08131.100 date 11.18.2010

GENERAL NOTES

1. ALL EXPOSED CONCRETE TO BE CLAD WITH ELASTOMERIC COATING

2. ALL GUTTERS & DOWNSPOUTS TO BE

WINDOW MULLION COLOR

KYNAR COATED METAL TO MATCH

CONTROL JOINT, TYP.

TUBED STYLE ANODIZED

ALUMINUM SNOW GUARD

STUCCO

TILE ROOF

| ELEVATION | WEST ELEVATION (01/A4.01) | SOUTH PLAZA ELEVATION (01/A4.02) | NORTH BREEZEWAY ELEVATION LOOKING EAST (01/A4.03) | NORTH BREEZEWAY ELEVATION LOOKING WEST (01/A4.04) | WEST BREEZEWAY ELEVATION LOOKING NORTH (01/A4.05) | WEST BREEZEWAY ELEVATION LOOKING SOUTH (01/A4.06) | WEST PLAZA ELEVATION (01/A4.07) | SOUTH ELEVATION (01/A4.08) | EAST ELEVATION (01/A4.09) | NORTH ELEVATION (01/A4.10) | TOTALS | % |
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| ACCENT | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 23 | 0 | 0 | 23 | 0.03 |
| STUCCO | 1579 | 3405 | 2098 | 2860 | 1089 | 562 | 1737 | 2136 | 3316 | 1928 | 20,710 | 30.09 |

** 1616 SF (8.68%) OF TOTAL STONE AREA IS INCORPORATED IN SITE RETAINING WALLS ADJACENT TO AND INTEGRAL OF THE BUILDING

0 1 NORTH ELEVATION
SCALE: 1/8" = 1'-0"



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AVERAGE

65'-2.9"

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project Mountain Village Hotel 628, 632, 636, 642, 683 Mountain Village Blvd, Mountain Village, CO 81435
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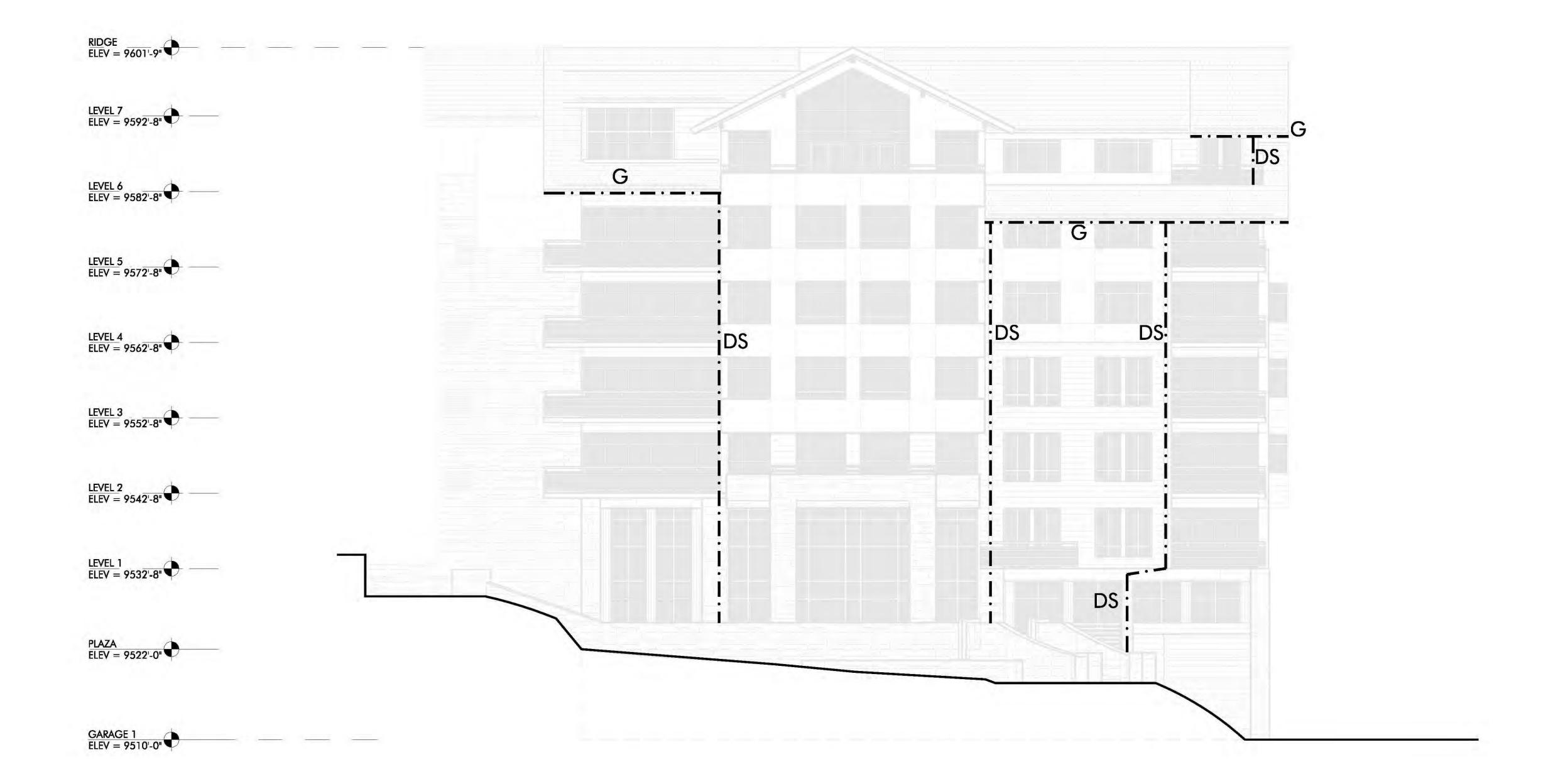
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FINAL PLAN ISSUE NOT FOR CONSTRUCTION

EXTERIOR ELEVATION -**OVERALL**

| SNOW ME | LT KEY | |
|--------------|--------------------------|--|
| * | SPLASH BLOCKS IN TERRACE | |
| \leftarrow | DIRECTION OF WATER FLOW | |
| | GUTTER OR DOWNSPOUT | |
| G | GUTTER | |
| DS | DOWNSPOUT | |





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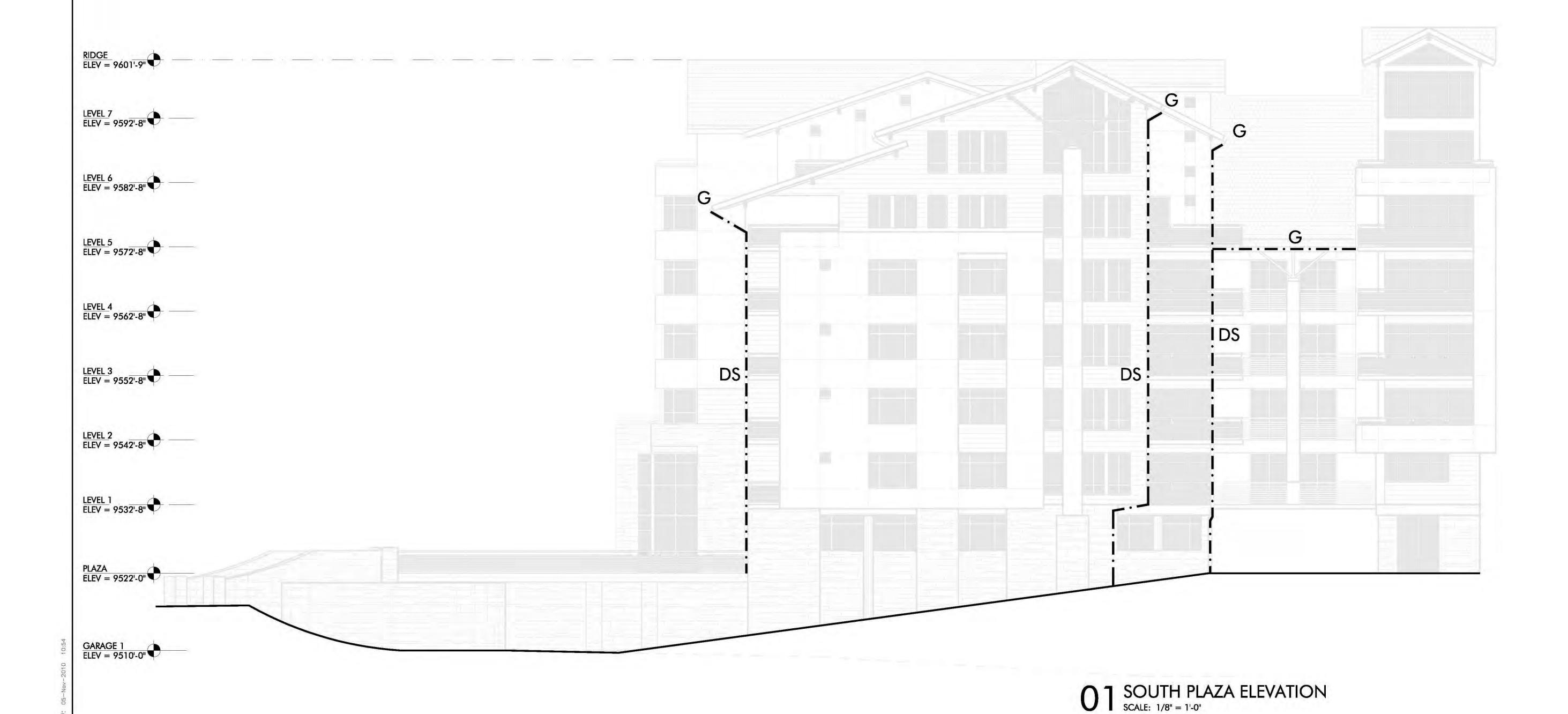
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EXTERIOR ELEVATION -SNOW MELT STUDY

| 4. | | |
|----------|--------------------------|--|
| * | SPLASH BLOCKS IN TERRACE | |
| ← | DIRECTION OF WATER FLOW | |
| | GUTTER OR DOWNSPOUT | |
| G | GUTTER | |
| DS | DOWNSPOUT | |



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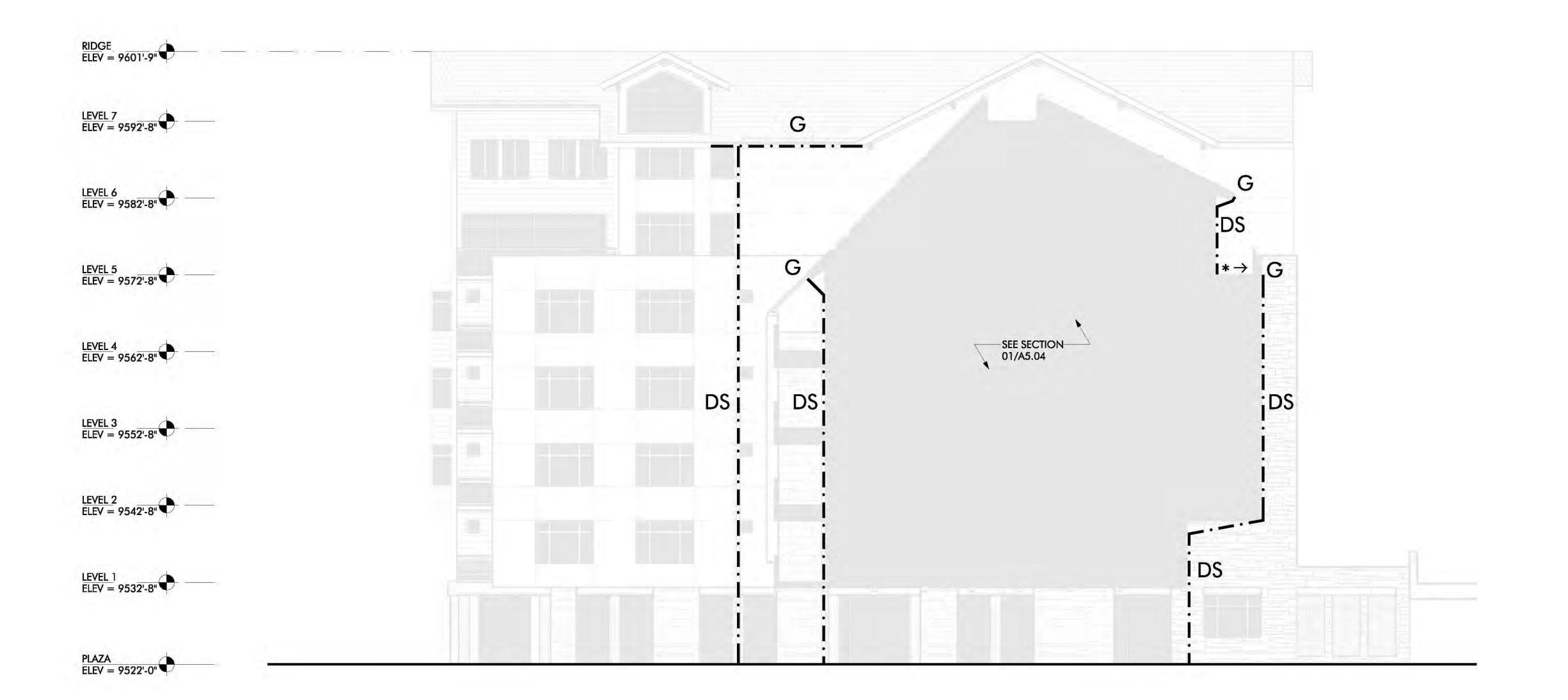
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| SNOW ME | LT KEY | |
|--------------|--------------------------|--|
| * | SPLASH BLOCKS IN TERRACE | |
| \leftarrow | DIRECTION OF WATER FLOW | |
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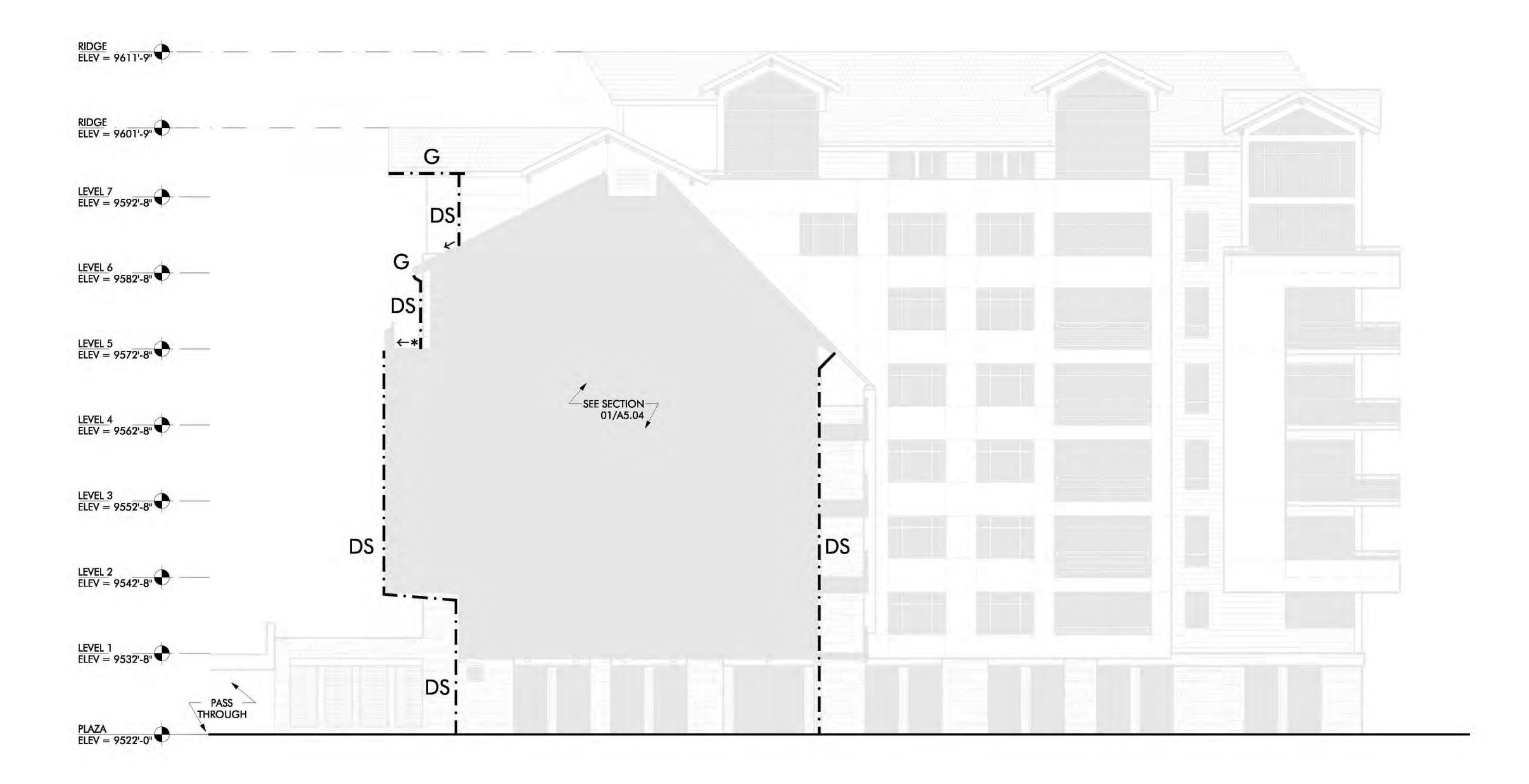
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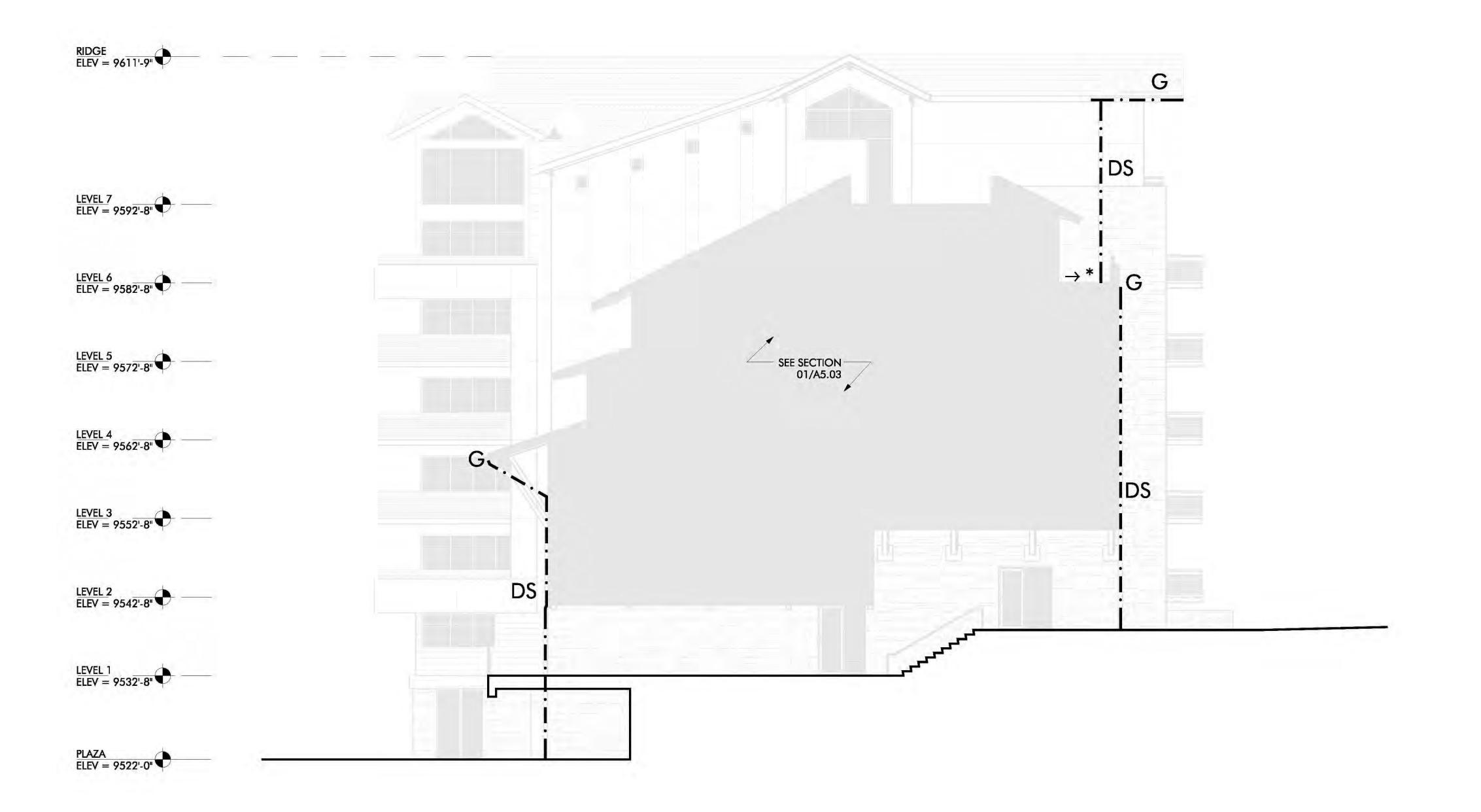
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| SNOW MEL | T KEY | |
|----------|--------------------------|--|
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| ← | DIRECTION OF WATER FLOW | |
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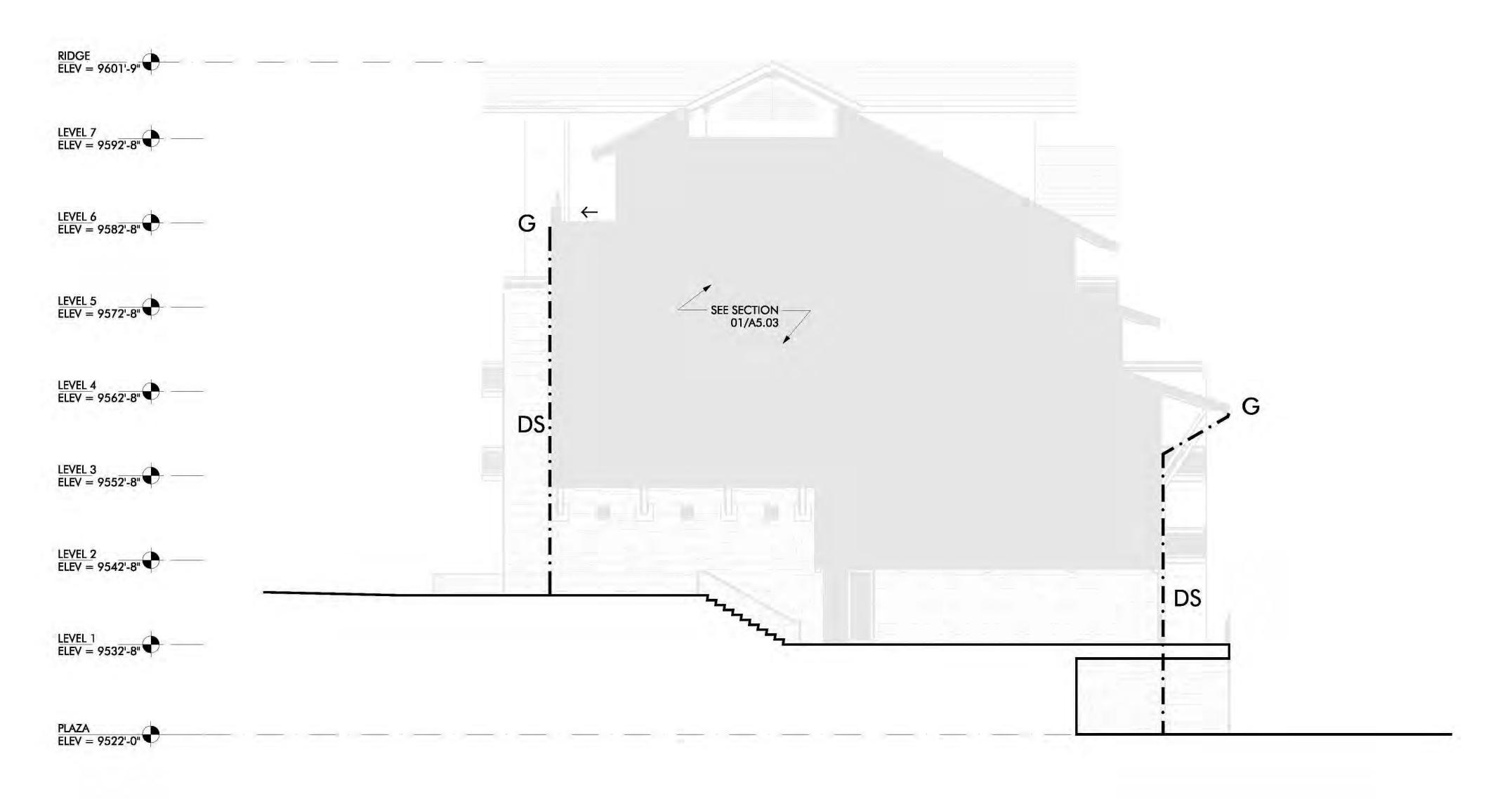
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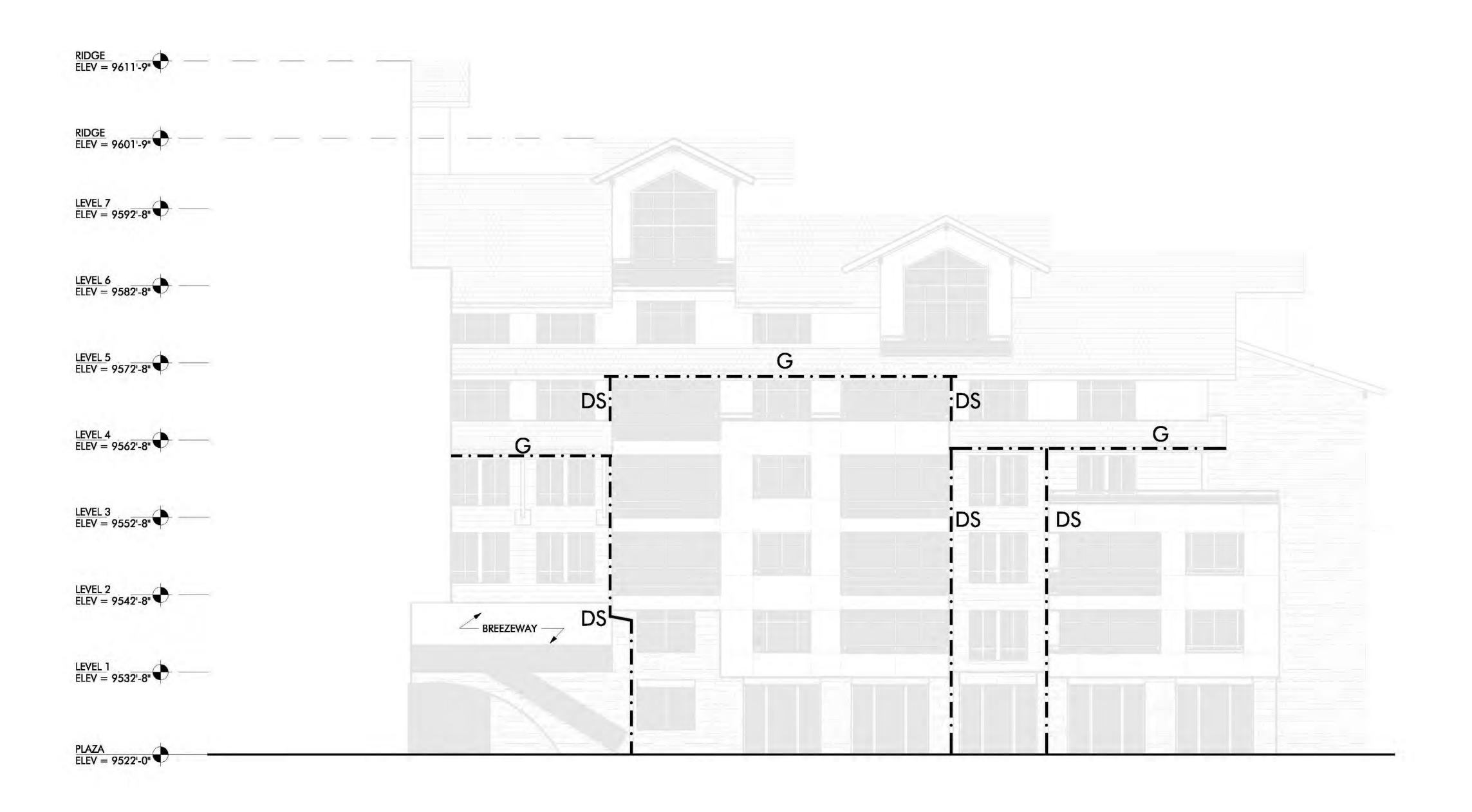
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EXTERIOR ELEVATION SNOW MELT STUDY

| SNOW ME | LT KEY | |
|----------|--------------------------|--|
| * | SPLASH BLOCKS IN TERRACE | |
| ← | DIRECTION OF WATER FLOW | |
| _,_ | GUTTER OR DOWNSPOUT | |
| G | GUTTER | |
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FINAL PUD PLAN ISSUE

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Arch: Chris W. Barnes Reg. No.: 400465 owner/applicant MV Colorado Development Partners, LLC c/o Unity Hunt, 1601 Elm St. Ste. 4000, Dallas, TX 75201

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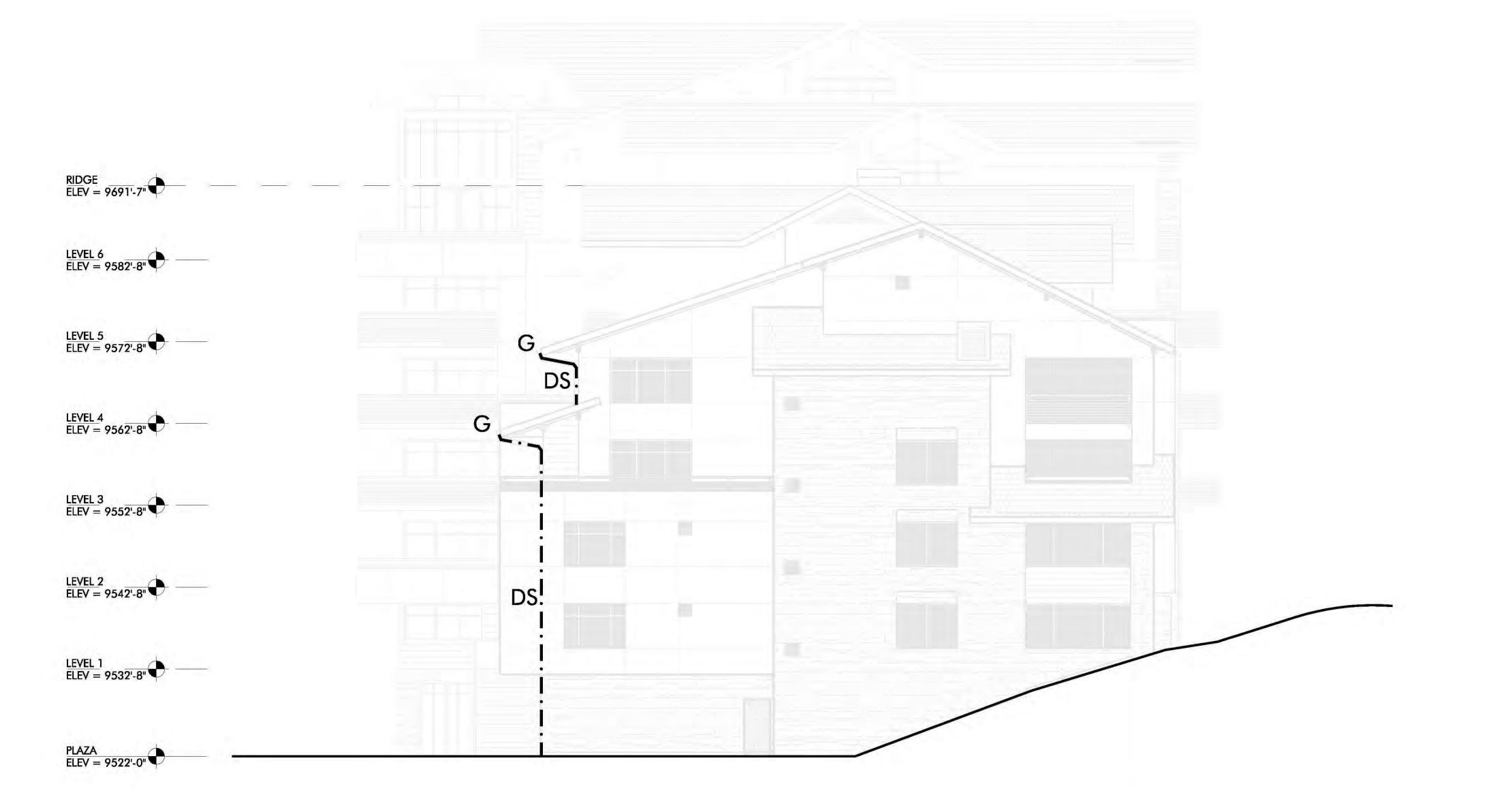
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| G | GUTTER | |
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architecture | interiors | planning | graphics 8070 Park Lane, Suite 300 | Dallas, Texas 75231 Tel 972.701.9000 | Fax 972.991.3008 www.bokapowell.com

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revision

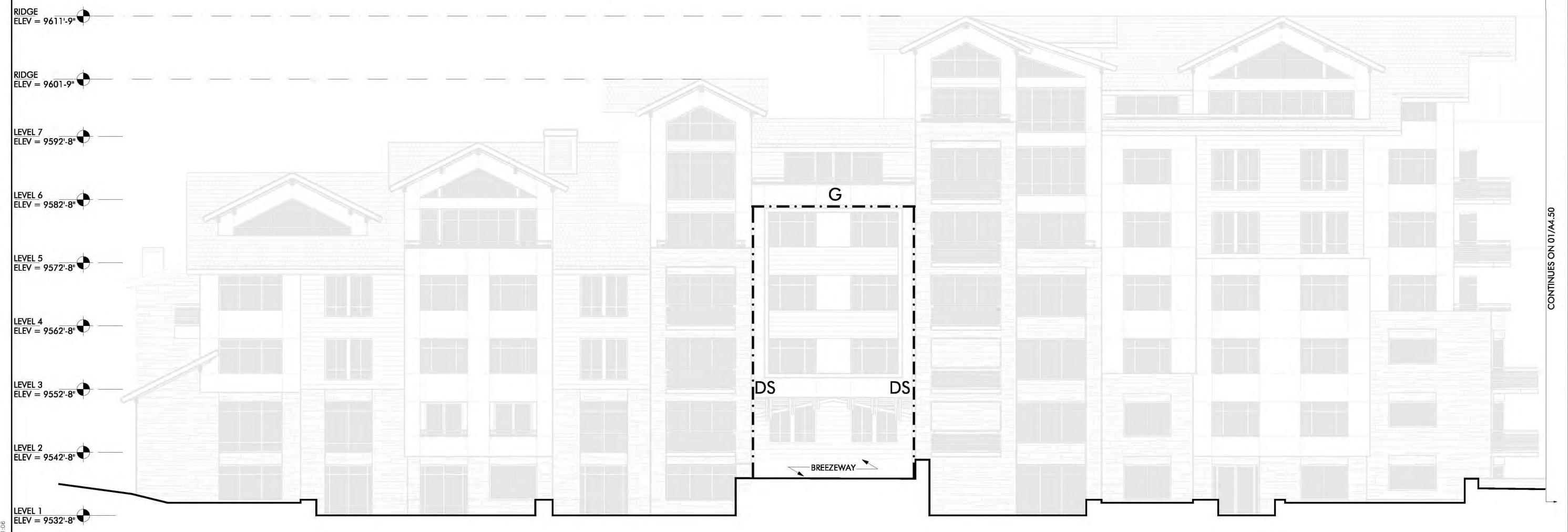
EXTERIOR ELEVATION -SNOW MELT STUDY

project number 08131.100

date 11.18.2010

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| NIINTAIN VII I AGE HNTEI 678 637 636 647 683 | Mountain Village Blvd, Mountain Village, CO 81435; LOTS 73-76R, 89A, 109,110 |
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revisions

EXTERIOR ELEVATION -SNOW MELT STUDY

project number 08131.100 date 11.18.2010

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| | FINAL PUD PLAN |
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| MOUNTAIN VILLAGE HOTEL 628, 632, 636, 642, 683 | Mountain Village Blvd, Mountain Village, CO 81435; LOTS 73-76R, 89A, 109,110 |

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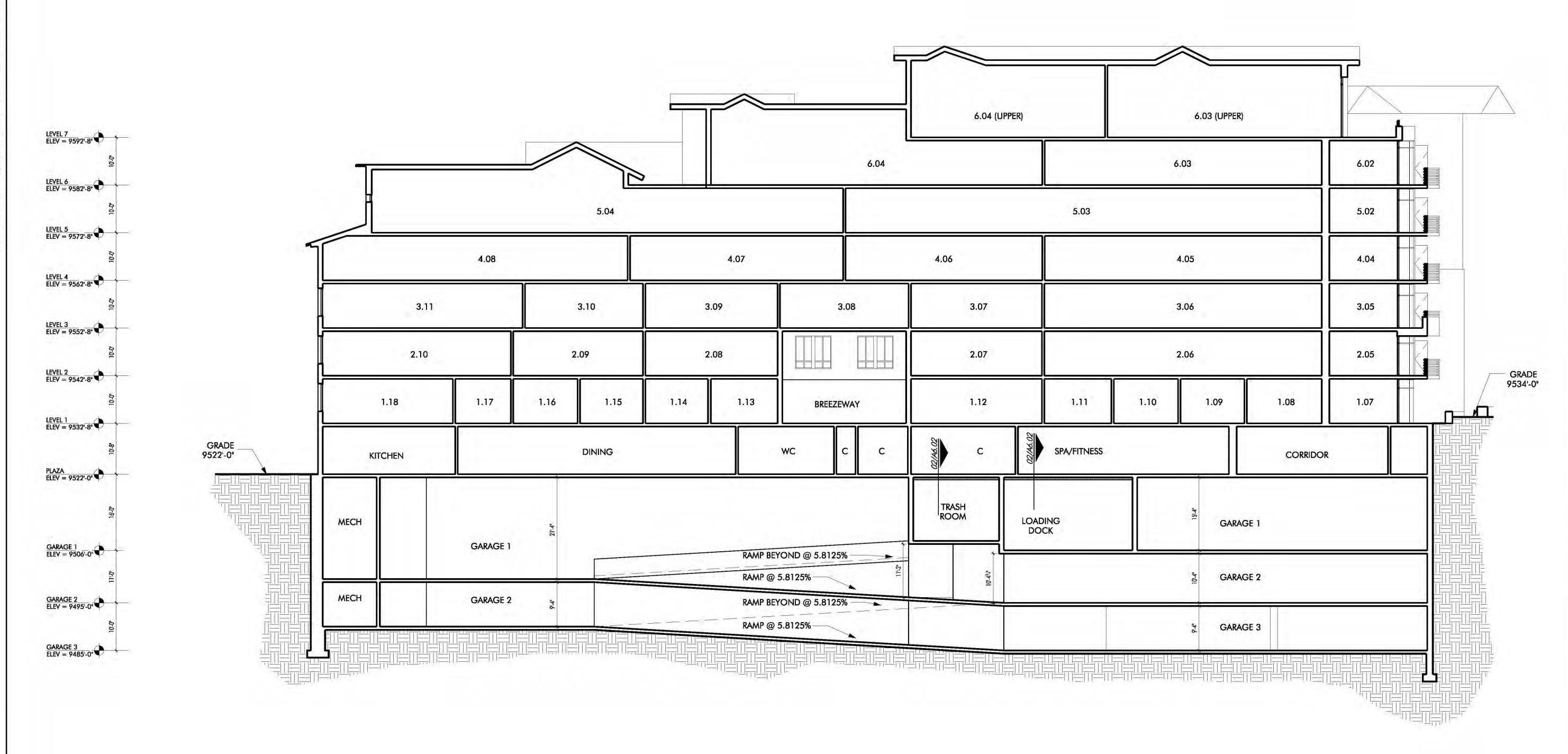
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evision

EXTERIOR ELEVATION -SNOW MELT STUDY

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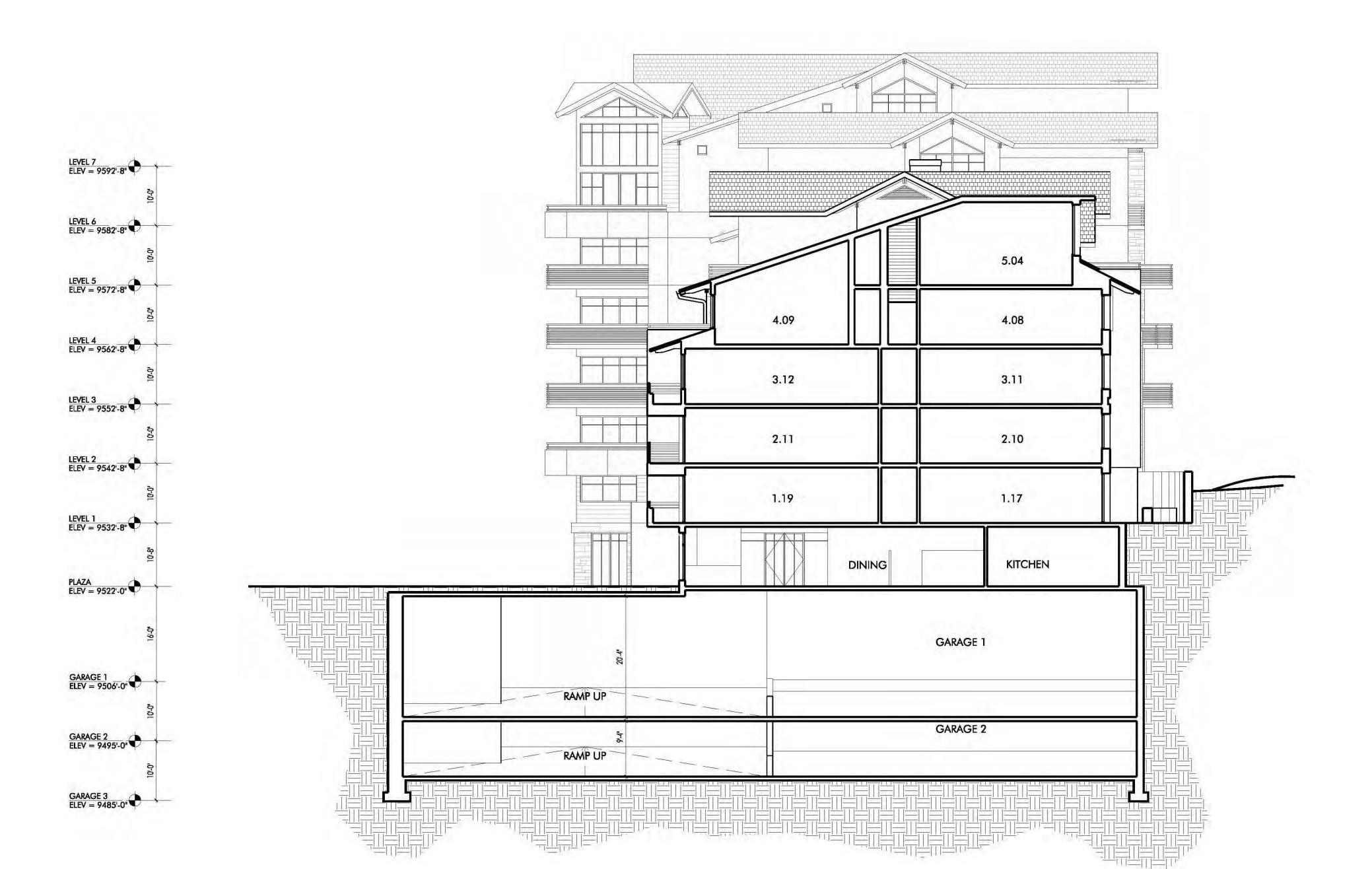
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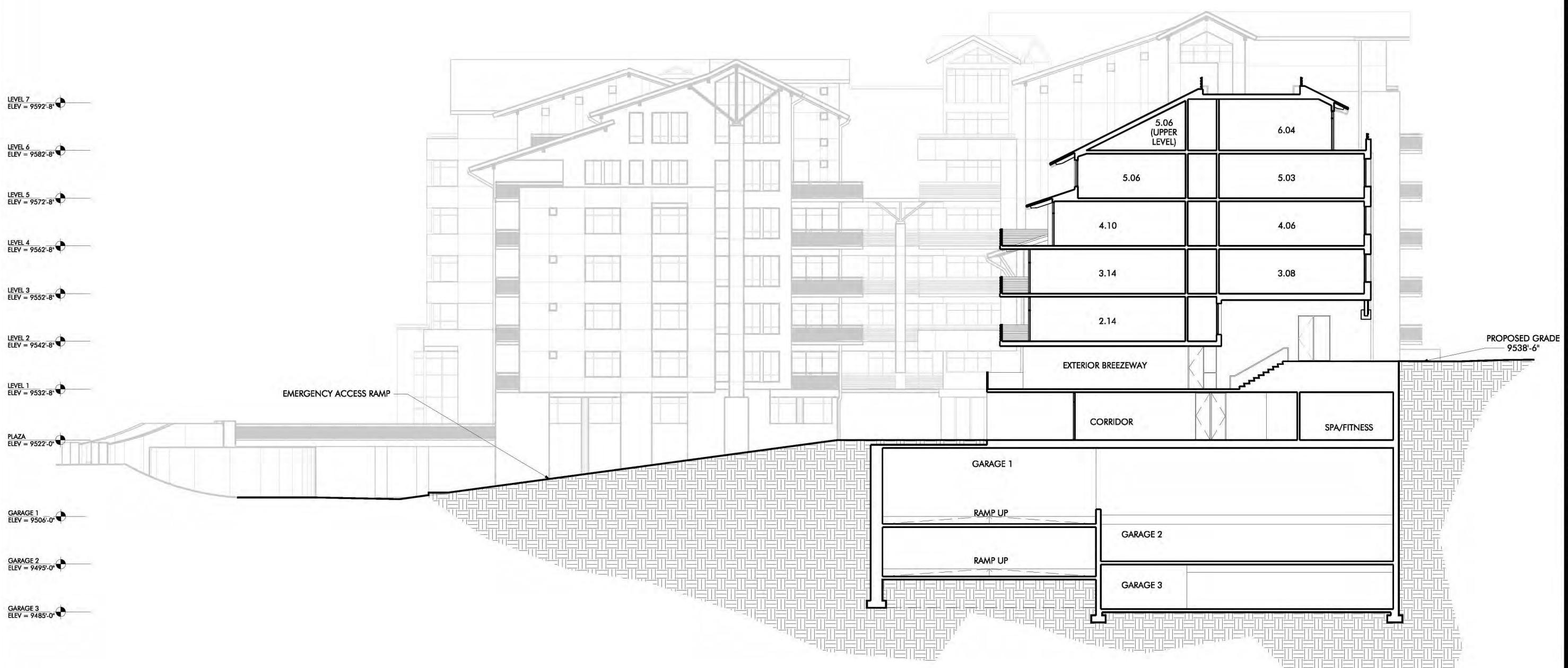
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BUILDING SECTION

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01 SECTION C-C A5.03



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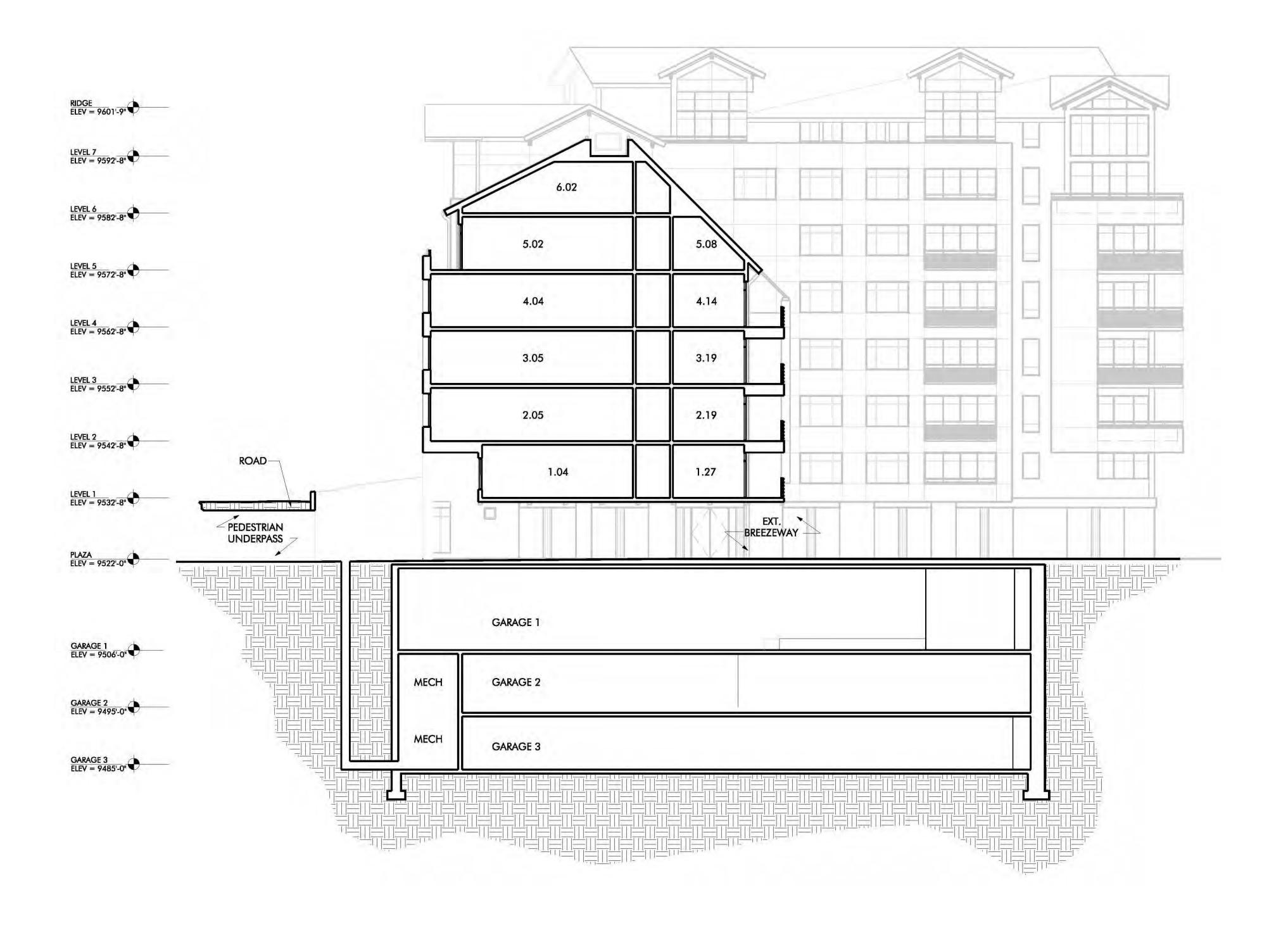
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01 SECTION D-D SCALE: 3/32" = 1'-0" A5.04





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landscape architect Landworks Design Inc. 3457 Ringsby Court, Unit 110 Denver, CO 80216 Phone: 303.433.4257

project
Mountain Village Hotel
628, 632, 636, 642, 683
Mountain Village Blvd,
Mountain Village Blvd,
Mountain Village, CO 81435
LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF
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ACCESS TRACT 89-A, TELLURIDE MOUNTAIN VILLAGE,
ACCORDING TO THE FIRST REPLAT OF COMBINED LOTS 133
AND 89-1, TELLURIDE MOUNTAIN VILLAGE, FILING 1
RECORDED DECEMBER 26, 1989 IN PLAT BOOK 1 AT PAGE 980,
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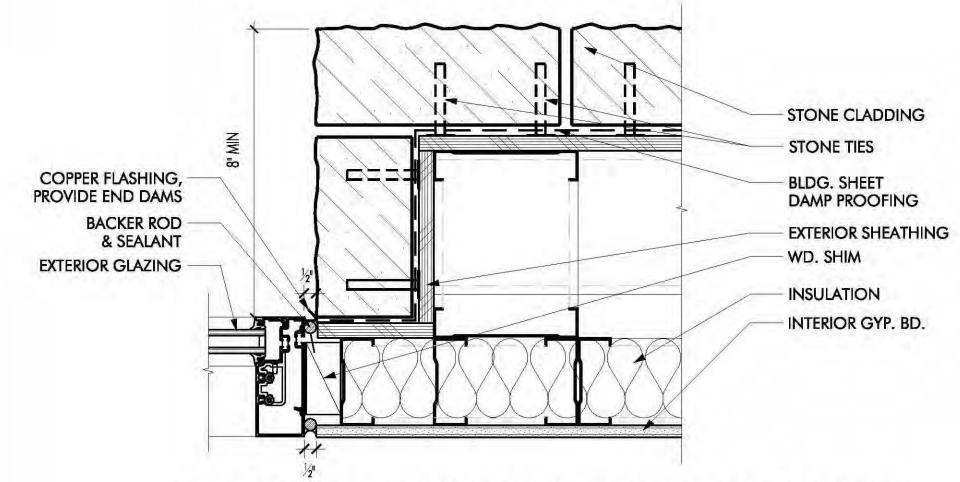
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BUILDING SECTION

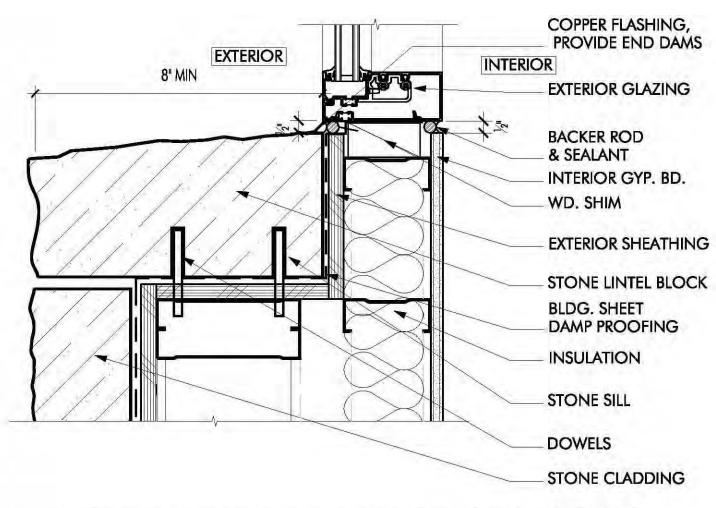
project number 08131.100 date 11.18.2010

01 SECTION E-E A5.05

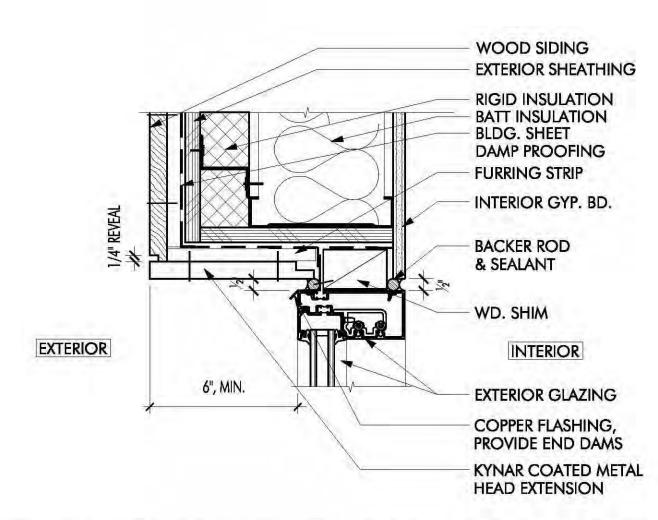
9 TYPICAL HEAD DETAIL @ STONE CLADDING SCALE: 3" = 1'-0"



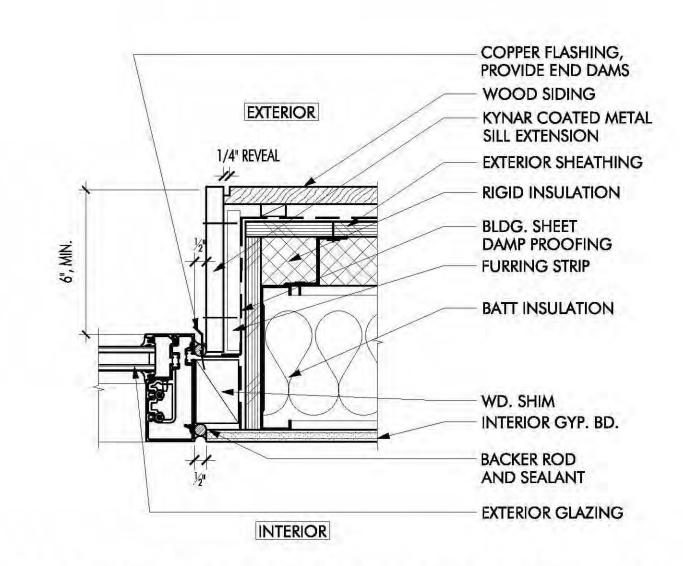
08 TYPICAL JAMB DETAIL @ STONE CLADDING SCALE: 3" = 1'-0"



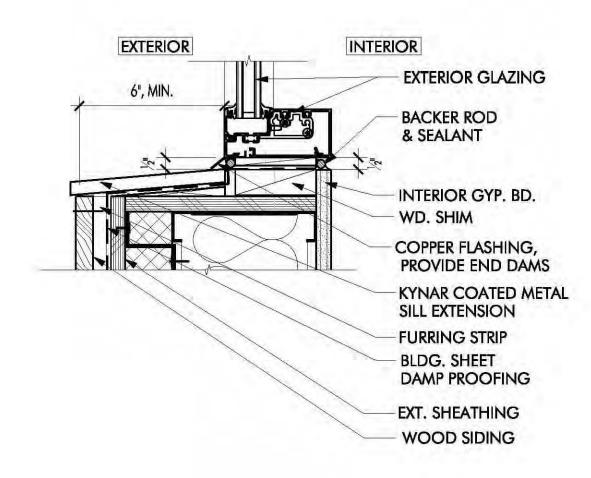
7 TYPICAL SILL DETAIL @ STONE CLADDING SCALE: 3" = 1'-0"



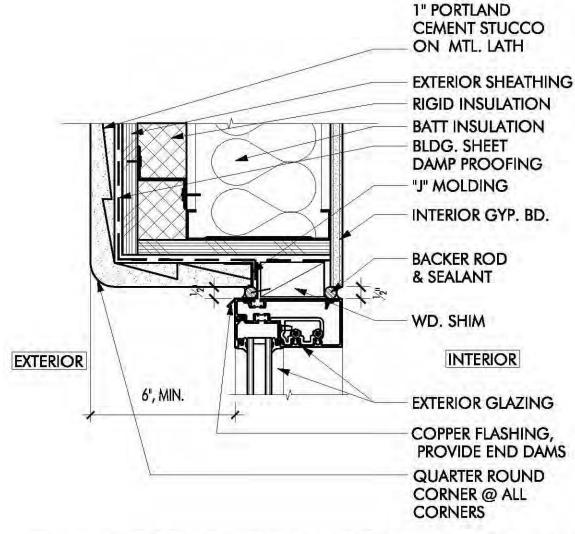
06 TYPICAL HEAD DETAIL @ WOOD SIDING SCALE: 3" = 1'-0"



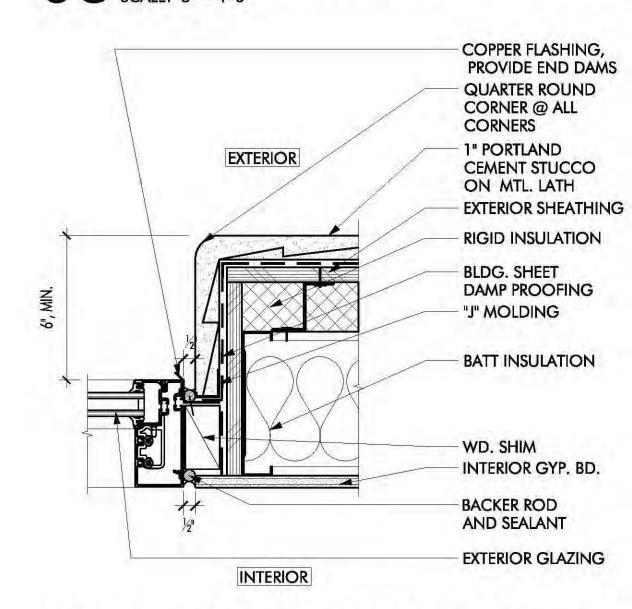
05 TYPICAL JAMB DETAIL @ WOOD SIDING SCALE: 3" = 1'-0"



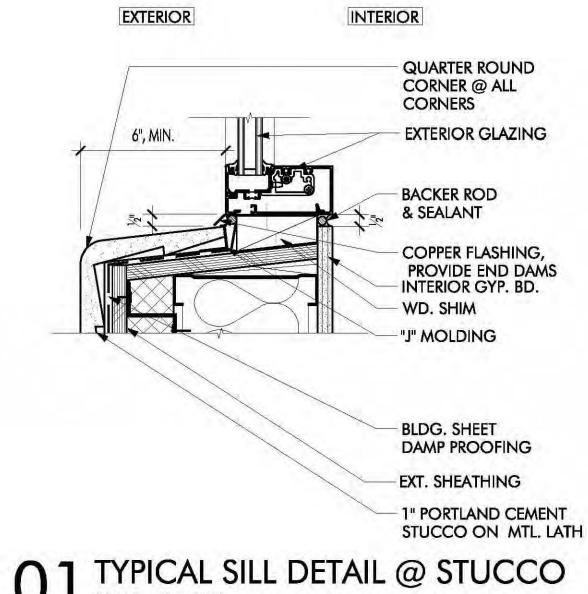
04 TYPICAL SILL DETAIL @ WOOD SIDING SCALE: 3" = 1'-0"



03 TYPICAL HEAD DETAIL @ STUCCO



02 TYPICAL JAMB DETAIL @ STUCCO



O 1 TYPICAL SILL DETAIL @ STUCCO SCALE: 3" = 1'-0"



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TYPICAL EXTERIOR **DETAILS**

project number 08131.100 date 11.18.2010

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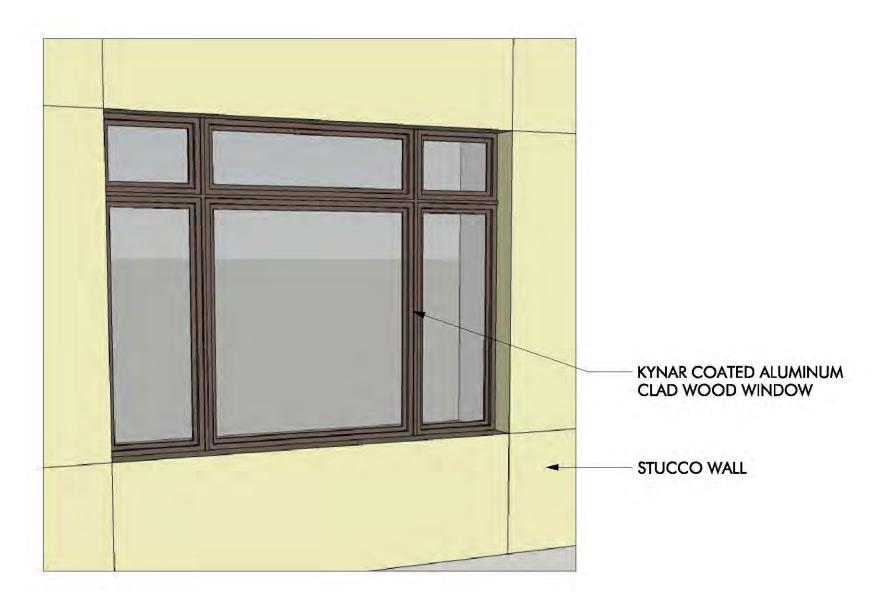
FINAL PUD PLAN for MOUNTAIN VILLAGE HOTEL 628, 632, 636, 642, 683 Mountain Village Blvd, Mountain Village, CO 81435; LOTS 73-76R, 89A, 109,110



09 TYPICAL WINDOW @ PLAZA SCALE: NO SCALE



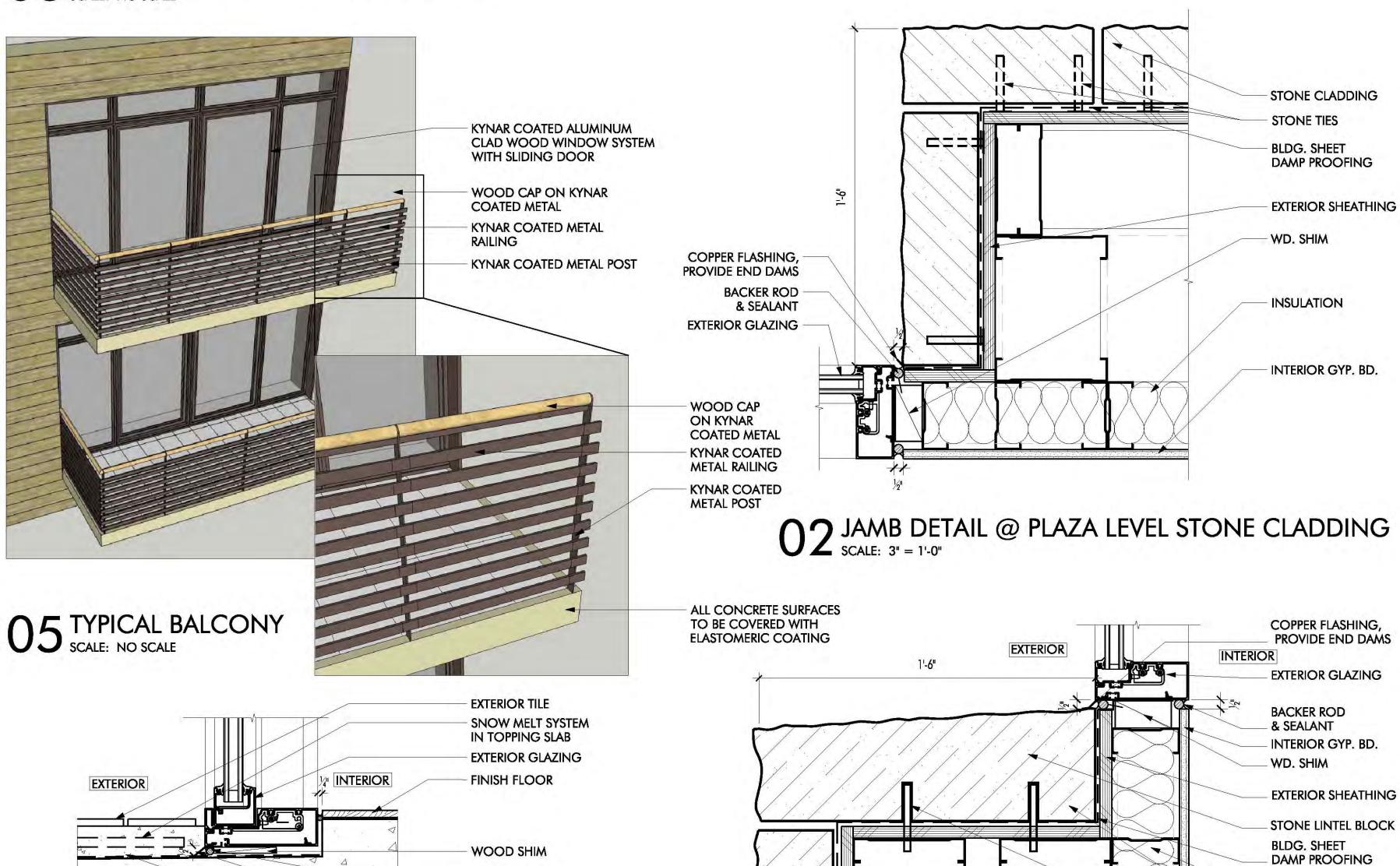
08 TYPICAL WINDOW IN STONE SCALE: NO SCALE



07 TYPICAL WINDOW IN STUCCO SCALE: NO SCALE



06 TYPICAL WINDOW IN WOOD SIDING SCALE: NO SCALE



O4 TYPICAL THRESHOLD @ BALCONY SLIDING DOOR SCALE: 3" = 1'-0"

BACKER ROD & SEALANT

BLDG. SHEET
DAMP PROOFING

COPPER FLASHING,

PROVIDE END DAMS

O 1 SILL DETAIL @ PLAZA LEVEL STONE CLADDING SCALE: 3" = 1'-0"



INTERIOR

EXTERIOR

03 HEAD DETAIL @ PLAZA LEVEL STONE CLADDING SCALE: 3" = 1'-0"

EXTERIOR SHEATHING

STONE LINTEL BLOCK

INTERIOR GYP. BD.

EXTERIOR GLAZING

COPPER FLASHING, PROVIDE END DAMS

STONE CLADDING

STEEL ANGLE

INSULATION

BLDG. SHEET DAMP PROOFING

WD. SHIM

BACKER ROD

& SEALANT

STONE HANGER

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visions

INSULATION

STONE SILL

STONE CLADDING

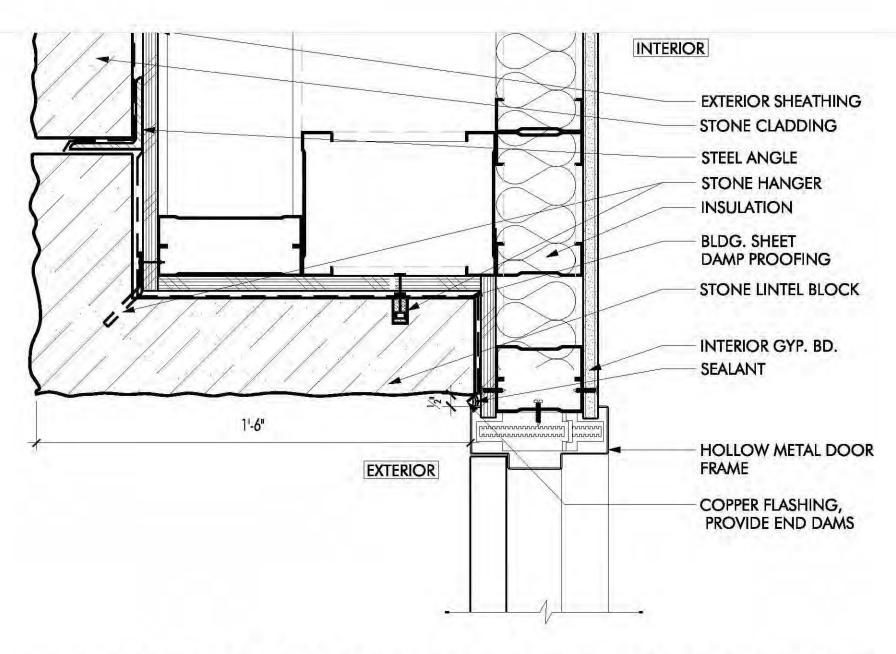
DOWELS

TYPICAL EXTERIOR DETAILS

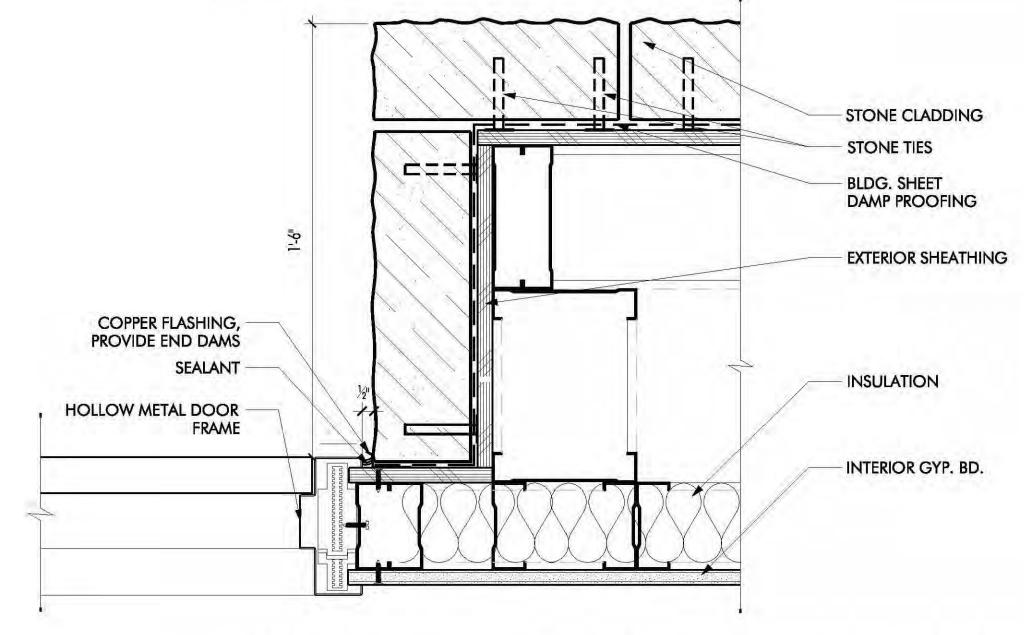
project number 08131.100 date 11.18.2010

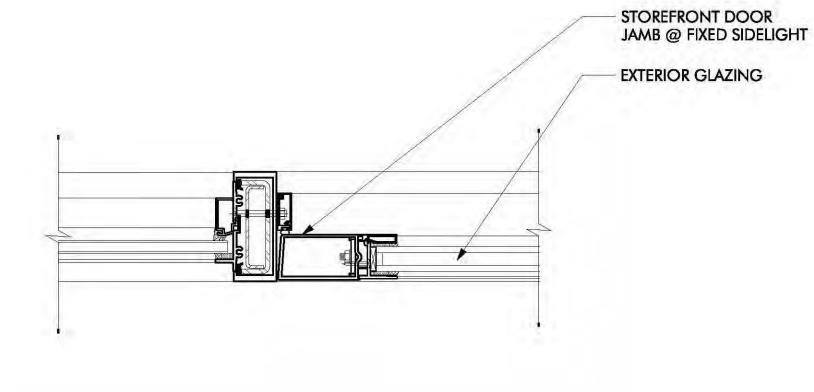
A6.01a

06 TYP STOREFRONT HEAD @ PLAZA LEVEL

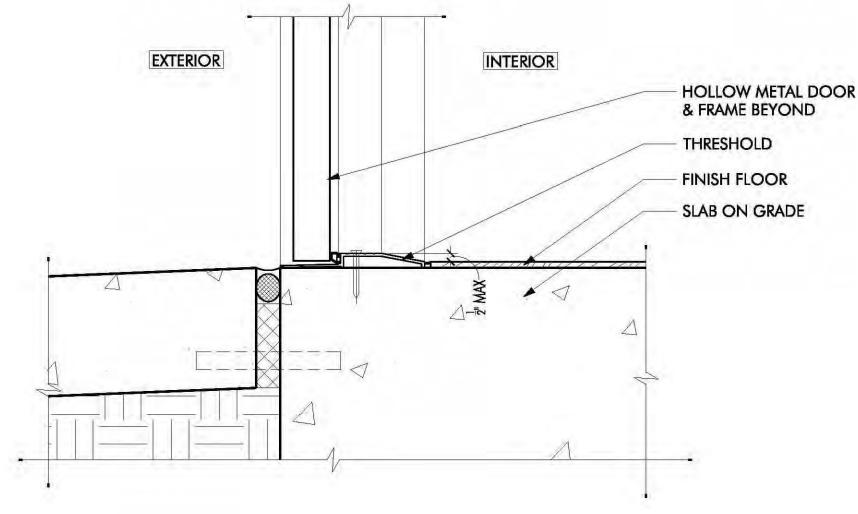


03 HM HEAD DETAIL @ PLAZA LEVEL STONE CLADDING SCALE: 3" = 1'-0"

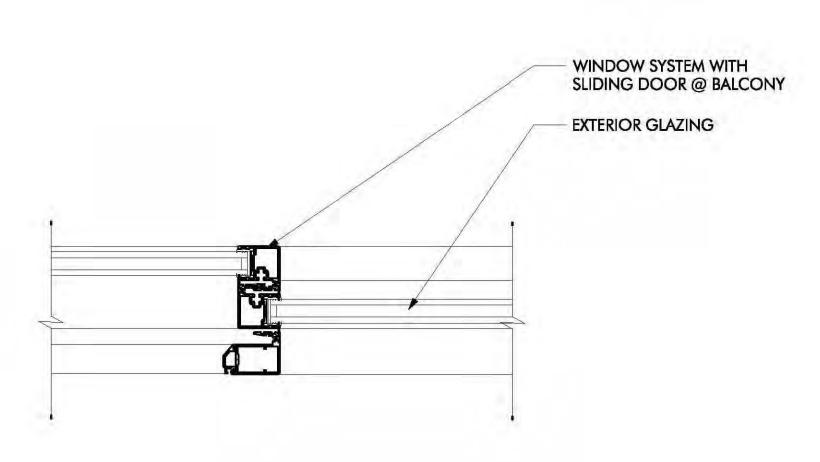




05 TYP STOREFRONT JAMB @ PLAZA LEVEL



O 1 TYP HM THRESHOLD DETAIL @ PLAZA LEVEL



08 TYPICAL SLIDING DOOR HEAD @ BALCONY SCALE: NO SCALE

EXTERIOR

6", MIN.

WOOD SIDING

EXTERIOR SHEATHING

- RIGID INSULATION

BATT INSULATION

DAMP PROOFING

INTERIOR GYP. BD.

INTERIOR

COPPER FLASHING, PROVIDE END DAMS

KYNAR COATED METAL HEAD EXTENSION

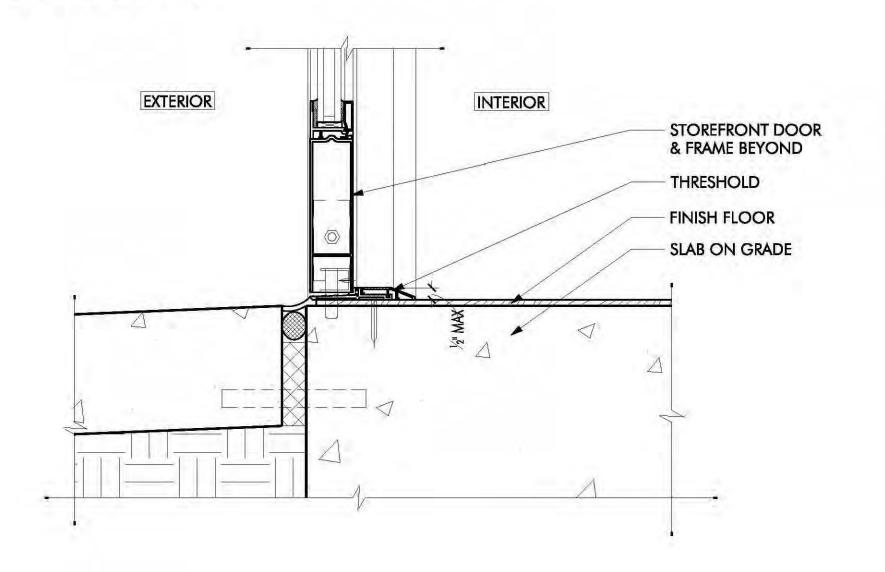
WINDOW SYSTEM WITH SLIDING DOOR @ BALCONY

BLDG. SHEET

BACKER ROD & SEALANT

WD. SHIM

07 TYPICAL SLIDING DOOR JAMB @ BALCONY SCALE: NO SCALE



04 TYP STOREFRONT THRESHOLD @ PLAZA LEVEL SCALE: 3" = 1'-0"

02 HM JAMB DETAIL @ PLAZA LEVEL STONE CLADDING SCALE: 3" = 1'-0"

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TYPICAL EXTERIOR **DETAILS**

project number 08131.100 date 11.18.2010

A6.01b

Page Bottom

BXUV.V432 Fire Resistance Ratings - ANSI/UL 263

Authorities Having Jurisdiction should be consulted in all cases as to the particular requirements covering the installation and use of UL Listed or Classified products, equipment, system, devices, and materials.
 Authorities Having Jurisdiction should be consulted before construction.

Design/System/Construction/Assembly Usage Disclaimer

- Fire resistance assemblies and products are developed by the design submitter and have been investigated by UL for compliance with applicable requirements. The published information cannot always address every construction nuance encountered in the field.

 When field issues arise, it is recommended the first contact for assistance be the technical service staff provided by the product manufacturer noted for the design. Users of fire resistance assemblies are advised to consult the general Guide Information for each product category and each group of assemblies. The Guide Information includes specifics concerning alternate materials and alternate methods of construction.

 Only products which hear LL's Mark are considered as Classified Listed or Recognized.
- Only products which bear UL's Mark are considered as Classified, Listed, or Recognized.

Fire Resistance Ratings - ANSI/UL 263

See General Information for Fire Resistance Ratings - ANSI/UL 263

(Exposed to Fire on Interior Face Only)

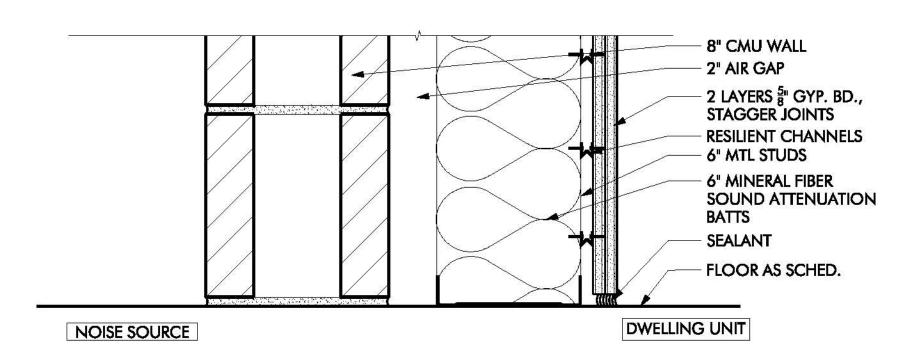
June 15, 2000

Bearing Wall Rating-1 Hr

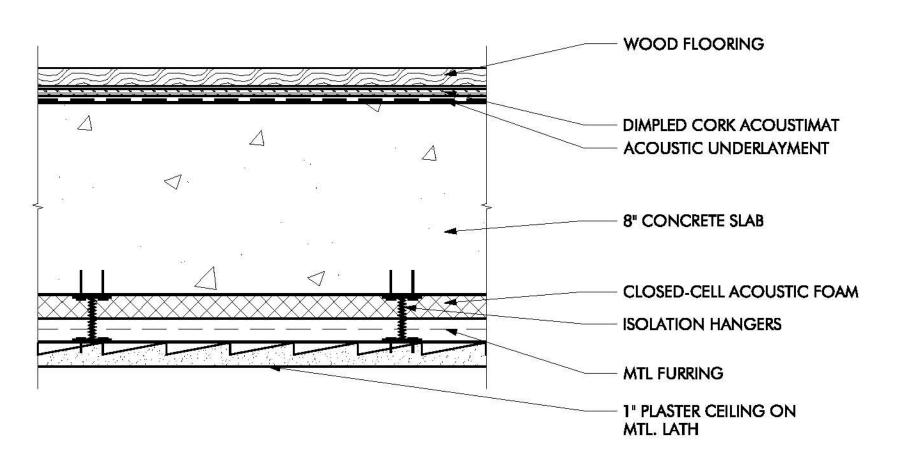
Load Restricted for Canadian Applications — See Guide BXUV7

1. Steel Floor and Ceiling Tracks — (Not Shown)—Top and bottom tracks of wall assemblies shall consist of steel members, min No. 20 MSG (0.0329 in., min bare metal thickness) steel or min No. 20 GSG (0.036 in. thick) galvanized steel or No. 20 MSG (0.033 in. thick) primed steel, that provide a sound structural connection between steel studs, and to adjacent assemblies such as floor, ceiling, and/or other walls. Attached to floor and ceiling assemblies with steel fasteners spaced not greater than 24 in. OC.

2. Steel Studs — Corrosion protected steel studs, min 3-1/2 in. wide, min No. 20 MSG (0.0329 in., min bare metal



O3 ACOUSTIC SEPARATION PARTITION DETAIL SCALE: 3" = 1'-0"



thickness) steel or min No. 20 GSG (0.036 in. thick) galvanized steel or No. 20 MSG (0.033 in. thick) primed steel, cold formed, shall be designed in accordance with the current edition of the Specification for the Design of Cold-Formed Steel formed, shall be designed in accordance with the current edition of the Specification for the Design of Cold-Formed Steel Structural Members by the American Iron and Steel Institute (AISI). All design details enhancing the structural integrity of the wall assembly including the axial design load of the studs, shall be specified by the steel stud designer and/or producer, and shall meet the requirements of all applicable local code agencies. The max stud spacing of wall assemblies shall not exceed 24 in. Studs attached to floor and ceiling tracks with 1/2 in. long Type S-12 steel screws on both sides of studs or by welded or bolted connections designed in accordance with the AISI specifications.

Lateral Support Members — (Not Shown)—Where required for lateral support of studs, support may be provided by
means of steel straps, channels or other similar means as specified in the design of a particular steel stud wall system.

4. Gypsum Board* — Any 5/8 in. thick (min) gypsum wallboard bearing the UL Classification Marking as to Fire Resistance. Applied vertically with Type S-12 steel screws, spaced 8 in. OC at edges and joints and 12 in. OC at the field. See Gypsum Board (CKNX) Category for names of Classified Companies.

Gypsum Sheathing* — Any 5/8 in. thick (min) gypsum sheathing bearing the Fire Resistance Classification Marking.
 Applied vertically and attached to studs and runner tracks with 1 in. long Type S-12 bugle head screws spaced 12 in. OC (max) along studs and tracks.

See Gypsum Board (CKNX) Category for names of Classified Companies.

6. Wood Structural Panel Sheathing — (Optional)—Min 7/16 in. thick, 4 ft wide wood structural panels, min grade "C-D" or "Sheathing". Installed with long dimension of panel (strength axis) or face grain of plywood parallel with or perpendicular to studs. Vertical joints centered on studs. Horizontal joints backed with steel straps or channels (Item 3), min 1-1/2 in. wide by No. 20 GSG (0.036 in. thick) galvanized steel or No. 20 MSG (0.033 in. thick) primed steel, when specified in the design of a particular steel stud wall system. Sheathing attached over gypsum sheathing (Item 5) to studs and tracks with Type S-12 bugle head screws; or No. 8 self-drilling/tapping steel screws; or min 0.100 in. diam. Hardened steel nails with helical or knurled thread (min sheathing fastener length equivalent to total thickness of gypsum and wood structural panel sheathing and steel framing plus 1/4 in.). Sheathing fasteners spaced min 6 in. OC at perimeter of panels and 12 in. OC along interior studs. and 12 in. OC along interior studs.

Exterior Facings — Installed in accordance with manufacturer's installation instructions and local building code requirements. One of the following exterior facings is to be applied over sheathing (Item 5 or 6 (if used)):

A. Siding — Aluminum, steel or vinyl siding attached over sheathing to studs.

B. Brick Veneer — Any type of nom 4 in. wide brick veneer. When brick veneer is used, the rating is applicable with exposure on either face. Brick veneer fastened with corrugated metal Wall ties attached over sheathing to steel studs with No. 6 steel screw per tie; ties spaced not more than each sixth course of brick. Min 1 in. air space provided between brick veneer and sheathing.

C. Particleboard Siding — Hardboard exterior sidings including patterned panel or lap siding. D. Wood Structural Panel or Lap Siding — APA Rated Siding marked Exterior, plywood, OSB, or composite panels with veneer faces and structural wood core, per PS 1 or APA Standard PRP-108, including textured, rough sawn, medium density overlaid, brushed, and grooved panels or lap

E. Stucco — Portland cement type - min nom 3/4 in. thickness. Metal lath or mesh base fastened over sheathing to steel studs with No. 6 screws or other approved fasteners for attaching lath to

F. Exterior Insulation and Finish System (EIFS) — Nom 1 in. Foamed Plastic* insulation bearing the UL Classification Marking, attached over sheathing and finished with coating system, or Portland cement or synthetic stucco systems, in accordance with manufacturer's instructions. See Foamed Plastic (BRYX or CCVW) category for names of Classified companies.

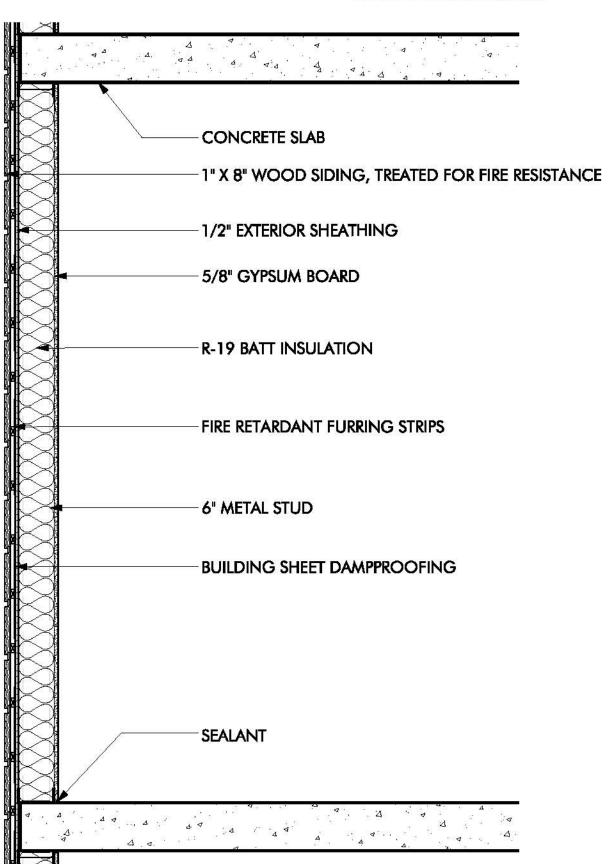
8. Batts and Blankets* — Any glass fiber or mineral wool insulation bearing the UL Classification marking as to fire resistance, of a thickness to completely fill stud cavity. See Batts and Blankets (BKNV or BZJZ) category for names of

*Bearing the UL Classification Mark

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Page Top



CODE ANALYSIS:

AS PER IBC 2003 FOR R2 OCCUPANCY GROUP AND TABLE 503 THIS IS A TYPE IB CONSTRUCTION.

FIRE RATING REQUIREMENTS FOR EXTERIOR WALLS AS PER TABLE 601/602 1 HR.- FOR DISTANCE FROM ADJACENT BUILDING BETWEEN 10'-0" TO 30'-0"

0 HR.- FOR DISTANCE FROM ADJACENT BUILDING MORE THAN 30'-0"

MORE THAN 90% OF THE BUILDING EXTERIOR WALLS ARE NOT REQUIRED TO BE FIRE RATED AS PER CODE. ONLY 10% OF THE EXTERIOR WALL WILL BE REQUIRED TO BE FIRE RATED.

0 1 WALL SECTION @ FIRE RATED EXT WOOD SIDING SCALE: 3" = 1'-0"

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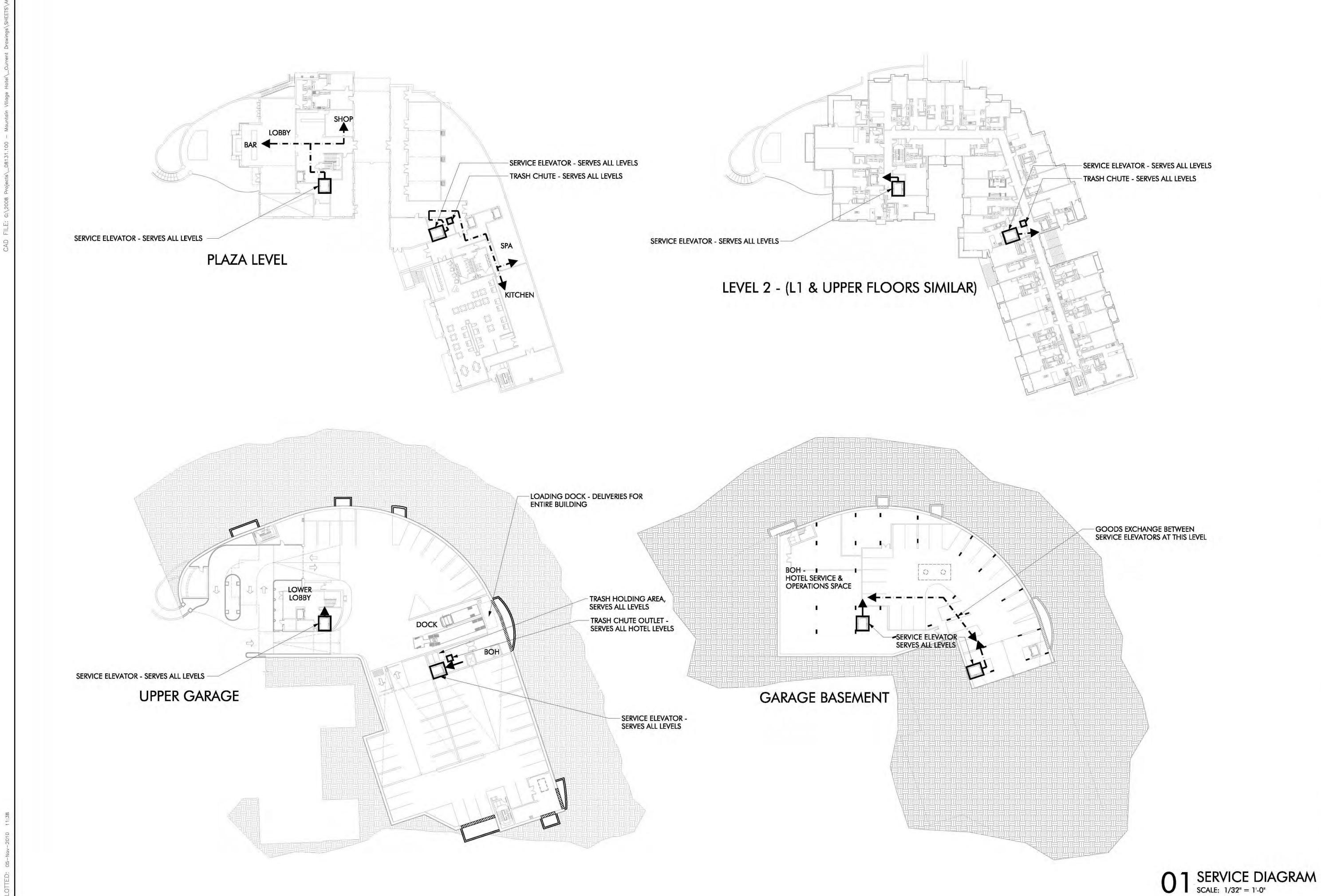
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MISCELLANEOUS DETAILS

project number 08131.100 date 11.18.2010

A6.02

02 ACOUSTIC SEPARATION CEILING DETAIL
SCALE: 3" = 1'-0"





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Reg. No.: 400465

owner/applicant
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project
Mountain Village Hotel
628, 632, 636, 642, 683
Mountain Village Blvd,
Mountain Village Blvd,
Mountain Village, CO 81435
LOTS 73-76R, 89A, 109,110
LOT 73-76R AND TRACT OS-3BR-1, TOWN OF MOUNTAIN
VILLAGE, A REPLAT, REZONE, AND DENSITY TRANSFER OF
LOTS 73 AND 76, TOWN OF MOUNTAIN VILLAGE AND REPLAT
OF TRACT OS-3BR, TOWN OF MOUNTAIN VILLAGE,
ACCORDING TO THE PLAT RECORDED JANUARY 25, 2007 IN
PLAT BOOK 1 AT PAGE 3807 AT RECEPTION NO. 389801,
COUNTY OF SAN MIGUEL, STATE OF COLORADO.

LOT 109, REPLAT NO. 3, TELLURIDE MOUNTAIN VILLAGE, FILING 1, ACCORDING TO THE PLAT RECORDED JULY 31, 1985 IN PLAT BOOK 1 AT PAGE 577, COUNTY OF SAN MIGUEL, STATE OF COLORADO.

LOT 110, REPLAT NO. 3, TELLURIDE MOUNTAIN VILLAGE, FILING 1, ACCORDING TO THE PLAT RECORDED JULY 31, 1985 IN PLAT BOOK 1 AT PAGE 577, COUNTY OF SAN MIGUEL, STATE OF COLORADO.

ACCESS TRACT 89-A, TELLURIDE MOUNTAIN VILLAGE,
ACCORDING TO THE FIRST REPLAT OF COMBINED LOTS 133
AND 89-1, TELLURIDE MOUNTAIN VILLAGE, FILING 1
RECORDED DECEMBER 26, 1989 IN PLAT BOOK 1 AT PAGE 980,
COUNTY OF SAN, MIGUEL, STATE OF COLORADO.

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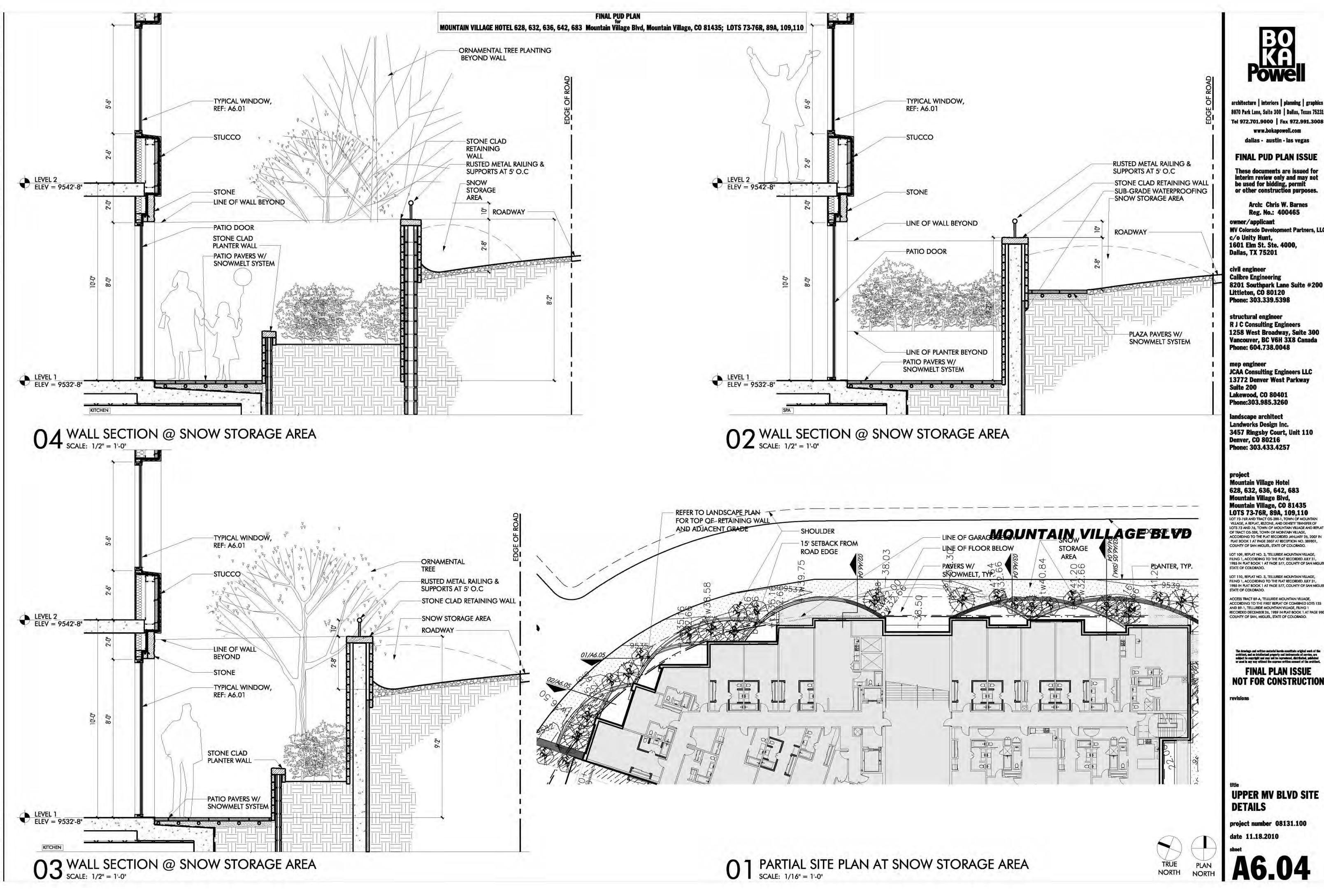
FINAL PLAN ISSUE NOT FOR CONSTRUCTION

revision

SERVICE DIAGRAM

project number 08131.100 date 11.18.2010

A6.03



FINAL PUD PLAN ISSUE

These documents are issued for interim review only and may not be used for bidding, permit or other construction purposes.

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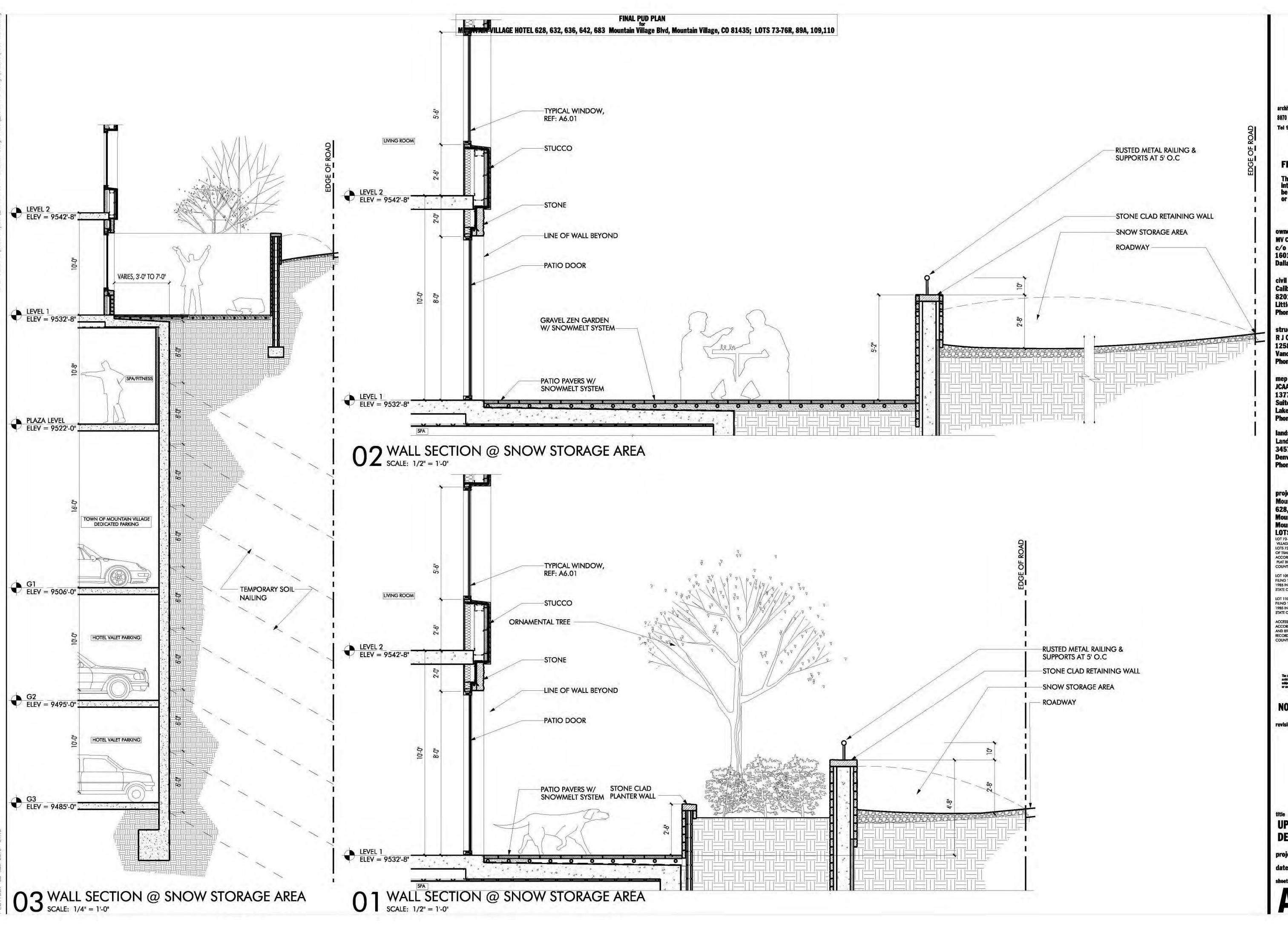
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UPPER MV BLVD SITE DETAILS





dallas - austin - las vegas

FINAL PUD PLAN ISSUE

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FINAL PLAN ISSUE NOT FOR CONSTRUCTION

UPPER MV BLVD SITE DETAILS

project number 08131.100 date 11.18.2010

A6.05

Agenda Item No. 11



COMMUNITY DEVELOPMENT DEPARTMENT PLANNING DIVISION

455 Mountain Village Blvd. Mountain Village, CO 81435 (970) 728-1392

TO: Design Review Board

FROM: Chris Hawkins, Director of Community Development

FOR: Meeting of May 7, 2015

DATE: May 1, 2014

RE: Conceptual Worksession to Discuss a Proposed Rezoning, Density Transfer and

Replat for Lots 376R and Lot 387R

PROJECT GEOGRAPHY

Legal Description: Lot 376R & Lot 387R **Address:** 127 & 129 Rocky Road

Applicant/Agent: Law Offices of Thomas G. Kennedy

Owner: Yellow Brick Road CO, LLC **Zoning:** Single-family Zone District

Existing Use: Vacant

Proposed Use: Single-family, open space, equestrian

Lot Area: Approximately 59.8 acres

Adjacent Land Uses:

North: Single-familySouth: USFS/Ski ResortEast: Single-family

West: Ski Ranches Subdivision, Single-family

ATTACHMENTS

Exhibit A: Applicant Narrative Exhibit B: Conceptual Plan Set

RECORD DOCUMENTS

- Town of Mountain Village Community Development Code (as adopted March 2013)
- Town of Mountain Village Home Rule Charter (as amended on June 28, 2005)
- Design Review Application as maintained by the Community Development Department.

BACKGROUND

The owner of the property is proposing a unique clustered development where the two existing single-family lots are rezoned and replatted as shown in the conceptual plans as summarized as follows:

Two single-family areas that are approximately 2.3 and 7.5 acres in size, with the larger area proposed to be split into two lots for a maximum of three-single family dwellings (Please refer to the accessory dwelling unit discussion below).

- > Three passive open space tracts containing approximately 34 acres in size
- ➤ Three active open space tracts containing approximately 10 acres
- One right-of-way active open space tract containing approximately 5.6 acres

The clustered plan would create a development with approximately 83% open space.

The applicant is proposing that the 7.5 acre single-family area be subdivided to have an accessory dwelling unit larger than permitted by the Community Development Code (CDC). The owner has agreed to tie this larger accessory dwelling unit to the primary unit in perpetuity, and not allow for the separate sale or conveyance even though it will be on a separate lot. Staff is supportive of this unique approach since the increase in density requires a density transfer of one single-family dwelling to the site, and the lot cannot be conveyed separately from the adjacent primary home.

The owner also desires to have a caretaker and maintenance facility located by the recently built bridge. The development is also seeking approval for equestrian stables and a small pasture area. Both of these uses will necessitate future conditional use permit applications based on the proposed Full Use Active Open Space zoning in the workforce housing and stable areas.

A pending amendment to the CDC establishes the following key policies for the DRB's consideration:

17.3.4(F)(4)

- 1. Further Subdivision Limited. Single-family lots may not be further subdivided to create additional lots and additional density may not be transferred onto a single-family lot; provided, however, a single-family lot may be further subdivided and additional density may be transferred onto a single-family lot by the Rezoning Process in limited situations only if:
 - a. The density is currently permitted on a lot; or
 - b. The Comprehensive Plan envisions higher density; or
 - c. A PUD is approved pursuant to the PUD Regulations; or
 - d. The Town Council determines that the rezoning is exceptional and meets conditions to mitigate the upzoning, such as but not limited to clustered development, the provision of additional open space, or other community benefits: and.
 - e. The rezoning is compatible and fits with surrounding area development.

The applicant is proposing the rezoning dues to the following exceptional considerations:

- 1. Clustered development with 83% open space.
- 2. Forestry management/fire mitigation across the whole 59 acres.
- 3. Construction of workforce housing.

The Town approved and the owner has constructed a bridge across the ski run to the platted lots. The owner has also been working on implementing a forest management plan for the property. In addition, the owner has been working with the Town on the first driveway segment from the bridge leading to the proposed single-family lots, with a Design Review Process application to be submitted for such in the very near future.

The property owner has reached an agreement with Ski Ranches to relocate a trail route through the property that provides access to trails heading east and southeast from the property. The current trail is the route for a water and gas line that the owner has agreed to relocate, with the Town and Source Gas working closely with the applicant's team on plans.

RECOMMENDATION

Staff is very supportive of the rezoning because we believe it provides for the community benefits outlined above, while also being very compatible with surrounding area development. Staff recommends that the DRB provide a general comments and direction on the proposal. As outlined in CDC Section 17.4.6.E, any comments or general direction by the DRB shall not be considered binding or represent any promises, warranties, guarantees and/or approvals in any manner or form. A conceptual worksession shall not be construed as a comprehensive review of the proposal under discussion, and as such, additional issues and/or concerns will most likely arise as part of the formal development review process.

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CONCEPTUAL WORKSESSION REPLAT/REZONE/DENSITY TRANSFER

NARRATIVE

April 20, 2015

Owner: Yellow Brick Road CO LLC

YBR Property: Lots 376RA, 387R and Access Tract A-376R

The Owner has requested that the Town schedule a worksession with the Town Design Review Board (DRB) and the Town Council to review and provide guidance on certain development plans being proposed by on the YBR Property. Of particular note, the Owner plans to submit a formal application with the Town seeking to replat/rezone and transfer density to the YBR Property to accommodate certain single-family development and related uses and activities as outlined in this narrative. This narrative describes the Owner's proposed development goals and objectives to be discussed by the Town as part of the requested worksession.

A draft of a Replatting and Rezoning map for the YBR Property consistent with development plan outlined in this Narrative is being submitted with the worksession application. In addition, an existing conditions map is likewise being submitted with the worksession application.

YBR Property Description

The YBR Property is located along the southwesterly edge of the Mountain Village, adjoining the Marmot ski run to the east, the Telluride Ski Ranches subdivision (located outside of the municipal boundaries of the Town and within the unincorporated boundaries of San Miguel County) to the west, USFS land to the south and Lot 388, Mountain Village to the north. A vicinity map is appended as **Exhibit "A"**. 1

The YBR Property cumulatively consists of approximately 59.774 acres (mol) and is currently platted as follows:

- Lot 376RA (15.04 acres and zoned for single-family residential)
- Lot 387R (44.449 acres and zoned for single-family residential)
- Access Tract A-376R (0.285 acres)

The YBR Property is heavily forested. A Forest Management Plan has been prepared for the YBR Property and was reviewed and accepted by the Town. The Forest Management Plan is being implemented by the Owner and will take several seasons to complete.

¹ The YBR Property was not part of the originally platted boundaries of the Mountain Village. The Telluride Company acquired certain lands from the USFS as part of a land exchange that was completed in the early 1990's. The acquired property was subsequently annexed into the Mountain Village and platted as four large lots, including the two lots that constitute the YBR Property and Lots 388 and 420R to the north. At the time that the USFS land exchange was being completed and its development in the Mountain Village was being considered, to address concerns introduced by the Ski Ranches and the Goodman family (owners of a lot in the Ski Ranches), The Telluride Company, the Ski Ranches and the Goodman family entered into an agreement establishing certain use restrictions on portions of the YBR Property ("Ski Ranches/Goodman Agreement"). The Ski Ranches/Goodman Agreement also established certain usage rights for certain recreational trails on the YBR Property. At the outset the Owner wishes to acknowledge the use restrictions established in the Ski Ranches/Goodman Agreement and confirm that the proposed development of the YBR Property conforms to those restrictions.

The topography of the YBR Property experiences a variety of undulations, with a high point in its southerly portion and along both sides of the Skunk Creek riparian area, which runs in a north/south alignment generally in the westerly half of the YBR Property. An area of wetlands is located along a portion of the middle easterly edge of the YBR Property near the access bridge.

The YBR Property is currently unimproved, although a certain existing road/trail is present on the YBR Property, which accommodates an existing waterline and gasline for the benefit of the Telluride Ski Ranches and crosses a portion of the YBR Property in an east-west direction. The waterline road/trail also accommodates certain private recreational uses (hiking, biking, equestrian, cross-country skiing) by members of the Telluride Ski Ranches and Goodman Family, consistent with easement agreements between the parties that were contained in the Ski Ranches/Goodman Agreement. A recent modification to the Ski Ranches/Goodman Agreement allows Yellow Brick to relocate the trail. The existing and relocated waterline/gasline/Goodman-Ski Ranch trail is being accounted for in the Owner's development plans for the YBR Property.

Development Goals/Objectives

The YBR Property is currently platted as two lots and zoned single-family residential. As currently platted and zoned, the Owner has the right to develop a main residence, an accessory dwelling unit (up to 1500 sf) and accessory buildings and structures on each of the two lots, as the same are allowed by the Mountain Village Community Development Code (CDC). Development of these improvements would be accessed from Rocky Road, over Access Tract A1-F26 and access easement AE-376. The Owner constructed an access bridge within access easement AE-376 to serve the YBR Property last year. The Owner will construct an internal access road to serve its allowable development. Utilities will be extended to the YBR Property from various corridors adjacent to the YBR Property.

The Owner seeks to cluster its development activities within the southeasterly portion of the YBR Property. The remainder of the YBR Property will be platted and zoned as private open space parcels in the areas that development is not occurring.

Proposed Land Uses and Activities

The particular land use activities proposed by the Owner are as follows:

- Re-subdividing the YBR Property into a total of three single-family lots, an access tract and a series of open space parcels
- Providing for the construction of a total of three main residential structures and one Accessory/Guest House
- Constructing a pod near that entry to the YBR Property that would accommodate aWorkforce Housing Unit, secured entry area and equipment/vehicle storage structure
- Constructing a private equestrian stable for use by the Owner and guests

The proposed land use allocation/break down of the YBR Property contemplated by the Owner in its development plan is as follows:

| Land Use | Acreage | Percentage of Yellow Brick Road YBR Property |
|----------------------------------|---------|---|
| Access Tract for shared driveway | 5.57 | 9.33% |
| Active Open Space Parcel | 10.01 | 16.76% |
| Passive Open Space Parcel | 34.31 | 57.44% |
| Residential Lots | 9.84 | 16.46% |

As noted, less than 10% of the YBR Property would be allocated to residential development. About 6% for a shared road and utility corridor and the balance allocated to open space. 74% of the YBR Property will be placed in open space. 57% of the YBR Property will be zoned as passive open space. The low density, clustered development will result in compatible development for the uses and activities occurring in the area.

The clustering will reduce the overall impact of development on the YBR Property, by creating the need for only one corridor for a shared driveway and one corridor for shared utilities. Areas proposed for development will be located away from important environmental zones. The Owner is placing the Skunk Creek riparian area, certain wetland areas and other wildlife corridors into undeveloped Passive Open Space.

<u>Implementation - Replatting/Rezoning/Density Transfer.</u> To authorize these land uses, the Owner proposes to proceed with a replat/rezone/density transfer application for the YBR Property, which is a two-step process, with a review/recommendation by the Design Review Board and an action/decision by Town Council, both at noticed, public meetings.

• General Provisions

- o The Owner seeks to replat the two existing lots into a series of three smaller residential lots, an access tract and various privately owned open space parcels. A draft of a Replatting and Rezoning map is being submitted with the worksession application illustrating this proposed division of land.
- As described above and depicted on the Replatting and Rezoning map, development would be concentrated within the southeasterly portion of the YBR Property.
- Within this southeasterly quadrant of the YBR Property, the residential development would generally occur in two pods, one is referred to as the "upper area" and the other is referred to as the "lower peninsula area"

• Development within Upper Area

- Within the upper area, the Owner proposes to create two residential lots (namely Lot 387R-A and Lot 387R-B). One lot (Lot 387R-A) will accommodate a main residential structure and the other lot (Lot 387R-B) would accommodate a second residence that would effectively serve as a supporting residence subordinate in size to the main residence on Lot 387R-A but important in location, use and functionality to the main residence. Since this supporting residence is larger than the 1500 sf lot that is allowed for an accessory dwelling unit under the CDC, the Owner is proposing to: (i) plat a separate lot, which would be zoned single-family residential and (ii) buy and transfer a singlefamily density right to support its platting and zoning. Lot 387R-A and Lot 387R-B would each be zoned single-family residential. The area for both Lot 387R-A and Lot 387R-B is shown on the draft Platting/Zoning Plan for the YBR Property. This area contains approximately 7.50 acres. The Owner is still working on finalizing the siting for residences in this area, so the final lot boundaries, dimensions and size will need to be established with the filing of Owner's formal application for replat/rezone/density transfer. These lots would be of varying shapes and sizes to be determined, but probably in the range of about 3-4 acres.
- In recognition of overall massing concerns that could occur with this replatting, the
 Owner is proposing a series of restrictions, which would be placed on namely Lot 387R-

A and Lot 387R-B, which would be enforceable by and would run to the benefit of the Town.

- Lot 387R-A and Lot 387R-B would be required to be owned together and could not be separately owned or conveyed.
- Lot 387R-A would be restricted to only a main residence, an Accessory Dwelling/Guest House would not be allowed on Lot 387R-A
- The supporting residence on Lot 387R-B would be restricted to a maximum size not to exceed 5000 sf of livable space. No other residence would be allowed to be constructed on Lot 387R-B. The Owner has acquired a single-family density unit from an owner in the density bank, which will be included on and applied to Lot 387R-B as part of a replat/rezone/density transfer application.
- Development Within The Lower Peninsula Area. Within the lower peninsula area, the Owner proposes to create one lot, namely Lot 376R-AR, which would accommodate a main residence and typical an Accessory/Guest House. Lot 376R-AR would contain 2.34 acres. Lot 376R-AR would be zoned single-family residential. The Owner does not envision constructing the main residence on Lot 376R-AR at this time, rather, the focus of the development would be a small cabin that would not exceed the 1500 sf maximum size allowed by the CDC.
- Access Tracts. The Owner will plat an Access Tract connecting the newly constructed bridge to
 each of the lots, insuring that each lot will have frontage on and access to the Access Tract. A
 shared driveway serving all development contemplated on the YBR Property will be located
 within the Access Tract.

• Open Space Parcels.

- Within the southeasterly portion of the YBR Property where the upper and lower residential pods have been identified, the Owner will plat three open space parcels, namely Tract OS 387-1, Tract OS 387-2 and Tract OS 387-3.
- o Tract OS 387-1 and Tract OS 387-3 would be zoned Full Use Active Open Space to allow the placement of a workforce housing unit to house an onsite employee of Owner, a maintenance/equipment structure and a modest private equestrian boarding facility to accommodate horses of the Owner. A tubing hill is also contemplated on these parcels. The Owner would place a covenant on the YBR Property running to the benefit of the Town, which would preclude these parcels from being used for other more expansive uses that are otherwise allowed in the Full Use Active Open Space zone. The CDC requires that the Owner apply for and obtain a Conditional Use Permit to allow the placement, use and operation of a workforce housing unit and the equestrian operation on Tract OS 387-1 and Tract OS 387-3. These applications would either be run simultaneously with the replat/rezone/density transfer or follow thereafter, depending on design and siting considerations of the workforce housing unit, maintenance/equipment structure and equestrian boarding facility. A Conditional Use Permit is a two-step review process by the DRB and Town Council following notice and a public hearing.
- Tract OS 387-2 would be zoned as Passive Open Space and would accommodate the relocated Goodman/Ski Ranch Trail and portions of the relocated waterline and gasline.

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² The Owner would own the Workforce Housing Unit and lease it to its full time employee. Owner understands that it will need to seek an exemption to the Town's Workforce Housing regulations to be able to own and lease the unit as described.

- o The Owner will plat Tract OS 387-4 in an area to the north of the development area, which adjoins Tract OS 387-1 and the equestrian stabling area. Tract OS 387-4 will serve as a pasturing area for the Owner's horses and will require the need for the placement of wildlife friendly fencing to contain the area. Because this usage requires the same zoning designation that would accommodate the equestrian stabling area, Tract OS 387-4 will be zoned Full Use Active Open Space, but will be subjected to the same covenants proposed for similarly zoned Tract OS 387-1 and Tract OS 387-3
- O The balance of the YBR Property, some 32.5 acres, will be platted as Tract OS 387-5 and Tract OS 387-6 and will zoned Passive Open Space. Portions of Tract OS 387-5 and Tract OS 387-6 will accommodate the balance of the relocated Goodman/Ski Ranch Trail and portions of the relocated waterline and gasline. It is contemplated that of Tract OS 387-5 and Tract OS 387-6 will largely undisturbed, other than for forest management, underground utilities and private recreational trails.

• Justification for the Rezoning/Density Transfer.

- The platting/zoning plan will result in the creation and placement of some 44 acres of land (mol) as open space, which is roughly 70% of the YBR Property. The portions of the newly created open space parcels that will be platted as passive open space embrace important environmental areas such as the Skunk Creek riparian area, wetland areas located on the YBR Property and wildlife corridors that align with these riparian areas.
- o The designation of these open space parcels for Passive Open Space likewise conforms to the use covenants provided for in the Ski Ranches/Goodman Agreement.
- o The clustering will concentrate development of the 60 acre YBR Property holdings into a designated area and reduce overall impacts associated with development to much smaller portion of the YBR Property. The clustering will enable the construction of one shared road serving the development, thus eliminating the need for multiple road corridors and utility corridors. This will reduce the overall need for clearing/grading and tree removal
- Owner has generated a table showing the size of nearby platted lots in the Mountain Village, see attached Exhibit "B". The average size of these nearby lots is 1.33 acres, with two larger lots, namely Lot 351 being 2.675 acres and Lot 352R, which is a replat of Lots 348 and 352 being larger at 5.8 acres. As such, the lots being proposed for the YBR Property are in keeping with the scale of other platted residential lots in the Mountain Village. Lot 388 to the north was platted and annexed into the Town with the YBR Property, contains 39.317 acres. Lot 388 and the YBR Property were part of the same land exchange in the early 1990's that was completed by The Telluride Company with the USFS. Four large lots were platted and annexed into the Town at that time.
- o The development includes the construction and use of a workforce housing unit, which would be occupied by employees of the Owner.
- The implementation of the Forest Management Plan for the YBR Property, which undertaking will greatly exceed the requirements and expectations by the Town under the provisions of the CDC which govern forest management. The implementation of the plan will substantially improve the overall health of the forest and greatly reduce fire hazards at this important location at the edge of the Mountain Village.

The Owner looks forward to presenting this information to the Town Council and DRB during the worksession.

Exhibit "A" (Vicinity Map)

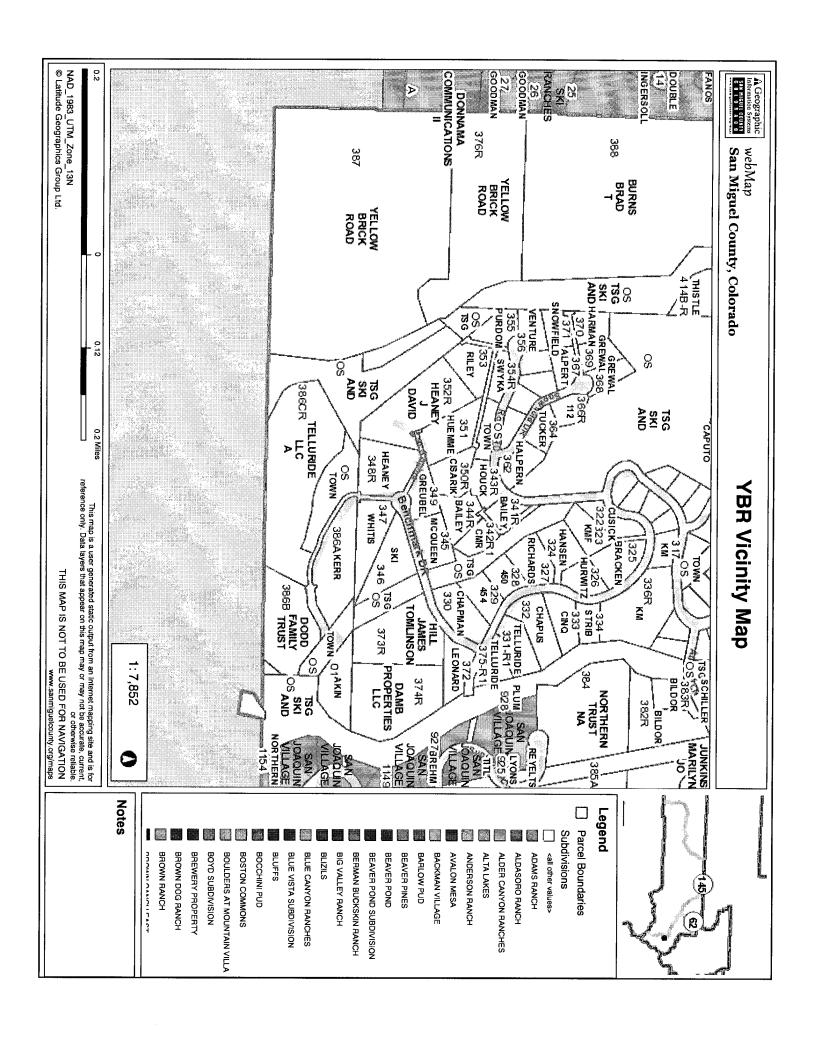


Exhibit "B" (Adjoining Lot Table)

| Lot | Lot Size (acres mol) |
|------------------------|----------------------|
| 351 | 2.675 |
| 352R (replat combining | 5.8 |
| Lots 348 and 352) | |
| 353 | 1.9 |
| 354R | 0.959 |
| 355 | 1.3 |
| 356 | 1.140 |
| 357 | 0.789 |
| 358 | 0.820 |
| 371 | 1.067 |
| | |
| | |
| | |
| | |
| | |
| | |

Exhibit "C" (Proof of Ownership)

432114
Page 1 of 3
SAN MIGUEL COUNTY, CO
M. KATHLEEN ERIE, CLERK-RECORDER
03-17-2014 01:10 PM Recording Fee \$21.00
Documentary Fee: \$1,085.00

RIEBEL

WITNESSETH, That the granter for and in consideration of the sum of Ten Allition Eight Hundred Fifty Thousand and 60/100 Dollars, the receipt and sufficiency of which is hereby acknowledged, has granted, bargained, sold and conveyed, and by these presents does grant, bargain, sell, convey and confirm unto the grantee, his licits and essigns forever, all the real property together with improvements, if any, situate, lying and being in the Country of San Aliguel and State of Colorado described as follows:

Contilant ash tendret sing state of consegue accounce as tonown

Lots 376RA, 387R and Access Tract A-376R, according to the Replat of Lots 376R, 387, Access Easement AE-376 and Access Tract A-376, The Town of Mountain Village, recorded January 14, 2005 in Plat Book I at page 3427 and Correction Plat recorded March 4, 2014 in Plat Book I at page 4634, County of San Miguel, State of Colorado)

TOGETHER WITH

Access Easement recorded January 14, 2005 at Reception No. 371761 and First Amendment to Access Easement Agreement recorded December 23, 2013 at Reception No. 431117;

TOGETHER WITH

Access Rights created over and across Access Tract A1-F26 pursuant to Access Tract A1-F26 Agreement recorded March 4, 2014 at Reception No. 431896.

as known by street and number as: Vacant Land, Mountain Village, GO 81435

TOGETHER with all and singular the hereditaments and appurtenances thereto belonging, or in anywise apportaining, and the reversion and reversions, remainder and remainders, rents, issues and profits thereof, and all the estate, right, title, interest, claim and demand whatsoever of the granter, either in law or equity, of, in and to the above bargained premises, with the hereditaments and oppurtenances.

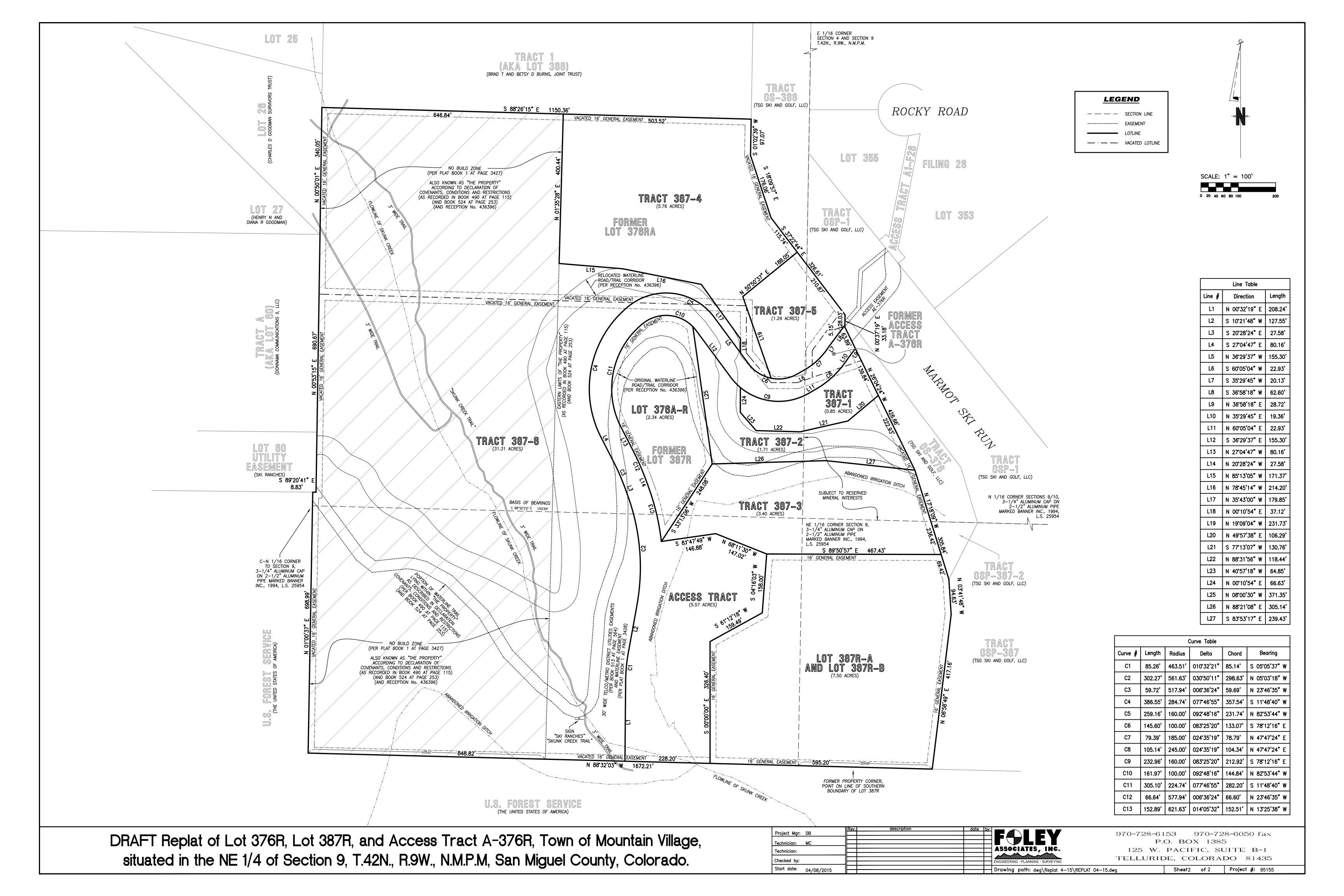
TO HAVE AND TO HOLD the said premises above bargained and described, with the appurtenances,

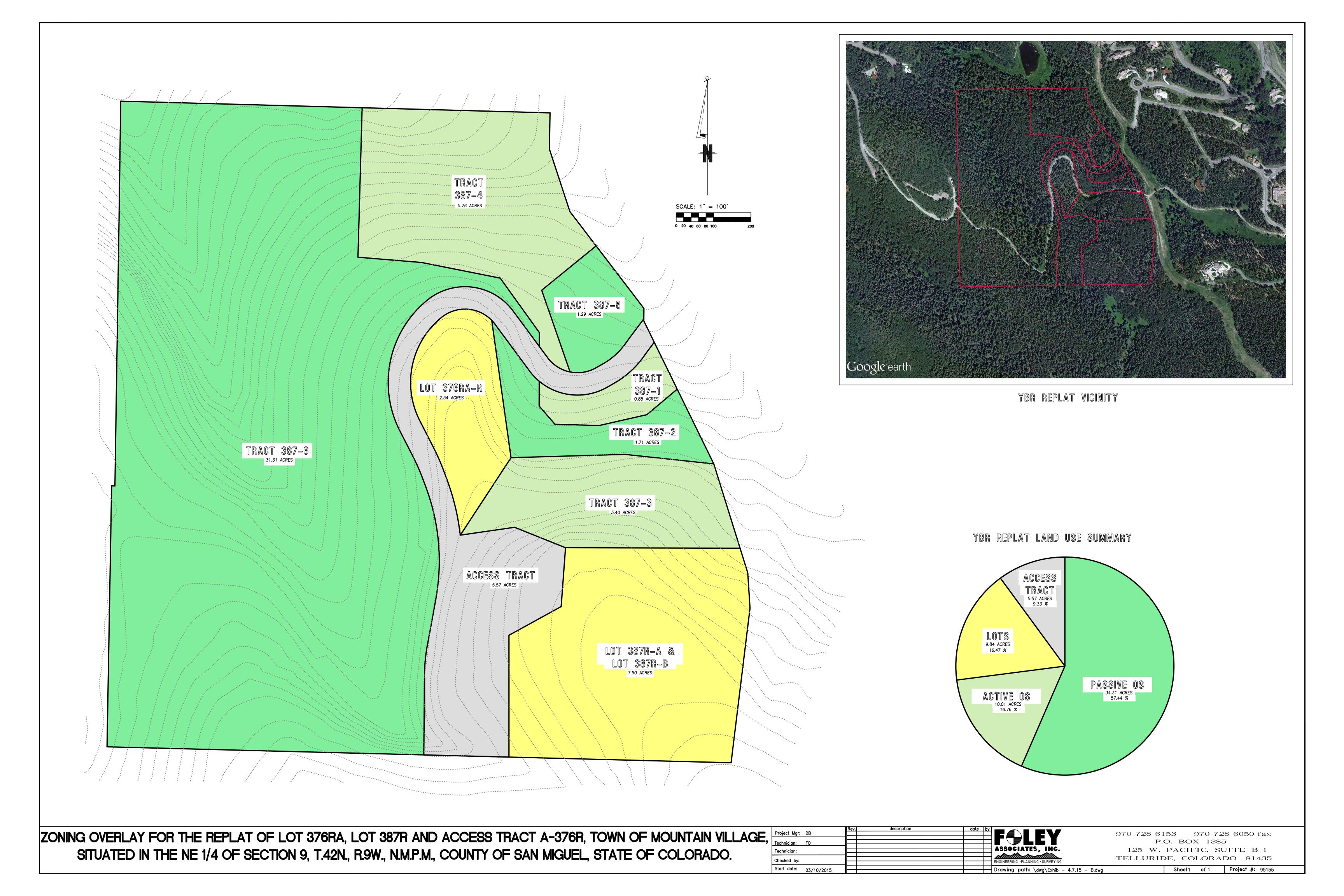
TO HAVE AND TO HOLD the said premises above bargained and described, with the appurenances, unto the grantee, list letter and ossigns forever. And the granter, for hinself, his heirs, and personal representatives, does covenant, grant, bargain, and agree to and with the grantee, his heirs and ossigns, that at the time of the casealing and delivery of these presents, he is well seized of the premises above conveyed, has good, sure, perfect, absolute and indefeasible estate of inheritance, in law, in fee simple, and has good right, full power and lawful authority to grant, bargain, sell and convey the same in manner and form as aforesaid, and that the same are free and clear from all former and other grants, bargains, sales, liens, taxes, assessments, encumbrances and restrictions of whatever kind or nature what so over, except

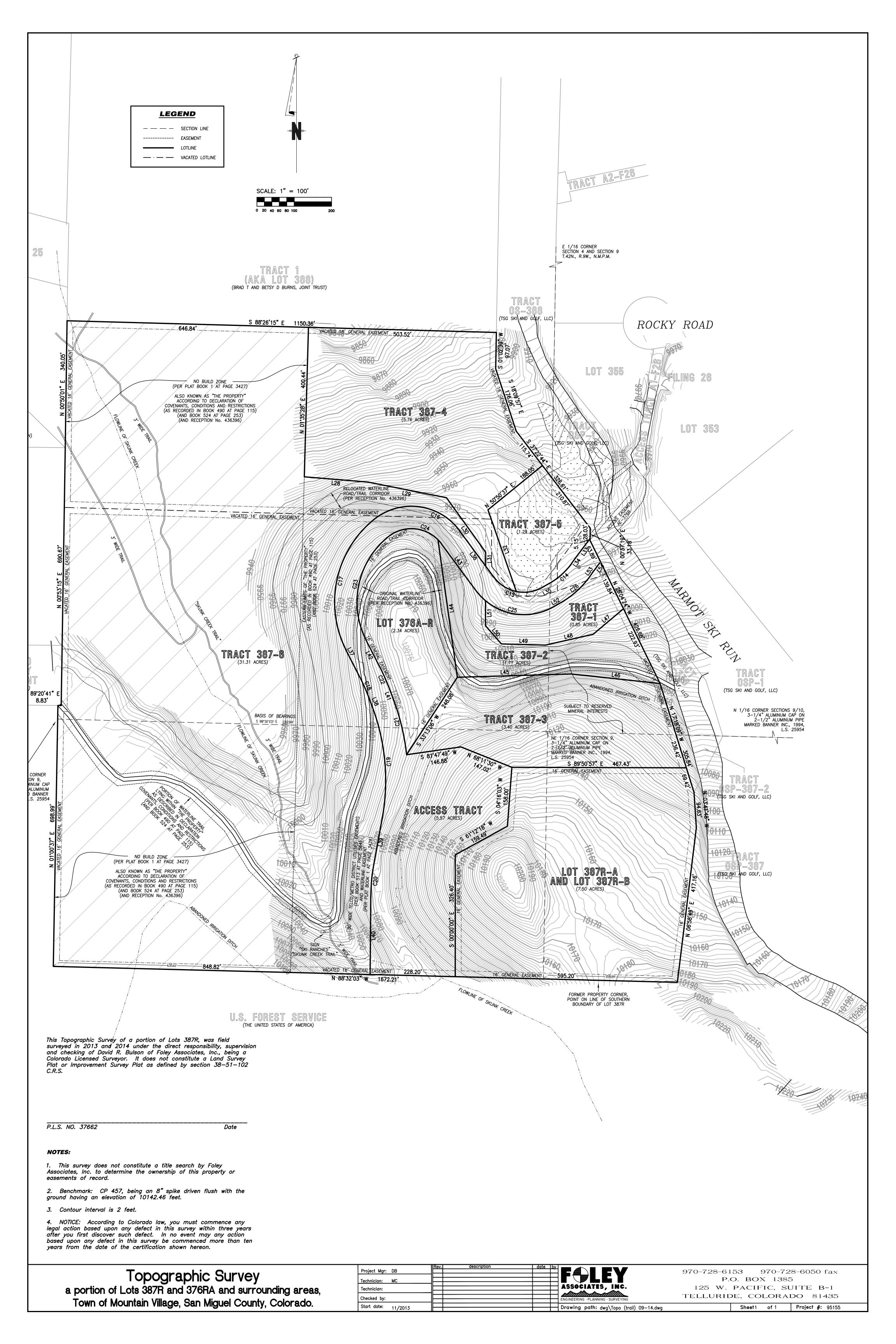
SEE ATTACHED EXHIBIT EXC - 251CEA

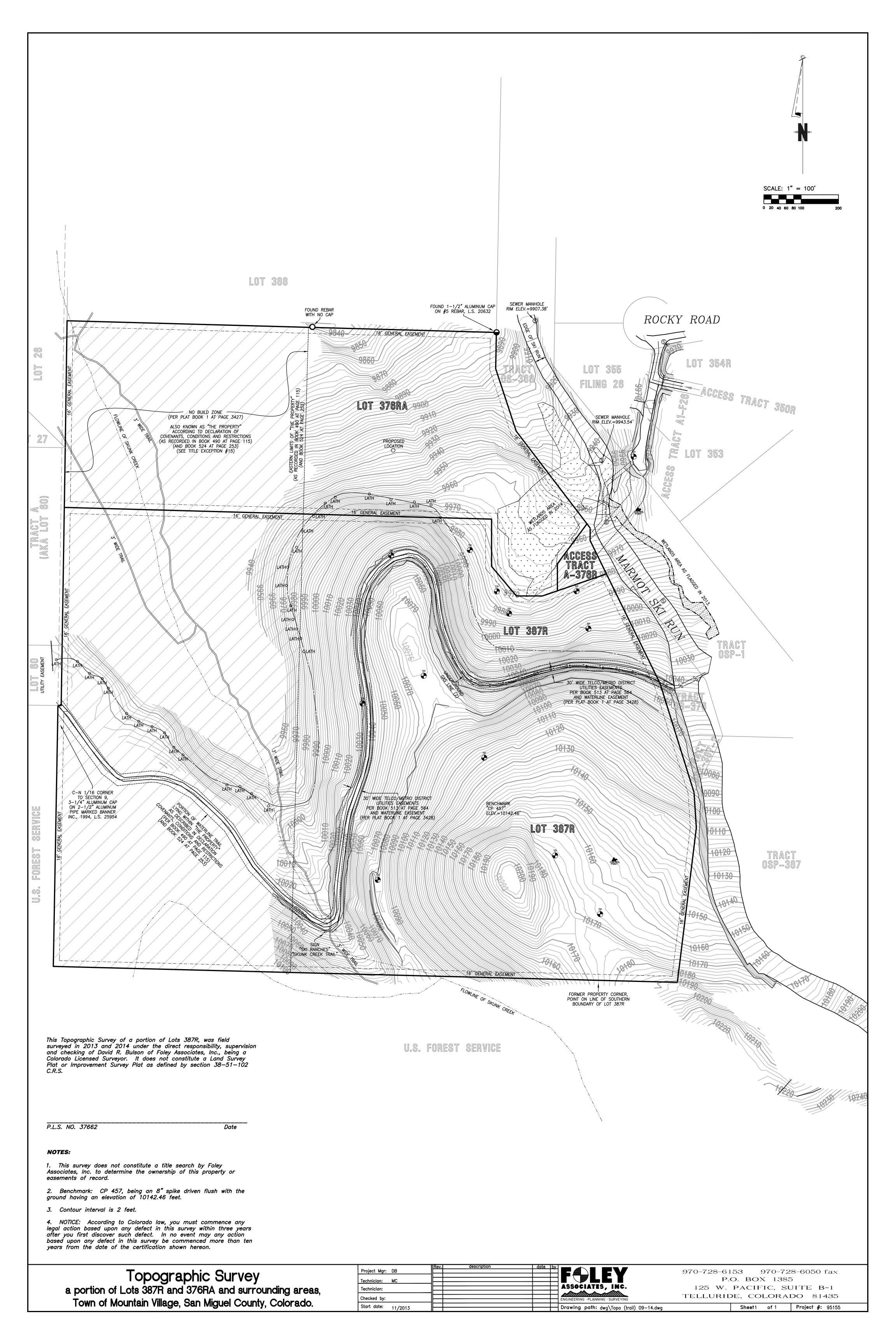
The granter shall and will WARRANT AND FOREVER DEFEND the above-bargained premises in the quiet and peaceable passession of the grantee, his helts and assigns, against all and every person or persons lawfully claiming the whole or any part thereof. The singular number shall include the plural, the plural the singular, and the use of any gender shall be applicable to all genders.

IN WITNESS WHEREOF, the grantor has executed this deed on the date set forth above.













SIGN-IN SHEET

DRB Meeting Thursday, May 7, 2015 Please write clearly

| ATTENDEE NAME | ADDRESS |
|------------------------|----------------------|
| (PLEASE PRINT | |
| CLEARLY) | |
| Jean Valler | |
| Skyler Busen (THA) | |
| Deplow HSWDEDSON | |
| Epert Waleur | |
| Kn CHUROTK | |
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Dear Mountain Village Design review board and Town Council:

I submit this letter in opposition to the application of MV Colorado Development Partners, LLC to extend the length and validity of their previously approved development plans which are set to expire if not initiated on December 8, 2015. I have been an owner of a unit at Westermere since around 1990.

The project and its approvals were granted with many variances to allowable construction which would be within the parameters of the current zoning.

The basis for these granted variances <u>including a previous allowed extension from the usual three years to five years without such this approval would have expired on December 8, 2013, were many commitments to the town among them:</u>

- a) Contribution of public restrooms.
- b) Employee housing mitigation payments to the town.
- c) Plaza improvements.
- d) Improvements to the Westermere Breezeway.

Other reasons used for the basis of granting such variances included:

- a) The project will help to create and preserve an attractive community.
- b) The project will promote the economic vitality of the town.

If this application is approved, the developer will have complied with none of the items used in granting the initial approvals. Perhaps some may be executed somewhere in the next 5 years (10 years from the initial approval), perhaps not. Apparently the developer is under no obligation to actually complete (or begin) this project. I assume the funds and improvements / benefits to the Village have not been received and may or may not be received in the future. The monetary promises are not escalated and the value of payments which would have been made in 2010 is certainly not worth as much to the public as they will be if made in 2020.

It is for these reasons that I believe the developer should have already made plans to begin his approved project prior to the already extended time given in the initial approval or lose his rights to the granted approvals and if they choose, begin the process again. The only reasoning for this extension would be to allow the developer more time to actually commit to this project which should have been a given prior to their application, the Village does not receive any benefit. If the Village truly valued the "HOT" beds this project would have provided, the project should have come with guarantees or penalties for non-compliance.

Mountain Village is not the same town it was 5 years ago and will not be the same 5 years from now. Approvals for variances should be for projects which are going to be

built, not contemplated as a possibility. The needs and desires of the Village are not the same now as they were in the past and they will again change in the future.

Thank you for your consideration.

Bruce Crown Westermere 311